

Stormwater Management Report

For the Proposed:

Costco at Evergreen Walk

Located at:

Evergreen Walk – Unit 12 151 Buckland Road South Windsor, CT

Prepared for Submission to:

Town of South Windsor, Connecticut

August 14, 2020

Prepared for:

Evergreen Walk, LLC 501 Evergreen Way, Suite 503 South Windsor, Connecticut 06074

Prepared by:

BL Companies

100 Constitution Plaza, 10th Floor Hartford, Connecticut 06103 Phone: (860) 249-2200

Fax: (860) 249-2400

BL Project Number: 13C4718

An Employee-Owned Company





7			4			4	
- 1	n	n	t	Δ	n	T	c

Executive Summary	1
Existing Site Conditions and Hydrologic Conditions	
Developed Site Conditions and Hydrologic Conditions	
Stormwater Management	
Summary	
Juiiiiai y	C

Appendix A: Location Maps

Figure 1: USGS Location Map

Figure 2: Aerial Location Map

Figure 3: NRCS Soil Survey Map with Hydrologic Soil Group Data

Figure 4: FEMA Federal Insurance Rate Map

Figure 5: NOAA Atlas 14 Storm Data

Appendix B: Pre-development Hydrology (2-, 10-, 25-, and 100-year storms)

Appendix C: Post-development Hydrology (2-, 10-, 25-, and 100-year storms)

Appendix D: Water Quality Calculations

CTDEEP Water Quality Volume Calculations CTDEEP Water Quality Flow Calculations Treatment Train Efficiency Worksheet CTDOT Hydrodynamic Separator Sizing

Flow Splitter Manhole Calculations

Appendix E: Drainage Maps

DRA-1 – Existing Master Plan Drainage Areas

ED-1 – Existing Drainage Mapping

PD-1 – Proposed Drainage Mapping

Appendix F: Stormwater System Operation and Maintenance Manual



Executive Summary

The proposed project is to develop Unit 12 at Evergreen Walk with a Costco with fueling station. The building pad site consists of approximately 16.2 acres of the entire development parcel with a total area of approximately 240 acres. Unit 12 is part of the Evergreen Walk Master Plan Area.

The site to be developed currently consists of predominantly grassed area. The ground generally slopes from east to west. Stormwater currently flows either to the existing stormwater management basin (Detention Basin #7) or the existing wetlands system west of the site. The existing developments to the east (LA Fitness) and northeast (ECHN II) both have existing stormwater conveyance pipes that transverse the site although they do not collect stormwater runoff from the proposed development area. They will continue to function in their current capacity with some minor relocation due to the proposed site layout.

The proposed site improvements will include the proposed Costco building, fueling station, paved parking areas, landscaped areas, pedestrian sidewalks, site utilities, and a stormwater management system. As a master planned development, the existing stormwater management system has been designed to convey the stormwater discharge from the previous approved developed site conditions. The current proposed site development will increase the amount of impervious ground cover on-site as well as a reduction of storage volume in Detention Basin #7 as necessary to accommodate the needs of the business. The existing stormwater management system will be supplemented with a subsurface detention system in order to maintain stormwater runoff rates as approved in the master plan.

The proposed 163,265 SF Costco building will have public water, sanitary sewer, natural gas, electric, and telecommunications services connections. Domestic and fire protection water shall be connected from an existing 8" service main installed beneath Tamarack Avenue as part of the master plan construction activities. This service will also loop to an existing 8" stub to the site installed as part of the master plan construction activities. Sanitary sewer laterals shall be pumped to a doghouse manhole on the existing 8" sanitary sewer main installed beneath Evergreen Way at the intersection with the access road. A natural gas connection is to be connected to an existing service line to the east, beneath Evergreen Way, the size and exact location of the proposed service lateral are to be coordinated with the local service provider. Electrical and telecommunications services shall be connected at an existing electrical and telephone service located within Tamarack Avenue. Two new transformers will be located on site.

This report has been prepared to complement the submitted project plans as well as to represent the technical basis for the stormwater management system designs presented herein. The proposed



stormwater management system is designed to be in compliance with the 2002 State of Connecticut Guidelines for Soil Erosion and Sediment Control, and the 2004 State of Connecticut Stormwater Quality Manual.

Water Quality Best Management Practices (BMP's) have also been incorporated in this design and include catch basins with deep sumps, hooded catch basin outlets, hydrodynamic separators, and a subsurface detention system which will connect to the existing drainage system previously sized and installed under the master plan.

A HydroCAD model, using TR-55 methodology, was developed to evaluate the existing and proposed drainage conditions of the property. The results of the analysis demonstrate that there will not be an increase in peak stormwater runoff rates for the 2-, 10-, 25-, and 100-year storm events.

Existing Site Conditions and Hydrologic Conditions

General Site Information

The site soil identified by the United States Department of Agriculture (USDA) Natural Resources Conservation Services (NRCS) consists of Scitico, Shaker, and Maybid soils, Elmridge fine sandy loam, 3 to 8 percent slopes, Saco silt loam, Tisbury silt loam, 0 to 3 percent slopes, Tisbury silt loam, 3 to 8 percent slopes, Enfield silt loam, 0 to 3 percent slopes, and Enfield silt loam, 3 to 8 percent slopes. Per the USDA the NRCS Hydrologic Soil Group the soil rating for within the project area is C/D, C, B/D, C, C, B, and B, respectively. A copy of the USDA NRCS Hydrologic Soil Group Map is included in Appendix A for reference.

Per the FEMA Flood Insurance Rate Map Number 09003C0383F for Hartford County, Connecticut, map revised date: September 26, 2008, the site resides in FEMA Flood Hazard Area X (unshaded). This is defined as "areas determined to be outside the 0.2% annual chance floodplain". Zone X may have ponding and local drainage problems that don't warrant a detailed study or designation as base floodplain. A copy of the FEMA Flood insurance rate Map is included in Appendix A for reference.

Existing Hydrologic Conditions

The existing site drainage area that was analyzed totals 40.27 acres and is approximately 64.2% impervious as approved as part of the master plan development. This area consists of 4 drainage areas within the master plan drainage area map. The only stormwater management system onsite



is Detention Basin #7. Stormwater from the rest of the analyzed site flows to the wetland system to the west.

The following is a brief analysis of the existing design points as shown on the enclosed Existing Drainage Mapping (ED-1) Map, in Appendix E.

Existing Drainage Area 1 (EDA-1): This drainage area consists of the entire area of the site draining to Detention Basin #7 (DP-1), it is 19.31 acres and is approximately 71% impervious. EDA-1 as approved as part of the master plan development consists mainly of building roof area, impervious parking lot area, and landscaped areas. The buildings consist of 5 office buildings and a hotel.

Existing Drainage Area 2 (EDA-2): This drainage area consists of a large portion of the site draining to the wetlands to the west of the site which was designated in the master plan drainage area map (DP-2), it is 17.17 acres and is approximately 71% impervious. EDA-2 as approved as part of the master plan development consists mainly of building roof area, impervious parking lot area, and landscaped areas. The buildings consist of 2 office buildings, a LA Fitness, 4 retail buildings, and a hotel.

Existing Drainage Area 3 (EDA-3): This drainage area consists of the area of the site draining to the culvert under the western road crossing the wetland (DP-3), it is 1.05 acres and is approximately 0% impervious. EDA-3 was not included in the master plan development since there was no proposed development in that area. EDA-3 consists mainly of grass and brush wetland areas.

Existing Drainage Area 4 (EDA-4): This drainage area consists of the area of the site draining to the culvert under the eastern road, Tamarack Avenue, crossing the wetland (DP-4), it is 2.74 acres and is approximately 0% impervious. EDA-4 was not included in the master plan development since there was no proposed development in that area. EDA-4 consists mainly of grass and brush wetland areas.



Table 1 – Pre-Development (Existing Conditions) Drainage Characteristics.

Drainage Area	Area (acres)	Composite Curve Number	Impervious Cover (%)	Time of Concentration (minutes)
EDA-1 (Area to Detention Basin #7)	19.31	90	71	15.0
EDA-2 (Area to Wetland DP-2)	17.17	90	71	8.0
EDA-3 (Area to Wetland DP-3)	1.05	73	0	9.3
EDA-4 (Area to Wetland DP-4)	2.74	69	0	18.1

Table 2 – Pre-Development Conditions Peak Flows

Analysis Daint	Peak Flow (cfs)				
Analysis Point	2-yr	10-yr	25-yr	100-yr	
Design Point 1	0.69	1.72	2.31	3.59	
Design Point 2	46.65	82.76	105.35	140.06	
Design Point 3	1.83	5.39	7.91	12.06	
Design Point 4	1.42	4.16	6.10	9.28	

Developed Site Conditions and Hydrologic Conditions

The proposed site drainage area totals 40.61 acres and is approximately 69.3% impervious. The intent of the proposed site drainage is to mimic existing drainage patterns to the maximum extent practical, however the exact area that was modeled for proposed conditions varies slightly from the existing due to the modifications of the overall master plan catchment areas. The site stormwater system will provide stormwater detention and quality improvements through the installation of catch basins with deep sumps and hooded outlets, hydrodynamic separators, a subsurface detention system, and a formalized street sweeping program for the impervious surfaces. These measures will treat the stormwater quality flow through structural means to provide water quality treatment in conformance with the State of Connecticut Water Quality Manual.

For the hydrologic analysis, the developed site retained the same Design Points as the existing model. The following drainage areas were developed to model the proposed site improvements.

Proposed Drainage Area 1A (PDA-1A): This drainage area consists of the northeastern portion of the site that drains to Detention Basin #7. It consists of two ECHN II buildings(constructed), 2 office buildings approved in the master plan development, Detention Basin #7 and a corner of the proposed fueling station. It is 11.60 acres and is approximately 74% impervious. PDA-1A consists mainly of impervious parking area and building roof area with landscaping. The majority of the



drainage in this area will remain unchanged. The existing drain pipe from the majority of PDA-1A with be rerouted around the proposed fueling station. The corner of the fueling station will drain to a curb inlet type catch basin, through a hydrodynamic separator, connect with the rerouted pipe, and eventually into Detention Basin #7.

Proposed Drainage Area 1B (PDA-1B): This drainage area consists of the southwestern portion of the site that drains to Detention Basin #7. It consists of the majority of the Costco parking lot as well as a portion of the fueling station. It is 7.79 acres and is approximately 86% impervious. PDA-1B consists mainly of impervious parking area with landscaping. The drainage area will drain to a series of curb and curb-less inlet hooded catch basins with deep sumps, through hydrodynamic separators, into a subsurface detention system, and eventually into Detention Basin #7.

Proposed Drainage Area 2A (PDA-2A): This drainage area consists of the eastern portion of the site that drains to wetland drainage point 2. It consists of LA Fitness and 4 retail buildings approved in the master plan development. It is 10.79 acres and is approximately 67% impervious. PDA-2A consists mainly of impervious parking area and building roof area with landscaping. This area will remain unchanged with the proposed development. The site discharges through an existing storm pipe that will be rerouted through the proposed development to eventually discharge to the wetlands to the west of the site.

Proposed Drainage Area 2B (PDA-2B): This drainage area consists of the southwestern portion of the site that drains to wetland drainage point 2. It consists of the Costco roof area as well as a portion of the parking lot and drive aisles. It is 6.59 acres and is approximately 85% impervious. PDA-1B consists mainly of impervious parking and roof area with landscaping. The drainage area will drain to a series of hooded catch basins with deep sumps, through hydrodynamic separators, and eventually discharging to western wetland.

Proposed Drainage Area 3 (PDA-3): This drainage area consists of the area of the site draining to the culvert under the western road crossing the wetland (DP-3), it is 1.09 acres and is approximately 0% impervious. The drainage area will remain largely unchanged except for some minor regrading. PDA-3 consists mainly of grass and brush wetland areas.

Proposed Drainage Area 4 (PDA-4): This drainage area consists of the area of the site draining to the culvert under the eastern road, Tamarack Avenue, crossing the wetland (DP-4), it is 2.74 acres and is approximately 0% impervious. PDA-4 remains entirely unchanged. PDA-4 consists mainly of grass and brush wetland areas.



Table 3 – Post Development Drainage Characteristics.

Drainage Area / Design Point	Area (acres)	Composite Curve	Impervious Cover (%)	Time of Concentration
		Number		(minutes)
PDA-1A	11.60	91	74	15.9
(Area to Detention Basin #7)				
PDA-1B	7.79	94	86	5.0
(Area to Subsurface Detention System)				
PDA-2A	10.79	88	67	8.0
(Area to Wetland DP-2)				
PDA-2B	6.59	94	85	5.0
(Building Area)				
PDA-3	1.09	74	0	9.3
(Area to Wetland DP-3)				
PDA-4	2.74	69	0	18.1
(Area to Wetland DP-4)				

Table 4 – Post-Development Conditions Peak Flows

Analysis Point	Peak Flow (cfs)				
Analysis Foliit	2-yr	10-yr	25-yr	100-yr	
Design Point 1	0.68	1.71	2.30	3.59	
Design Point 2	44.30	79.63	101.82	136.05	
Design Point 3	1.88	5.49	8.04	12.23	
Design Point 4	1.42	4.16	6.10	9.28	



Table 5 – Existing vs Proposed Peak Rates of Runoff

	Peak Flow Rate in Cubic Feet per Second (cfs)				
Drainage Area	2-yr	10-yr	25-yr	100-yr	
Design Point 1		-		-	
Existing	0.69	1.72	2.31	3.59	
Proposed	0.68	1.71	2.30	3.59	
Percent Change	-1.4%	-0.6%	-0.4%	0.0%	
Design Point 2					
Existing	46.65	82.76	105.35	140.06	
Proposed	44.30	79.63	101.82	136.05	
Percent Change	-5.0%	-3.8%	-3.4%	-2.9%	
Design Point 3					
Existing	1.83	5.39	7.91	12.06	
Proposed	1.88	5.49	8.04	12.23	
Percent Change	2.7%	1.9%	1.6%	1.4%	
Design Point 4					
Existing	1.42	4.16	6.10	9.28	
Proposed	1.42	4.16	6.10	9.28	
Percent Change	0.0%	0.0%	0.0%	0.0%	

Stormwater Management

Hydrologic Modeling of the Entire Site

The hydrologic analysis to determine peak stormwater discharge rates was performed using the HydroCAD stormwater modeling system computer program, version 10.00 developed by HydroCAD Software Solutions, LLC. Hydrographs for each watershed were developed using the SCS Synthetic Unit Hydrograph Method. Rainfall depths and distribution per the NOAA Atlas 14 for South Windsor, CT were used for the calculation of peak flow rates and are listed in Table 6. The drainage areas, or subcatchments as labeled by the program, are depicted by hexagons on the attached drainage diagrams. Pre-development HydroCAD output can be found in Appendix B and Post-development HydroCAD output can be found in Appendix C.



Table 6 – Rainfall Depths per NOAA Atlas 14 Appendix B - 24-hour Rainfall Data

Return Period	24-hour Rainfall Depth
2-year	3.11"
10-year	4.91"
25-year	6.03"
100-year	7.77"

Summary

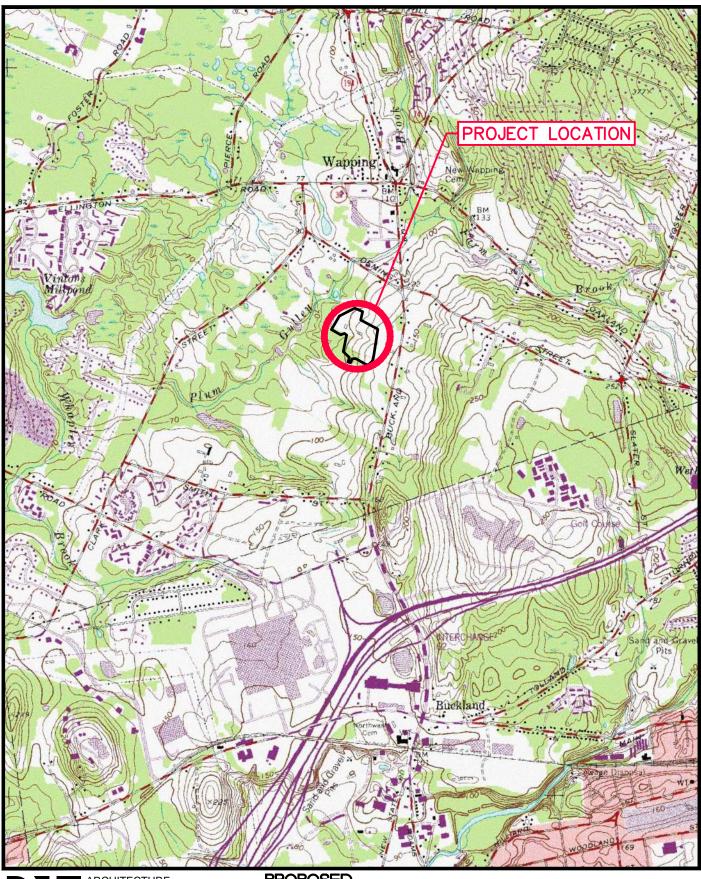
The post-development peak discharge rates for the total developed site have been matched or decreased for all storm events for Drainage Areas 1, 2, and 4. There is a slight increase in discharge rates for Drainage Area 3, however, Drainage Area 3 flows into Drainage Area 2 and there is a reduction in discharge rates overall. All post development stormwater will be discharged offsite to mimic existing drainage patterns. Stormwater quality is being addressed by formulized street sweeping, catch basins with deep sumps and hooded outlets, hydrodynamic separators, and a subsurface detention system. These features will provide the minimum required 80% TSS removal as required in the CT Stormwater Manual. The proposed stormwater management system will meet the stormwater quality requirements of the State of Connecticut.



APPENDIX A

LOCATION MAPS

Figure 1: USGS Location Map
Figure 2: Aerial Location Map
Figure 3: NRCS Soil Survey Map with Hydrologic Soil Group Data
Figure 4: FEMA Federal Insurance Rate Map
Figure 5: NOAA Atlas 14 Storm Data





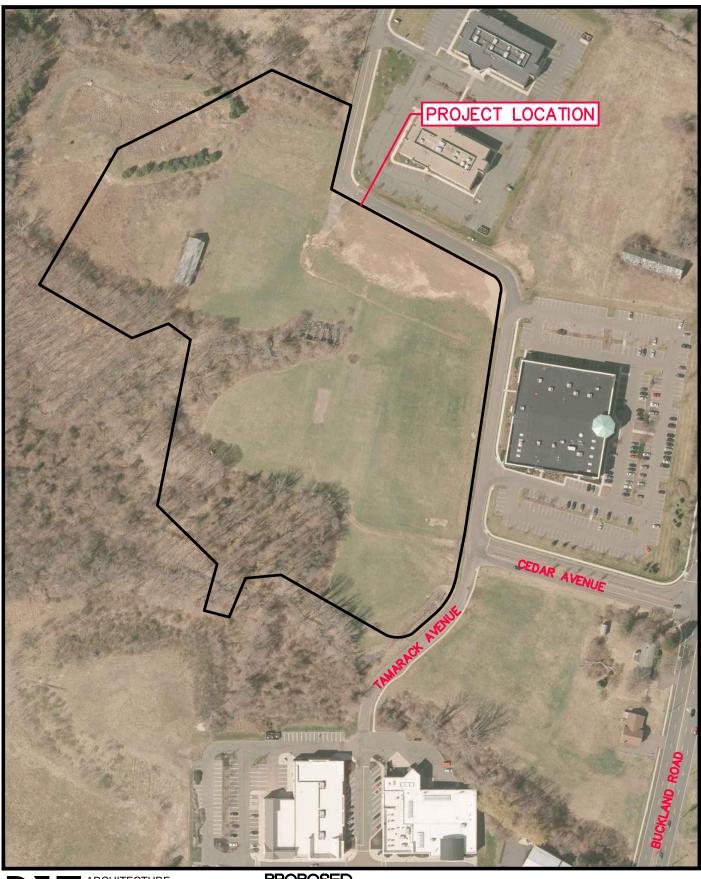
ARCHITECTURE ENGINEERING ENVIRONMENTAL LAND SURVEYING

PROPOSED COSTCO AT EVERGREEN WALK

EVERGREEN WALK — UNIT 12 151 BUCKLAND ROAD SOUTH WINDSOR, CONNECTICUT Designed
Drawn
Checked
Approved
Scale
Project No.
Date
CAD File

S.E.L. S.E.L. M.J.B. M.J.B. 1"=2,000' 13C4718 10/03/2018 LOC13C471801

FIGURE 1
USGS LOCATION MAP





ARCHITECTURE **ENGINEERING** ENVIRONMENTAL LAND SURVEYING

PROPOSED COSTCO AT EVERGREEN WALK

EVERGREEN WALK - UNIT 12 151 BUCKLAND ROAD SOUTH WINDSOR, CONNECTICUT

Designed Drawn Checked Approved Scale Project No. Date CAD File

S.E.L. S.E.L. M.J.B. M.J.B. 1"=2,000' 13C4718 10/03/2018 LOC13C471801



VRCS

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for State of Connecticut



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2 053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

Contents

Preface	2
Soil Map	5
Soil Map	6
Legend	7
Map Unit Legend	
Map Unit Descriptions	8
State of Connecticut	10
9—Scitico, Shaker, and Maybid soils	. 10
12—Raypol silt loam	
28A—Elmridge fine sandy loam, 0 to 3 percent slopes	
28B—Elmridge fine sandy loam, 3 to 8 percent slopes	16
29B—Agawam fine sandy loam, 3 to 8 percent slopes	18
108—Saco silt loam	. 20
702A—Tisbury silt loam, 0 to 3 percent slopes	. 22
702B—Tisbury silt loam, 3 to 8 percent slopes	. 23
704A—Enfield silt loam, 0 to 3 percent slopes	25
704B—Enfield silt loam, 3 to 8 percent slopes	27
Soil Information for All Uses	29
Soil Properties and Qualities	. 29
Soil Qualities and Features	
Hydrologic Soil Group	. 29

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



MAP LEGEND

å

00

Ŷ

Δ

Water Features

Transportation

00

Background

Spoil Area

Stony Spot

Wet Spot

Other

Rails

US Routes

Major Roads

Local Roads

Very Stony Spot

Special Line Features

Streams and Canals

Interstate Highways

Aerial Photography

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons

Soil Map Unit Lines

Soil Map Unit Points

Special Point Features

(o) Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

A Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

→ Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

ID MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: State of Connecticut Survey Area Data: Version 17, Sep 5, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Mar 28, 2011—Apr 18, 2011

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
9	Scitico, Shaker, and Maybid soils	1.5	1.5%
12	Raypol silt loam	2.5	2.4%
28A	Elmridge fine sandy loam, 0 to 3 percent slopes	2.1	2.1%
28B	Elmridge fine sandy loam, 3 to 8 percent slopes	14.0	13.4%
29B	Agawam fine sandy loam, 3 to 8 percent slopes	0.8	0.7%
108	Saco silt loam	14.5	13.8%
702A	Tisbury silt loam, 0 to 3 percent slopes	4.7	4.5%
702B	Tisbury silt loam, 3 to 8 percent slopes	10.8	10.4%
704A	Enfield silt loam, 0 to 3 percent slopes	15.2	14.6%
704B	Enfield silt loam, 3 to 8 percent slopes	38.4	36.7%
Totals for Area of Interest		104.6	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They

generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

State of Connecticut

9—Scitico, Shaker, and Maybid soils

Map Unit Setting

National map unit symbol: 9Irq Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 50 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Scitico and similar soils: 40 percent Shaker and similar soils: 30 percent Maybid and similar soils: 15 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Scitico

Setting

Landform: Depressions, drainageways, terraces

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Clayey glaciolacustrine deposits

Typical profile

Ap - 0 to 8 inches: silt loam Eg - 8 to 11 inches: silt loam

Bg1 - 11 to 18 inches: silty clay loam Bg2 - 18 to 30 inches: silty clay loam Bg3 - 30 to 38 inches: silty clay Cg1 - 38 to 52 inches: silty clay loam Cg2 - 52 to 65 inches: silty clay

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Poorly drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

low (0.00 to 0.06 in/hr)

Depth to water table: About 0 to 12 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: High (about 11.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: D Hydric soil rating: Yes

Description of Shaker

Setting

Landform: Depressions, drainageways, terraces

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Coarse-loamy eolian deposits over clayey glaciolacustrine

deposits

Typical profile

Oe - 0 to 2 inches: moderately decomposed plant material

Ap - 2 to 6 inches: fine sandy loam
Bg - 6 to 20 inches: sandy loam
Bw - 20 to 30 inches: sandy loam
2C - 30 to 65 inches: silty clay

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Poorly drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

high (0.00 to 0.20 in/hr)

Depth to water table: About 0 to 18 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: High (about 9.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: C/D Hydric soil rating: Yes

Description of Maybid

Setting

Landform: Depressions, drainageways, terraces

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Clayey glaciolacustrine deposits

Typical profile

A - 0 to 9 inches: silt loam

Bg1 - 9 to 18 inches: silty clay loam Bg2 - 18 to 26 inches: silty clay loam Cg1 - 26 to 36 inches: silty clay loam Cg2 - 36 to 60 inches: silty clay loam

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Very poorly drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

high (0.00 to 0.20 in/hr)

Depth to water table: About 0 to 6 inches

Frequency of flooding: None Frequency of ponding: Occasional

Available water storage in profile: High (about 11.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6w

Hydrologic Soil Group: C/D Hydric soil rating: Yes

Minor Components

Brancroft

Percent of map unit: 5 percent

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Elmridae

Percent of map unit: 5 percent

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Unnamed, sand or gravel substratum

Percent of map unit: 3 percent

Hydric soil rating: No

Unnamed, red parent material

Percent of map unit: 2 percent

12—Raypol silt loam

Map Unit Setting

National map unit symbol: 9ljx Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 54 inches
Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Raypol and similar soils: 80 percent Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Raypol

Setting

Landform: Depressions, drainageways

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Coarse-loamy eolian deposits over sandy and gravelly glaciofluvial deposits derived from granite and/or schist and/or gneiss

Typical profile

Ap - 0 to 8 inches: silt loam

Bg1 - 8 to 12 inches: very fine sandy loam

Bg2 - 12 to 20 inches: silt loam Bw1 - 20 to 26 inches: silt loam

Bw2 - 26 to 29 inches: very fine sandy loam

2C1 - 29 to 52 inches: stratified very gravelly coarse sand to loamy fine sand 2C2 - 52 to 65 inches: stratified very gravelly coarse sand to loamy fine sand

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Poorly drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to

high (0.57 to 1.98 in/hr)

Depth to water table: About 0 to 12 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Moderate (about 7.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: C/D Hydric soil rating: Yes

Minor Components

Haven

Percent of map unit: 5 percent Landform: Outwash plains, terraces

Down-slope shape: Convex Across-slope shape: Linear

Hydric soil rating: No

Enfield

Percent of map unit: 5 percent Landform: Outwash plains, terraces

Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

Ninigret

Percent of map unit: 3 percent Landform: Outwash plains, terraces

Down-slope shape: Linear Across-slope shape: Concave

Hydric soil rating: No

Tisbury

Percent of map unit: 2 percent Landform: Outwash plains, terraces

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Walpole

Percent of map unit: 2 percent

Landform: Depressions on terraces, drainageways on terraces

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Scarboro

Percent of map unit: 2 percent

Landform: Terraces, depressions, drainageways

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes
Unnamed, loamy substratum

Percent of map unit: 1 percent

28A—Elmridge fine sandy loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 9lm0 Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Elmridge and similar soils: 80 percent

Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Elmridge

Settina

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear

Parent material: Coarse-loamy eolian sands over clayey glaciolacustrine deposits

Typical profile

Ap - 0 to 6 inches: fine sandy loam Bw1 - 6 to 10 inches: fine sandy loam

Bw2 - 10 to 18 inches: fine sandy loam Bw3 - 18 to 25 inches: sandy loam 2C - 25 to 65 inches: silty clay

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Moderately well drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

low (0.00 to 0.06 in/hr)

Depth to water table: About 18 to 30 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Moderate (about 9.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2w

Hydrologic Soil Group: C Hydric soil rating: No

Minor Components

Brancroft

Percent of map unit: 4 percent

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Belgrade

Percent of map unit: 4 percent

Landform: Terraces
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Unnamed, red parent material

Percent of map unit: 2 percent

Hydric soil rating: No

Ninigret

Percent of map unit: 2 percent Landform: Outwash plains, terraces

Down-slope shape: Linear Across-slope shape: Concave

Hydric soil rating: No

Sudbury

Percent of map unit: 2 percent Landform: Terraces, outwash plains

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Scitico

Percent of map unit: 2 percent

Landform: Drainageways, terraces, depressions

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Shaker

Percent of map unit: 2 percent

Landform: Depressions, drainageways, terraces

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Berlin

Percent of map unit: 1 percent

Landform: Terraces
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Maybid

Percent of map unit: 1 percent

Landform: Depressions, drainageways, terraces

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

28B—Elmridge fine sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 9lm1 Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Elmridge and similar soils: 80 percent Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Elmridge

Setting

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear

Parent material: Coarse-loamy eolian sands over clayey glaciolacustrine deposits

Typical profile

Ap - 0 to 6 inches: fine sandy loam

Bw1 - 6 to 10 inches: fine sandy loam Bw2 - 10 to 18 inches: fine sandy loam Bw3 - 18 to 25 inches: sandy loam 2C - 25 to 65 inches: silty clay

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Moderately well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately

low (0.00 to 0.06 in/hr)

Depth to water table: About 18 to 30 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Moderate (about 9.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2w

Hydrologic Soil Group: C Hydric soil rating: No

Minor Components

Brancroft

Percent of map unit: 5 percent

Landform: Terraces

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Shaker

Percent of map unit: 3 percent

Landform: Depressions, drainageways, terraces

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Unnamed, red parent material

Percent of map unit: 2 percent

Hydric soil rating: No

Sudburv

Percent of map unit: 2 percent Landform: Outwash plains, terraces

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Belgrade

Percent of map unit: 2 percent

Landform: Terraces
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Ninigret

Percent of map unit: 2 percent Landform: Outwash plains, terraces

Down-slope shape: Linear Across-slope shape: Concave

Hydric soil rating: No

Scitico

Percent of map unit: 2 percent

Landform: Terraces, depressions, drainageways

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Berlin

Percent of map unit: 1 percent

Landform: Terraces
Down-slope shape: Linear
Across-slope shape: Linear
Hydric soil rating: No

Maybid

Percent of map unit: 1 percent

Landform: Depressions, drainageways, terraces

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

29B—Agawam fine sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2tyqx

Elevation: 0 to 820 feet

Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 250 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Agawam and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Agawam

Setting

Landform: Outwash terraces, kame terraces, kames, outwash plains, moraines Landform position (two-dimensional): Backslope, shoulder, footslope, summit Landform position (three-dimensional): Side slope, crest, tread, riser, rise, dip

Down-slope shape: Convex

Across-slope shape: Convex

Parent material: Coarse-loamy eolian deposits over sandy and gravelly glaciofluvial deposits derived from gneiss, granite, schist, and/or phyllite

Typical profile

Ap - 0 to 11 inches: fine sandy loam Bw1 - 11 to 16 inches: fine sandy loam Bw2 - 16 to 26 inches: fine sandy loam 2C1 - 26 to 45 inches: loamy fine sand 2C2 - 45 to 55 inches: loamy fine sand 2C3 - 55 to 65 inches: loamy sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: 15 to 35 inches to strongly contrasting textural

stratification

Natural drainage class: Well drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high

(0.14 to 14.17 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Salinity, maximum in profile: Nonsaline (0.0 to 1.9 mmhos/cm) Available water storage in profile: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2s

Hydrologic Soil Group: B Hydric soil rating: No

Minor Components

Sudbury

Percent of map unit: 5 percent

Landform: Outwash plains, terraces, deltas
Landform position (two-dimensional): Footslope
Landform position (three-dimensional): Tread, dip

Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Hinckley

Percent of map unit: 5 percent

Landform: Outwash plains, eskers, kames, deltas

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Nose slope, side slope, crest, head slope,

rise

Down-slope shape: Convex

Across-slope shape: Linear, convex

Hydric soil rating: No

Merrimac

Percent of map unit: 3 percent

Landform: Kames, eskers, moraines, outwash terraces, outwash plains
Landform position (two-dimensional): Backslope, footslope, shoulder, summit

Landform position (three-dimensional): Side slope, crest, riser, tread

Down-slope shape: Convex Across-slope shape: Convex

Hydric soil rating: No

Windsor

Percent of map unit: 2 percent

Landform: Deltas, outwash plains, dunes, outwash terraces

Landform position (three-dimensional): Riser, tread

Down-slope shape: Linear, convex Across-slope shape: Linear, convex

Hydric soil rating: No

108—Saco silt loam

Map Unit Setting

National map unit symbol: 9ljv Elevation: 0 to 1.200 feet

Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: Not prime farmland

Map Unit Composition

Saco and similar soils: 80 percent Minor components: 20 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Saco

Setting

Landform: Flood plains Down-slope shape: Concave Across-slope shape: Concave Parent material: Coarse-silty alluvium

Typical profile

A - 0 to 12 inches: silt loam Cg1 - 12 to 32 inches: silt loam Cg2 - 32 to 48 inches: silt loam

2Cg3 - 48 to 60 inches: stratified very gravelly coarse sand to loamy fine sand

Properties and qualities

Slope: 0 to 2 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Very poorly drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to

high (0.57 to 1.98 in/hr)

Depth to water table: About 0 to 6 inches

Frequency of flooding: Frequent

Frequency of ponding: Frequent

Available water storage in profile: High (about 10.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6w

Hydrologic Soil Group: B/D Hydric soil rating: Yes

Minor Components

Lim

Percent of map unit: 5 percent Landform: Flood plains Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Limerick

Percent of map unit: 5 percent Landform: Flood plains Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Winooski

Percent of map unit: 3 percent Landform: Flood plains Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Rippowam

Percent of map unit: 3 percent Landform: Flood plains Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

Hadley

Percent of map unit: 2 percent Landform: Flood plains Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Bash

Percent of map unit: 2 percent Landform: Flood plains Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

702A—Tisbury silt loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 2y07g

Elevation: 0 to 1,260 feet

Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Tisbury and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Tisbury

Setting

Landform: Valley trains, outwash plains, deltas, outwash terraces

Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Coarse-silty eolian deposits over sandy and gravelly glaciofluvial

deposits derived from granite, schist, and/or gneiss

Typical profile

Ap - 0 to 8 inches: silt loam Bw1 - 8 to 18 inches: silt loam Bw2 - 18 to 26 inches: silt loam

2C - 26 to 65 inches: extremely gravelly sand

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: 24 to 36 inches to strongly contrasting textural

stratification

Natural drainage class: Moderately well drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high

(0.14 to 14.17 in/hr)

Depth to water table: About 18 to 30 inches

Frequency of flooding: None Frequency of ponding: None

Salinity, maximum in profile: Nonsaline (0.0 to 1.9 mmhos/cm) Available water storage in profile: Low (about 4.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: C Hydric soil rating: No

Minor Components

Merrimac

Percent of map unit: 5 percent

Landform: Eskers, moraines, outwash terraces, outwash plains, kames

Landform position (two-dimensional): Shoulder, summit

Landform position (three-dimensional): Side slope, crest, tread

Down-slope shape: Convex Across-slope shape: Convex

Hydric soil rating: No

Agawam

Percent of map unit: 5 percent

Landform: Kames, moraines, outwash terraces, outwash plains, kame terraces

Landform position (two-dimensional): Summit, shoulder

Landform position (three-dimensional): Side slope, crest, tread

Down-slope shape: Convex Across-slope shape: Convex

Hydric soil rating: No

Ninigret

Percent of map unit: 3 percent

Landform: Outwash terraces, kames, moraines, outwash plains, kame terraces

Landform position (two-dimensional): Footslope, toeslope Landform position (three-dimensional): Base slope, tread

Down-slope shape: Linear, convex Across-slope shape: Concave, convex

Hydric soil rating: No

Raypol

Percent of map unit: 2 percent

Landform: Depressions, drainageways

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

702B—Tisbury silt loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2y07h

Elevation: 0 to 1,260 feet

Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Tisbury and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Tisbury

Setting

Landform: Deltas, valley trains, outwash plains, outwash terraces

Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Concave

Parent material: Coarse-silty eolian deposits over sandy and gravelly glaciofluvial

deposits derived from granite, schist, and/or gneiss

Typical profile

Ap - 0 to 8 inches: silt loam
Bw1 - 8 to 18 inches: silt loam
Bw2 - 18 to 26 inches: silt loam

2C - 26 to 65 inches: extremely gravelly sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: 24 to 36 inches to strongly contrasting textural

stratification

Natural drainage class: Moderately well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high

(0.14 to 14.17 in/hr)

Depth to water table: About 18 to 30 inches

Frequency of flooding: None Frequency of ponding: None

Salinity, maximum in profile: Nonsaline (0.0 to 1.9 mmhos/cm) Available water storage in profile: Low (about 4.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: C Hydric soil rating: No

Minor Components

Merrimac

Percent of map unit: 5 percent

Landform: Outwash plains, kames, eskers, moraines, outwash terraces Landform position (two-dimensional): Backslope, footslope, shoulder, summit,

toeslope

Landform position (three-dimensional): Side slope, crest, head slope, nose slope,

tread

Down-slope shape: Convex Across-slope shape: Convex

Hydric soil rating: No

Agawam

Percent of map unit: 5 percent

Landform: Kame terraces, kames, moraines, outwash terraces, outwash plains Landform position (two-dimensional): Backslope, shoulder, footslope, summit,

Landform position (three-dimensional): Side slope, crest, head slope, nose slope, tread

Custom Soil Resource Report

Down-slope shape: Convex Across-slope shape: Convex

Hydric soil rating: No

Ninigret

Percent of map unit: 3 percent

Landform: Outwash terraces, kames, outwash plains, kame terraces, moraines

Landform position (two-dimensional): Footslope, backslope, toeslope

Landform position (three-dimensional): Base slope, tread

Down-slope shape: Linear, convex Across-slope shape: Concave, convex

Hydric soil rating: No

Raypol

Percent of map unit: 2 percent

Landform: Depressions, drainageways

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

704A—Enfield silt loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 2y07p

Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Enfield and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Enfield

Setting

Landform: Outwash terraces, outwash plains Landform position (three-dimensional): Tread

Down-slope shape: Convex Across-slope shape: Linear

Parent material: Coarse-silty eolian deposits over sandy and gravelly glaciofluvial

deposits derived from granite, schist, and/or gneiss

Typical profile

Ap - 0 to 7 inches: silt loam
Bw1 - 7 to 15 inches: silt loam
Bw2 - 15 to 25 inches: silt loam

2C - 25 to 60 inches: stratified very gravelly coarse sand to loamy sand

Custom Soil Resource Report

Properties and qualities

Slope: 0 to 3 percent

Depth to restrictive feature: 16 to 39 inches to strongly contrasting textural

stratification

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to

high (0.57 to 1.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Low (about 4.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 1

Hydrologic Soil Group: B Hydric soil rating: No

Minor Components

Haven

Percent of map unit: 5 percent

Landform: Outwash terraces, outwash plains Landform position (three-dimensional): Tread

Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

Tisbury

Percent of map unit: 5 percent

Landform: Deltas, valley trains, outwash terraces, outwash plains

Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: No

Agawam

Percent of map unit: 3 percent

Landform: Kames, moraines, outwash terraces, outwash plains, kame terraces

Landform position (two-dimensional): Summit, shoulder

Landform position (three-dimensional): Side slope, crest, tread

Down-slope shape: Convex Across-slope shape: Convex

Hydric soil rating: No

Raypol

Percent of map unit: 2 percent

Landform: Depressions, drainageways

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

704B—Enfield silt loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2y07q

Elevation: 0 to 1,200 feet

Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F

Frost-free period: 140 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Enfield and similar soils: 85 percent Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Enfield

Setting

Landform: Outwash plains, outwash terraces Landform position (three-dimensional): Tread

Down-slope shape: Convex Across-slope shape: Linear

Parent material: Coarse-silty eolian deposits over sandy and gravelly glaciofluvial

deposits derived from granite, schist, and/or gneiss

Typical profile

Ap - 0 to 7 inches: silt loam
Bw1 - 7 to 15 inches: silt loam
Bw2 - 15 to 25 inches: silt loam

2C - 25 to 60 inches: stratified very gravelly coarse sand to loamy sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: 16 to 39 inches to strongly contrasting textural

stratification

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to

high (0.57 to 1.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Low (about 4.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: B Hydric soil rating: No

Minor Components

Haven

Percent of map unit: 5 percent

Landform: Outwash plains, outwash terraces Landform position (three-dimensional): Tread

Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

Tisbury

Percent of map unit: 5 percent

Landform: Outwash plains, deltas, valley trains, outwash terraces

Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: No

Agawam

Percent of map unit: 3 percent

Landform: Outwash plains, kame terraces, kames, moraines, outwash terraces Landform position (two-dimensional): Backslope, shoulder, footslope, summit,

toeslope

Landform position (three-dimensional): Side slope, crest, head slope, nose slope,

tread

Down-slope shape: Convex Across-slope shape: Convex

Hydric soil rating: No

Raypol

Percent of map unit: 2 percent

Landform: Drainageways, depressions

Down-slope shape: Concave Across-slope shape: Concave

Hydric soil rating: Yes

Soil Information for All Uses

Soil Properties and Qualities

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

Soil Qualities and Features

Soil qualities are behavior and performance attributes that are not directly measured, but are inferred from observations of dynamic conditions and from soil properties. Example soil qualities include natural drainage, and frost action. Soil features are attributes that are not directly part of the soil. Example soil features include slope and depth to restrictive layer. These features can greatly impact the use and management of the soil.

Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

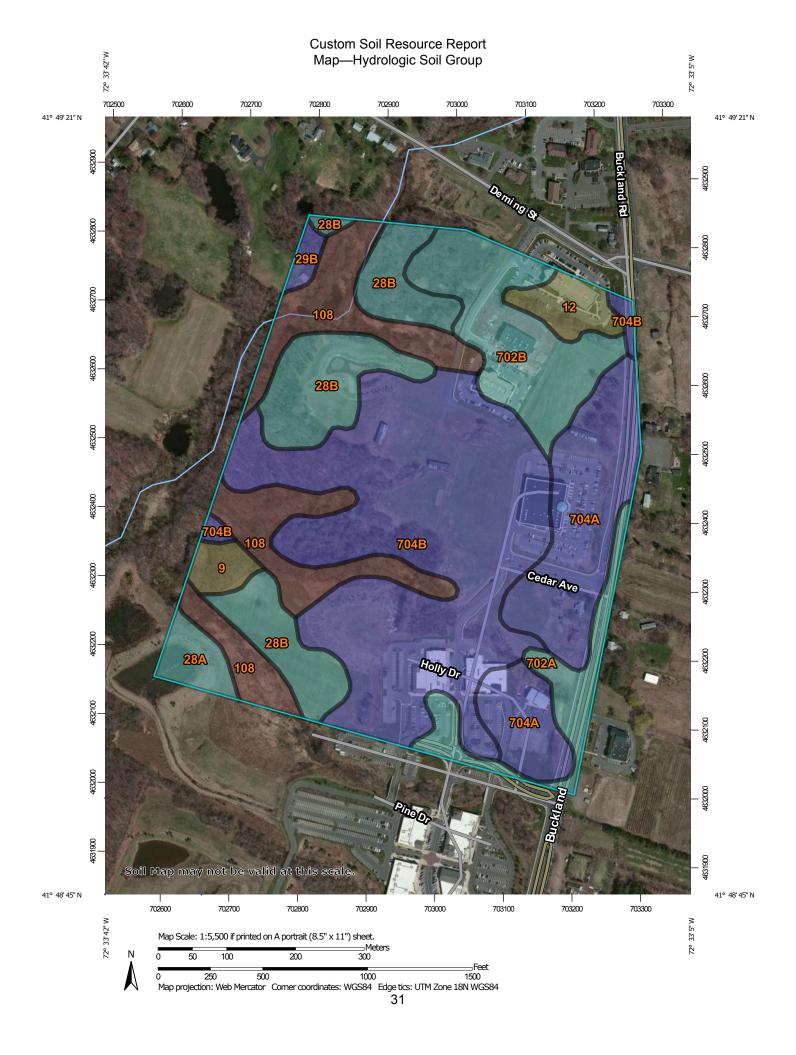
Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Custom Soil Resource Report

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.



MAP LEGEND MAP INFORMATION Area of Interest (AOI) The soil surveys that comprise your AOI were mapped at С 1:12.000. Area of Interest (AOI) C/D Soils D Warning: Soil Map may not be valid at this scale. Soil Rating Polygons Not rated or not available Α Enlargement of maps beyond the scale of mapping can cause **Water Features** A/D misunderstanding of the detail of mapping and accuracy of soil Streams and Canals line placement. The maps do not show the small areas of В contrasting soils that could have been shown at a more detailed Transportation scale. B/D Rails ---С Interstate Highways Please rely on the bar scale on each map sheet for map C/D **US Routes** measurements. Major Roads Source of Map: Natural Resources Conservation Service Not rated or not available Local Roads Web Soil Survey URL: -Coordinate System: Web Mercator (EPSG:3857) Soil Rating Lines Background Aerial Photography Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as C/D of the version date(s) listed below. Soil Survey Area: State of Connecticut Not rated or not available Survey Area Data: Version 17, Sep 5, 2018 **Soil Rating Points** Soil map units are labeled (as space allows) for map scales Α 1:50.000 or larger. A/D Date(s) aerial images were photographed: Mar 28, 2011—Apr 18. 2011 B/D The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor

shifting of map unit boundaries may be evident.

Table—Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
9	Scitico, Shaker, and Maybid soils	C/D	1.5	1.5%
12	Raypol silt loam	C/D	2.5	2.4%
28A	Elmridge fine sandy loam, 0 to 3 percent slopes	С	2.1	2.1%
28B	Elmridge fine sandy loam, 3 to 8 percent slopes	С	14.0	13.4%
29B	Agawam fine sandy loam, 3 to 8 percent slopes	В	0.8	0.7%
108	Saco silt loam	B/D	14.5	13.8%
702A	Tisbury silt loam, 0 to 3 percent slopes	С	4.7	4.5%
702B	Tisbury silt loam, 3 to 8 percent slopes	С	10.8	10.4%
704A	Enfield silt loam, 0 to 3 percent slopes	В	15.2	14.6%
704B	Enfield silt loam, 3 to 8 percent slopes	В	38.4	36.7%
Totals for Area of Inter-	est	-	104.6	100.0%

Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevation (BFEs) and/or floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM, Users should be aware that BFEs shown on the FIRM repears rounded whole-foot slewsteins. These BFEs are intended for flood insurance rating purposes only and should not be used carefully the profiles of the FIS should be utilized in concerningly, those delivation can be appropriated to the FIS should be utilized in objunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Rase Flood Elevation (BFEs) shown on this map apply only land-ward of 0.0" North American Vertical Datum (NAVD). Users of this FIRM should be aware that coastal flood elevations may also be provided in the Summary of Sillwater Elevations table in the Flood Insurance Study report for this community. Eventions shown in the Summary of Sillwater Eventions shown in the Summary of Sillwater Eventions shown in the Summary of Sillwater Eventions shown the shown that the shown that the shown that the shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Poyars. Floodway words and other pertirent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood** control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures in this jurisdiction.

National Geodetic Survey, NOA Silver Spring Metro Center 1316 East-West Highway Silver Spring, Meryland 20910 (301) 713-3242

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the National Geofectine Survey at (301) 713-3242, or vicit their website at www.ngs.nose.gov.

Base map information shown on this FIRM was provided in digital format by CT DEP.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, may exert should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

An accompanying Flood Insurance Study report, Letters of Map Revision or Letters of Map Amendment revising portions of this panel, and digital versions of this PANEL may be available. Contact the FEMA Map Service Center at the following phone numbers and internet address for infomation on all related products available from FEMA.

If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA-website at http://www.fema.gov/business/nfip/

This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FRM for this jurisdiction. The floodplaine and floodways that were transferred from the previous FRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report may reflect stream channel distances that differ from what is shown on this map.



LEGEND

SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD EVENT

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equilled or exceeded in very given year. The Special Flood Hazard Aves in the vere subject to flooding by the 1% annual chance flood. Aves of Special Flood Floates' include Zones A, AE, AH, AO, AR, AB9, V, and VE. The Base Flood Elevation is the water surface deviation of the 1% enrual chance flood.

ZONE A No base flood elevations determined.

ZONE AH Rood depths of 1 to 3 feet (usually areas of ponding); base flood elevations determined.

Flood depths of 1 to 3 feet usually sheet flow on aloping terrain average depths determined. For areas of alluvial fan flooding, velocities also determined. ZONE AO

ZONE A99

Coastal flood zone with velocity hazard (wave action); base flood alevat determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encreachment so that the 1% annual chance flood can be carried without whetherful programs in flood halphore.

OTHER FLOOD AREAS

Areas of 0.2% annual chance flood; areas of 1% annual chance flood with sources depths of less than 1 foot or with drainings areas from 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONEX ZONE D Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs) CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas

> 1% annual chance floodplain boundary 0.2% annual chance floodplain boundary

Floodway boundary

CBRS and OPA boundary

Boundary dividing Special Flood Hazard Areas of different Saver Flood Stavations, flood depths or valorities.

-----513------Base Flood Elevation line and value; elevation in 0* Sees Flood Elevation value where uniform within zon elevation in O*

A A 23------(23)

Geographic coordinates referenced to the North American Datum of 1963 (NAD 83)

4276000M 1000-meter Universal Transverse Mercator grid values, zone 18 600000 FT 5000 foot grid ticks

DX5510 X

River Mile

MAP REPOSITORY Refer to Repository Listing on Index Map

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL

For community map revision history prior to countywide mapping, refer to the Co-Map History table located in the Flood Insurance Shady report for this buildiction.

400 MAP SCALE 1" = 500"

250 0 500 1000 FEET

PANEL 0383F FIRM FLOOD INSURANCE RATE MAP HARTFORD COUNTY, CONNECTICUT (ALL JURISDICTIONS) F.F. PANEL 383 OF 675 (SEE MAP INDEX FOR FIRM PANEL LAYOUT) CONTAINS MANCHESTER, TOWN OF 090301 0383 F Notice to User. The Map Number shown below should be used when placing map orders; the Community Number shows should be used an insurance applications for the subject. fru Federal Emergency Management Agency



NOAA Atlas 14, Volume 10, Version 2 Location name: South Windsor, Connecticut, USA* Latitude: 41.8163°, Longitude: -72.5538° Elevation: 148.62 ft**

* source: ESRI Maps ** source: USGS



Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

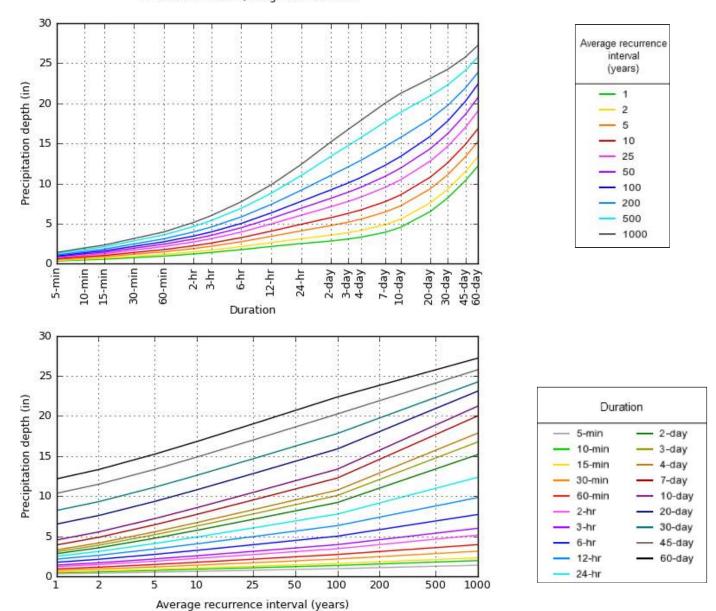
PDS-	DS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹									
Duration				Average	recurrence	interval (y	ears)			
Daration	1	2	5	10	25	50	100	200	500	1000
5-min	0.334 (0.259-0.431)	0.406 (0.314-0.524)	0.523 (0.403-0.677)	0.620 (0.476-0.807)	0.754 (0.560-1.03)	0.857 (0.625-1.19)	0.960 (0.681-1.39)	1.09 (0.732-1.61)	1.26 (0.816-1.93)	1.39 (0.880-2.17)
10-min	0.473 (0.367-0.610)	0.575 (0.445-0.742)	0.741 (0.571-0.959)	0.878 (0.674-1.14)	1.07 (0.794-1.46)	1.21 (0.885-1.69)	1.36 (0.964-1.97)	1.54 (1.04-2.28)	1.79 (1.16-2.74)	1.97 (1.25-3.08)
15-min	0.557 (0.431-0.718)	0.676 (0.523-0.873)	0.871 (0.672-1.13)	1.03 (0.793-1.35)	1.26 (0.934-1.71)	1.43 (1.04-1.99)	1.60 (1.13-2.31)	1.82 (1.22-2.69)	2.10 (1.36-3.22)	2.32 (1.47-3.62)
30-min	0.748 (0.580-0.964)	0.910 (0.705-1.18)	1.18 (0.907-1.52)	1.40 (1.07-1.82)	1.70 (1.26-2.31)	1.93 (1.41-2.69)	2.16 (1.53-3.13)	2.46 (1.65-3.63)	2.84 (1.84-4.36)	3.14 (1.98-4.90)
60-min	0.939 (0.728-1.21)	1.14 (0.886-1.48)	1.48 (1.14-1.92)	1.76 (1.35-2.29)	2.14 (1.59-2.91)	2.43 (1.77-3.39)	2.73 (1.93-3.95)	3.10 (2.08-4.58)	3.59 (2.32-5.49)	3.95 (2.50-6.18)
2-hr	1.21 (0.946-1.56)	1.47 (1.14-1.89)	1.89 (1.47-2.43)	2.24 (1.72-2.90)	2.71 (2.03-3.69)	3.08 (2.27-4.29)	3.45 (2.47-5.00)	3.96 (2.67-5.83)	4.64 (3.01-7.07)	5.15 (3.27-8.00)
3-hr	1.40 (1.09-1.79)	1.69 (1.32-2.17)	2.17 (1.69-2.79)	2.57 (1.99-3.32)	3.11 (2.34-4.22)	3.54 (2.61-4.91)	3.96 (2.85-5.73)	4.57 (3.09-6.70)	5.38 (3.50-8.17)	5.99 (3.81-9.28)
6-hr	1.75 (1.38-2.23)	2.13 (1.67-2.71)	2.74 (2.14-3.50)	3.24 (2.52-4.17)	3.94 (2.98-5.32)	4.48 (3.33-6.20)	5.01 (3.64-7.25)	5.83 (3.95-8.49)	6.90 (4.50-10.4)	7.72 (4.92-11.9)
12-hr	2.14 (1.69-2.71)	2.62 (2.07-3.32)	3.41 (2.68-4.33)	4.06 (3.17-5.18)	4.95 (3.76-6.65)	5.64 (4.21-7.76)	6.33 (4.62-9.10)	7.38 (5.02-10.7)	8.77 (5.74-13.2)	9.83 (6.29-15.0)
24-hr	2.51 (1.99-3.15)	3.11 (2.47-3.91)	4.09 (3.24-5.17)	4.91 (3.86-6.24)	6.03 (4.61-8.08)	6.90 (5.19-9.47)	7.77 (5.71-11.2)	9.15 (6.24-13.2)	11.0 (7.20-16.3)	12.4 (7.93-18.8)
2-day	2.83 (2.26-3.54)	3.56 (2.84-4.45)	4.75 (3.78-5.97)	5.74 (4.54-7.25)	7.11 (5.47-9.49)	8.16 (6.18-11.2)	9.21 (6.84-13.3)	11.0 (7.53-15.8)	13.4 (8.81-19.8)	15.2 (9.78-22.9)
3-day	3.08 (2.47-3.84)	3.88 (3.10-4.84)	5.19 (4.14-6.50)	6.28 (4.97-7.90)	7.77 (6.00-10.4)	8.93 (6.78-12.2)	10.1 (7.52-14.5)	12.1 (8.29-17.3)	14.8 (9.72-21.8)	16.8 (10.8-25.2)
4-day	3.30 (2.65-4.11)	4.15 (3.33-5.17)	5.55 (4.43-6.93)	6.70 (5.33-8.42)	8.30 (6.42-11.0)	9.52 (7.25-13.0)	10.8 (8.03-15.4)	12.9 (8.85-18.3)	15.7 (10.4-23.1)	17.9 (11.5-26.7)
7-day	3.91 (3.15-4.84)	4.87 (3.92-6.03)	6.43 (5.16-8.00)	7.73 (6.16-9.66)	9.51 (7.39-12.6)	10.9 (8.31-14.8)	12.3 (9.17-17.4)	14.6 (10.1-20.7)	17.7 (11.7-25.9)	20.0 (13.0-29.8)
10-day	4.53 (3.66-5.59)	5.54 (4.47-6.85)	7.19 (5.79-8.92)	8.56 (6.85-10.7)	10.5 (8.13-13.7)	11.9 (9.09-16.0)	13.4 (9.97-18.8)	15.7 (10.9-22.2)	18.9 (12.5-27.5)	21.2 (13.8-31.5)
20-day	6.51 (5.30-7.99)	7.59 (6.16-9.32)	9.34 (7.56-11.5)	10.8 (8.68-13.4)	12.8 (9.97-16.6)	14.3 (10.9-19.0)	15.9 (11.8-21.9)	18.1 (12.5-25.2)	20.9 (14.0-30.2)	23.1 (15.0-34.0)
30-day	8.21 (6.70-10.1)	9.31 (7.59-11.4)	11.1 (9.02-13.7)	12.6 (10.2-15.6)	14.7 (11.4-18.8)	16.2 (12.4-21.3)	17.8 (13.1-24.2)	19.8 (13.8-27.5)	22.3 (14.9-32.1)	24.3 (15.8-35.6)
45-day	10.4 (8.47-12.6)	11.5 (9.39-14.0)	13.3 (10.9-16.3)	14.9 (12.0-18.3)	17.0 (13.3-21.6)	18.6 (14.2-24.2)	20.3 (14.8-27.1)	21.9 (15.3-30.3)	24.1 (16.2-34.5)	25.8 (16.8-37.7)
60-day	12.2 (9.97-14.8)	13.3 (10.9-16.2)	15.2 (12.4-18.6)	16.8 (13.6-20.7)	19.0 (14.8-24.1)	20.7 (15.7-26.7)	22.4 (16.4-29.6)	23.8 (16.7-32.8)	25.8 (17.3-36.7)	27.2 (17.8-39.7)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

Back to Top

PDS-based depth-duration-frequency (DDF) curves Latitude: 41.8163°, Longitude: -72.5538°



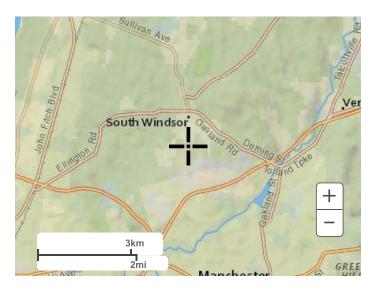
NOAA Atlas 14, Volume 10, Version 2

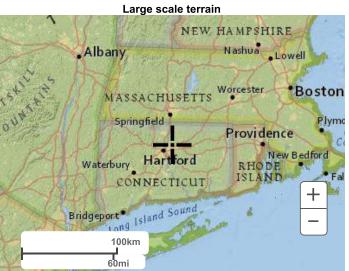
Created (GMT): Tue May 22 13:58:03 2018

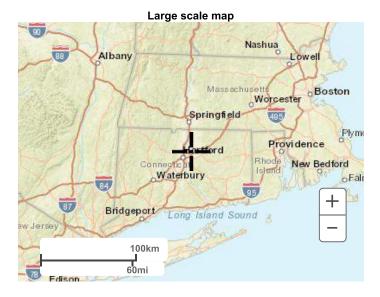
Back to Top

Maps & aerials

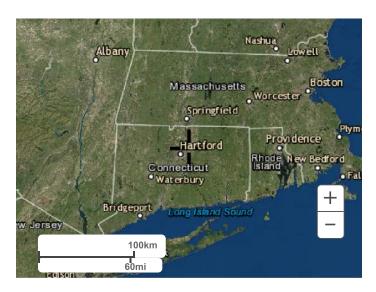
Small scale terrain







Large scale aerial



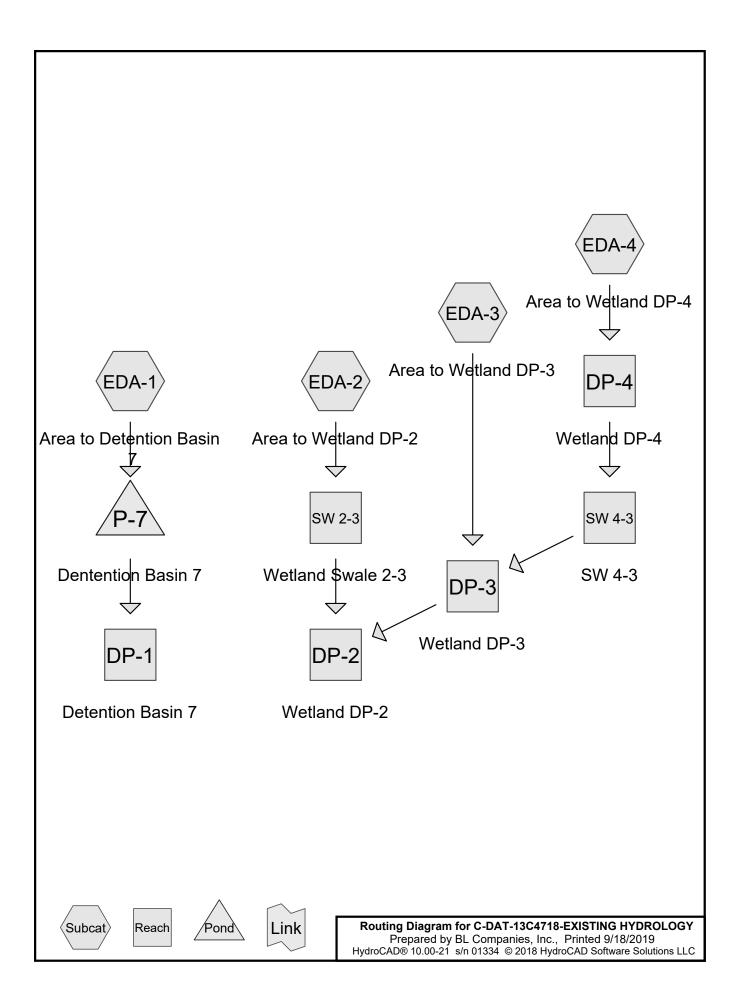
Back to Top

US Department of Commerce
National Oceanic and Atmospheric Administration
National Weather Service
National Water Center
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

Disclaimer



APPENDIX B PRE-DEVELOPMENT HYDROLOGY



C-DAT-13C4718-EXISTING HYD CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 2

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: Area to Detention Runoff Area=840,987 sf 70.56% Impervious Runoff Depth=2.09" Tc=15.0 min CN=90 Runoff=38.21 cfs 3.355 af

SubcatchmentEDA-2: Area to Wetland Runoff Area=747,775 sf 71.15% Impervious Runoff Depth=2.09" Flow Length=1,211' Tc=8.0 min CN=90 Runoff=45.99 cfs 2.983 af

SubcatchmentEDA-3: Area to Wetland DP-3 Runoff Area=45,946 sf 0.00% Impervious Runoff Depth=0.93" Flow Length=347' Tc=9.3 min CN=73 Runoff=1.06 cfs 0.081 af

SubcatchmentEDA-4: Area to Wetland

Runoff Area=119,565 sf 0.00% Impervious Runoff Depth=0.73"

Flow Length=808' Tc=18.1 min CN=69 Runoff=1.42 cfs 0.167 af

Reach DP-1: Detention Basin 7 Inflow=0.69 cfs 1.818 af

Outflow=0.69 cfs 1.818 af

Reach DP-2: Wetland DP-2 Inflow=46.65 cfs 3.231 af

Outflow=46.65 cfs 3.231 af

Reach DP-3: Wetland DP-3 Inflow=1.83 cfs 0.248 af

Outflow=1.83 cfs 0.248 af

Reach DP-4: Wetland DP-4 Inflow=1.42 cfs 0.167 af

Outflow=1.42 cfs 0.167 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=0.56' Max Vel=7.39 fps Inflow=45.99 cfs 2.983 af

 $n = 0.030 \quad L = 396.0' \quad S = 0.0556 \; '/' \quad Capacity = 1,486.07 \; cfs \quad Outflow = 45.33 \; cfs \; \; 2.983 \; afs \; \; 1.000 \; cfs \; \; 1.000 \; cf$

Reach SW 4-3: SW 4-3 Avg. Flow Depth=0.07' Max Vel=1.99 fps Inflow=1.42 cfs 0.167 af

n=0.030 L=343.7' S=0.0541 '/' Capacity=1,466.70 cfs Outflow=1.39 cfs 0.167 af

Pond P-7: Dentention Basin 7 Peak Elev=93.90' Storage=117,300 cf Inflow=38.21 cfs 3.355 af

Outflow=0.69 cfs 1.818 af

Total Runoff Area = 40.273 ac Runoff Volume = 6.587 af Average Runoff Depth = 1.96" 35.84% Pervious = 14.435 ac 64.16% Impervious = 25.837 ac

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 3

Summary for Subcatchment EDA-1: Area to Detention Basin 7

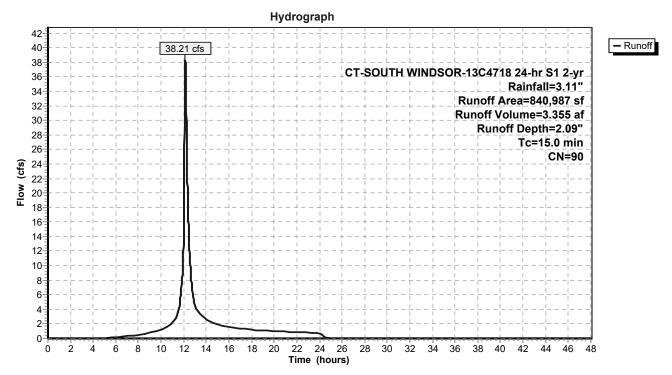
Runoff 38.21 cfs @ 12.15 hrs, Volume= 3.355 af, Depth= 2.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

Ar	rea (sf)	CN	Description						
2	99,131	98	Paved parking, HSG B						
2	82,062	98	Paved park	ing, HSG C					
	11,034	98	Paved park	ing, HSG E	3				
	1,191	98	Paved park	ing, HSG C					
18	80,158	69	50-75% Gra	ass cover, I	Fair, HSG B				
	59,799	79	50-75% Grass cover, Fair, HSG C						
	7,145	69	50-75% Grass cover, Fair, HSG B						
	467	79	50-75% Gra	ass cover, I	Fair, HSG C				
84	40,987	90	Weighted A	verage					
2	47,569		29.44% Per	vious Area	l				
5	93,418		70.56% Imp	ervious Ar	ea				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
15.0					Direct Entry,	Master Planned Tc			

Direct Entry, Master Planned Tc

Subcatchment EDA-1: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 4

Summary for Subcatchment EDA-2: Area to Wetland DP-2

Runoff = 45.99 cfs @ 12.06 hrs, Volume= 2.983 af, Depth= 2.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

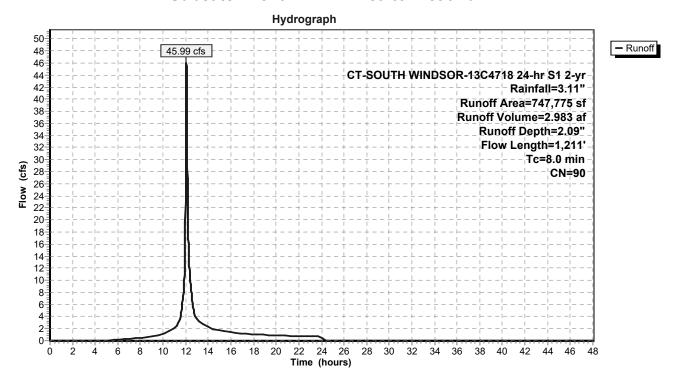
A	rea (sf)	CN E	escription					
5	17,459	98 F	aved park	ing, HSG B				
	10,227	98 F	Paved parking, HSG C					
	4,362	98 F	aved park	ing, HSG D				
2	13,896	69 5	0-75% Gra	ass cover, F	Fair, HSG B			
	588	79 5	0-75% Gra	ass cover, F	Fair, HSG C			
	1,243	84 5	0-75% Gra	ass cover, F	Fair, HSG D			
7	47,775	90 V	Veighted A	verage				
2	15,727	2	8.85% Per	vious Area				
5	32,048	7	1.15% lmp	ervious Ar	ea			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.6	74	0.1350	0.34		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.4	26	0.0250	1.13		Sheet Flow, 2			
					Smooth surfaces n= 0.011 P2= 3.11"			
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3			
					Paved Kv= 20.3 fps			
1.7	744	0.0050	7.35	23.11	Pipe Channel, RCP_Round 24"			
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'			
4.0	4-4		0.40		n= 0.009 Corrugated PE, smooth interior			
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4			
					Grassed Waterway Kv= 15.0 fps			
8.0	1,211	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 5

Subcatchment EDA-2: Area to Wetland DP-2



HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 6

Summary for Subcatchment EDA-3: Area to Wetland DP-3

Runoff = 1.06 cfs @ 12.08 hrs, Volume= 0.081 af, Depth= 0.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

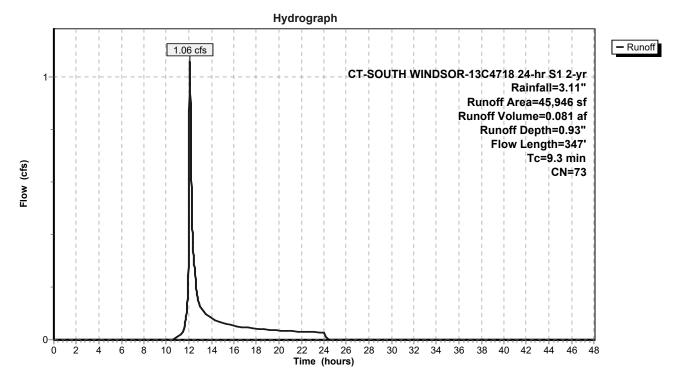
	Α	rea (sf)	CN	N Description							
_		0	98	Paved parking, HSG B							
		0	98	Paved park	ing, HSG C						
		0	98	Paved park	ing, HSG D						
		21,004	69	50-75% Gra	ass cover, I	Fair, HSG B					
		0	79	50-75% Gra	ass cover, I	Fair, HSG C					
		5,451	84								
		2,225	56	Brush, Fair,	HSG B						
_		17,266	77	77 Brush, Fair, HSG D							
		45,946	73	Weighted A	verage						
		45,946		100.00% Pe	ervious Are	ea					
	Тс	Length	Slope			Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	7.5	100	0.0400	0.22		Sheet Flow, 1					
						Grass: Short n= 0.150 P2= 3.11"					
	0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2					
						Grassed Waterway Kv= 15.0 fps					
	1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3					
_						Woodland Kv= 5.0 fps					
	9.3	347	Total								

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 7

Subcatchment EDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 8

Summary for Subcatchment EDA-4: Area to Wetland DP-4

Runoff = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af, Depth= 0.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

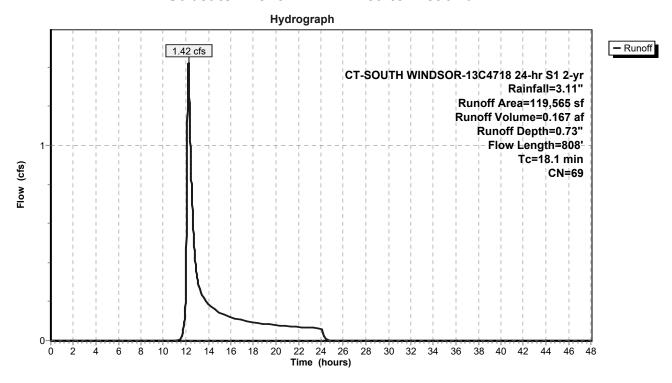
A	rea (sf)	CN D	escription					
	0	98 F	98 Paved parking, HSG B					
	0			ing, HSG C				
	0	98 F	aved park	ing, HSG D				
	57,679	69 5	0-75% Gra	ass cover, l	Fair, HSG B			
	26,837	79 5	0-75% Gra	ass cover, l	Fair, HSG C			
	0	84 5	0-75% Gra	ass cover, I	Fair, HSG D			
	25,526	56 E	rush, Fair,	, HSG B				
	9,523	70 B	rush, Fair,	, HSG C				
1	19,565	69 V	Veighted A	verage				
1	19,565	1	00.00% P	ervious Are	a			
Tc	Length	Slope	Velocity		Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	100	0.0800	0.29		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2			
					Grassed Waterway Kv= 15.0 fps			
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3			
					Woodland Kv= 5.0 fps			
0.1	74	0.0270	19.82	194.19	•			
					Area= 9.8 sf Perim= 15.7' r= 0.62'			
					n= 0.009 Corrugated PE, smooth interior			
18.1	808	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 9

Subcatchment EDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 10

Summary for Reach DP-1: Detention Basin 7

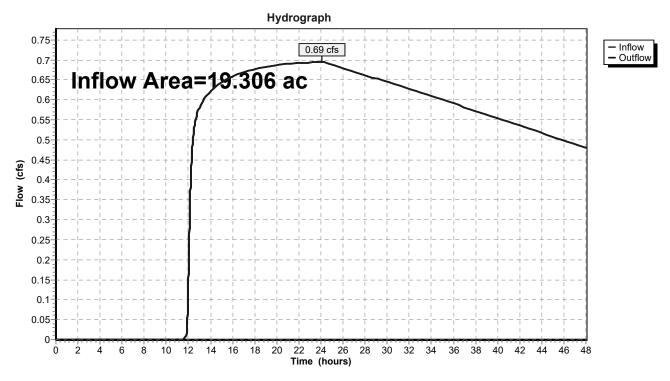
Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth > 1.13" for 2-yr event

Inflow = 0.69 cfs @ 24.10 hrs, Volume= 1.818 af

Outflow = 0.69 cfs @ 24.10 hrs, Volume= 1.818 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 11

Summary for Reach DP-2: Wetland DP-2

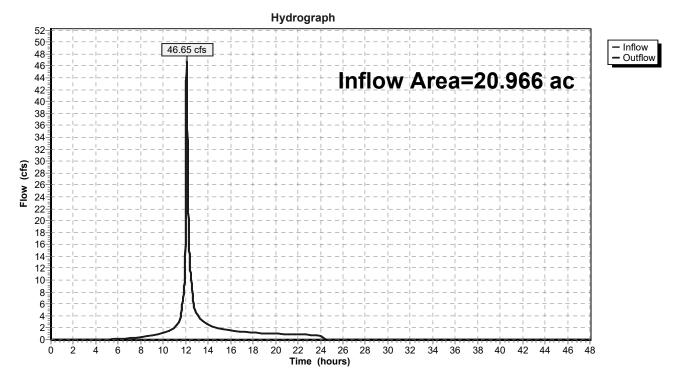
Inflow Area = 20.966 ac, 58.26% Impervious, Inflow Depth = 1.85" for 2-yr event

Inflow = 46.65 cfs @ 12.09 hrs, Volume= 3.231 af

Outflow = 46.65 cfs @ 12.09 hrs, Volume= 3.231 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 12

Summary for Reach DP-3: Wetland DP-3

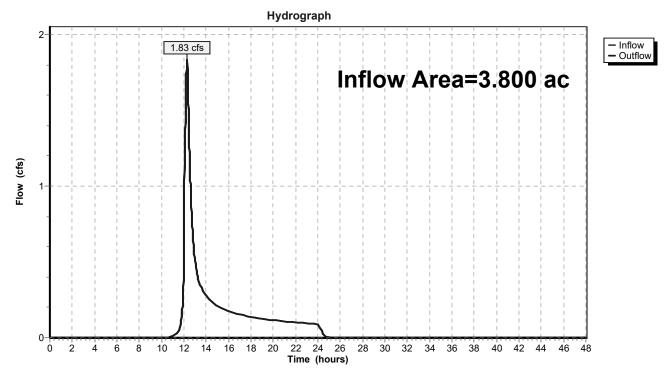
Inflow Area = 3.800 ac, 0.00% Impervious, Inflow Depth = 0.78" for 2-yr event

Inflow = 1.83 cfs @ 12.28 hrs, Volume= 0.248 af

Outflow = 1.83 cfs @ 12.28 hrs, Volume= 0.248 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 13

Summary for Reach DP-4: Wetland DP-4

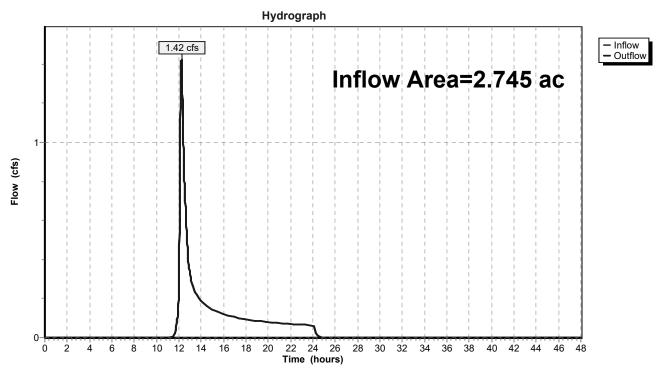
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 0.73" for 2-yr event

Inflow = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af

Outflow = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 14

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 17.167 ac, 71.15% Impervious, Inflow Depth = 2.09" for 2-yr event

Inflow = 45.99 cfs @ 12.06 hrs, Volume= 2.983 af

Outflow = 45.33 cfs @ 12.09 hrs, Volume= 2.983 af, Atten= 1%, Lag= 1.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 7.39 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.93 fps, Avg. Travel Time= 3.4 min

Peak Storage= 2,436 cf @ 12.07 hrs Average Depth at Peak Storage= 0.56' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

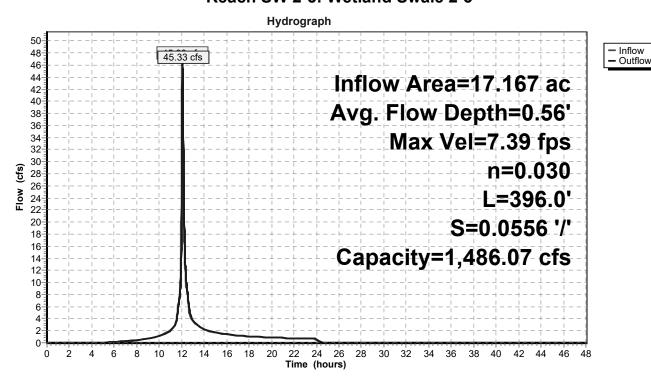
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 15

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 0.73" for 2-yr event

Inflow = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af

Outflow = 1.39 cfs @ 12.31 hrs, Volume= 0.167 af, Atten= 3%, Lag= 5.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.99 fps, Min. Travel Time= 2.9 min Avg. Velocity = 1.36 fps, Avg. Travel Time= 4.2 min

Peak Storage= 240 cf @ 12.26 hrs Average Depth at Peak Storage= 0.07'

Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

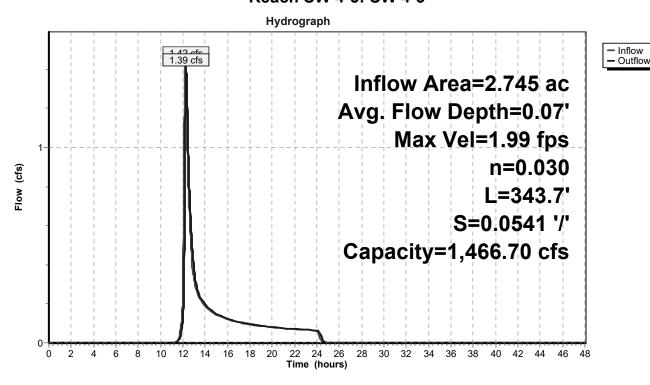
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 16

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth = 2.09" for 2-yr event

Inflow 38.21 cfs @ 12.15 hrs, Volume= 3.355 af

0.69 cfs @ 24.10 hrs, Volume= Outflow 1.818 af, Atten= 98%, Lag= 716.8 min

Primary 0.69 cfs @ 24.10 hrs, Volume= 1.818 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 93.90' @ 24.10 hrs Surf.Area= 37,037 sf Storage= 117,300 cf

Plug-Flow detention time= 1,054.9 min calculated for 1.818 af (54% of inflow)

Center-of-Mass det. time= 922.1 min (1,751.6 - 829.5)

Volume	Inve	ert Avail.Sto	rage Storage	Description	
#1	90.0	0' 396,47	79 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
90.0	00	19,765	0	0	
92.0	92.00 31,993		51,758	51,758	
94.0	00	37,305	69,298	121,056	
96.0	00	42,927	80,232	201,288	
98.0	00	48,699	91,626	292,914	
100.0	00	54,866	103,565	396,479	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	88.00'	18.0" Round	l Culvert	
			L= 71.0' CM	P, square edge	headwall, Ke= 0.500
			Inlet / Outlet I	nvert= 88.00' / 8	34.50' S= 0.0493 '/' Cc= 0.900
				,	ooth interior, Flow Area= 1.77 sf
#2	Device 1	91.00'		ifice/Grate C=	
#3	Device 1	95.00'	6.0" Vert. Or	ifice/Grate C=	0.600

6.0" Vert. Orifice/Grate C= 0.600

Limited to weir flow at low heads

36.0" x 78.0" Horiz. Orifice/Grate C= 0.600

Primary OutFlow Max=0.69 cfs @ 24.10 hrs HW=93.90' (Free Discharge)

-1=Culvert (Passes 0.69 cfs of 19.31 cfs potential flow)

98.00'

99.00'

2=Orifice/Grate (Orifice Controls 0.69 cfs @ 7.96 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

#4

#5

Device 1

Device 1

-4=Orifice/Grate (Controls 0.00 cfs)

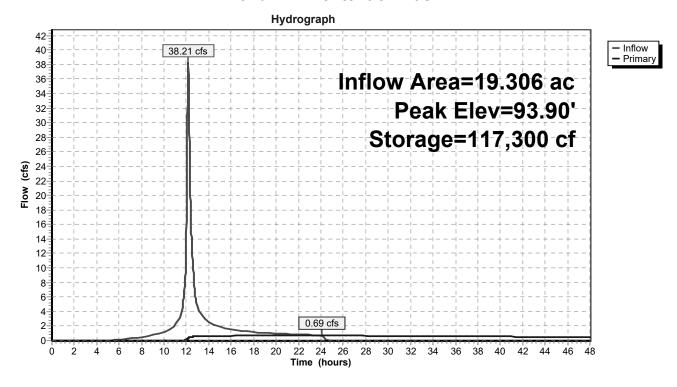
-5=Orifice/Grate (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 17

Pond P-7: Dentention Basin 7



C-DAT-13C4718-EXISTING HY CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 18

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: Area to Detention Runoff Area=840,987 sf 70.56% Impervious Runoff Depth=3.79" Tc=15.0 min CN=90 Runoff=66.14 cfs 6.097 af

SubcatchmentEDA-2: Area to Wetland Runoff Area=747,775 sf 71.15% Impervious Runoff Depth=3.79" Flow Length=1,211' Tc=8.0 min CN=90 Runoff=79.37 cfs 5.421 af

SubcatchmentEDA-3: Area to Wetland DP-3 Runoff Area=45,946 sf 0.00% Impervious Runoff Depth=2.21" Flow Length=347' Tc=9.3 min CN=73 Runoff=2.67 cfs 0.194 af

SubcatchmentEDA-4: Area to Wetland

Runoff Area=119,565 sf 0.00% Impervious Runoff Depth=1.89"

Flow Length=808' Tc=18.1 min CN=69 Runoff=4.16 cfs 0.433 af

Reach DP-1: Detention Basin 7 Inflow=1.72 cfs 3.394 af

Outflow=1.72 cfs 3.394 af

Reach DP-2: Wetland DP-2 Inflow=82.76 cfs 6.048 af

Outflow=82.76 cfs 6.048 af

Reach DP-3: Wetland DP-3 Inflow=5.39 cfs 0.627 af

Outflow=5.39 cfs 0.627 af

Reach DP-4: Wetland DP-4 Inflow=4.16 cfs 0.433 af

Outflow=4.16 cfs 0.433 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=0.78' Max Vel=8.96 fps Inflow=79.37 cfs 5.421 af

 $n = 0.030 \quad L = 396.0' \quad S = 0.0556 \; '/' \quad Capacity = 1,486.07 \; cfs \quad Outflow = 78.58 \; cfs \; \; 5.421 \; aff \; (1.000 \pm 1.000 \pm 1.0$

Reach SW 4-3: SW 4-3 Avg. Flow Depth=0.13' Max Vel=3.00 fps Inflow=4.16 cfs 0.433 af

n=0.030 L=343.7' S=0.0541'/ Capacity=1,466.70 cfs Outflow=4.12 cfs 0.433 af

Pond P-7: Dentention Basin 7 Peak Elev=95.97' Storage=199,916 cf Inflow=66.14 cfs 6.097 af

Outflow=1.72 cfs 3.394 af

Total Runoff Area = 40.273 ac Runoff Volume = 12.145 af Average Runoff Depth = 3.62" 35.84% Pervious = 14.435 ac 64.16% Impervious = 25.837 ac

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 19

Summary for Subcatchment EDA-1: Area to Detention Basin 7

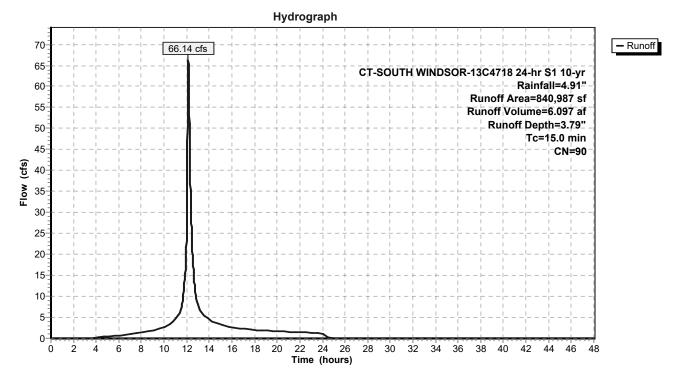
Runoff = 66.14 cfs @ 12.15 hrs, Volume= 6.097 af, Depth= 3.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

A	rea (sf)	CN	Description						
2	299,131	98	Paved parking, HSG B						
2	282,062	98	Paved park	ing, HSG C)				
	11,034	98	Paved park	ing, HSG B	3				
	1,191	98	Paved park	ing, HSG C)				
1	80,158	69	50-75% Gra	ass cover, l	Fair, HSG B				
	59,799	79	50-75% Grass cover, Fair, HSG C						
	7,145	69	50-75% Grass cover, Fair, HSG B						
	467	79	50-75% Gra	ass cover, I	Fair, HSG C				
8	340,987	90	Weighted A	verage					
2	247,569		29.44% Per	vious Area	1				
5	93,418		70.56% Imp	ervious Ar	ea				
Tc	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
15.0					Direct Entry,	Master Planned Tc			

• .

Subcatchment EDA-1: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 20

Summary for Subcatchment EDA-2: Area to Wetland DP-2

Runoff = 79.37 cfs @ 12.06 hrs, Volume= 5.421 af, Depth= 3.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

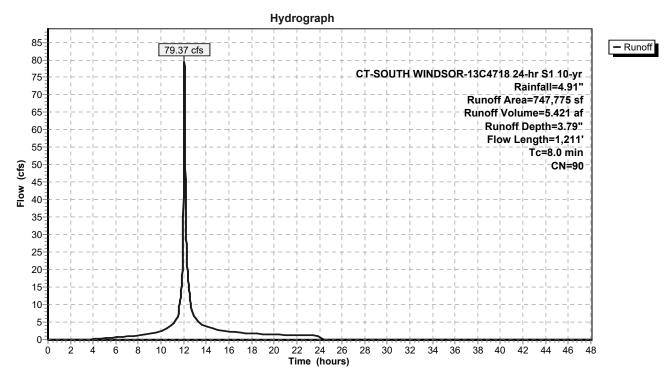
A	rea (sf)	CN E	Description					
5	17,459	98 F	Paved parking, HSG B					
	10,227	98 F	Paved parking, HSG C					
	4,362	98 F	Paved park	ing, HSG D				
2	13,896	69 5	0-75% Gra	ass cover, F	Fair, HSG B			
	588	79 5	0-75% Gra	ass cover, F	Fair, HSG C			
	1,243	84 5	0-75% Gra	ass cover, F	Fair, HSG D			
7	47,775	90 V	Veighted A	verage				
2	15,727	2	8.85% Per	vious Area				
5	32,048	7	'1.15% lmp	ervious Ar	ea			
Tc	Length	Slope	Velocity		Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.6	74	0.1350	0.34		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.4	26	0.0250	1.13		Sheet Flow, 2			
					Smooth surfaces n= 0.011 P2= 3.11"			
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3			
					Paved Kv= 20.3 fps			
1.7	744	0.0050	7.35	23.11	Pipe Channel, RCP_Round 24"			
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'			
4.0	454	0.0000	0.40		n= 0.009 Corrugated PE, smooth interior			
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4			
	4.04.				Grassed Waterway Kv= 15.0 fps			
8.0	1,211	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 21

Subcatchment EDA-2: Area to Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 22

Summary for Subcatchment EDA-3: Area to Wetland DP-3

Runoff = 2.67 cfs @ 12.08 hrs, Volume= 0.194 af, Depth= 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

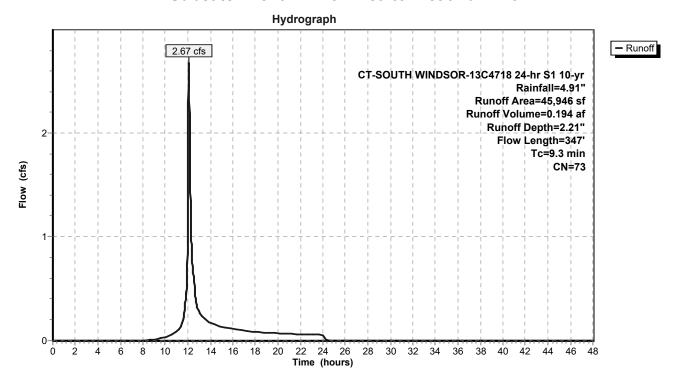
	Α	rea (sf)	CN	Description								
		0	98	Paved parking, HSG B								
		0	98	Paved parking, HSG C								
		0	98	Paved park	Paved parking, HSG D							
		21,004	69	50-75% Gra	ass cover, I	Fair, HSG B						
		0	79	50-75% Gra	ass cover, I	Fair, HSG C						
		5,451			•	Fair, HSG D						
		2,225		Brush, Fair,								
		17,266 77 Brush, Fair, HSG D										
		45,946		73 Weighted Average								
		45,946		100.00% Pe	ervious Are	ea						
	Тс	Length	Slope	•	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	7.5	100	0.0400	0.22		Sheet Flow, 1						
						Grass: Short n= 0.150 P2= 3.11"						
	0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2						
						Grassed Waterway Kv= 15.0 fps						
	1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3						
_						Woodland Kv= 5.0 fps						
	9.3	347	Total									

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 23

Subcatchment EDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 24

Summary for Subcatchment EDA-4: Area to Wetland DP-4

Runoff = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af, Depth= 1.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

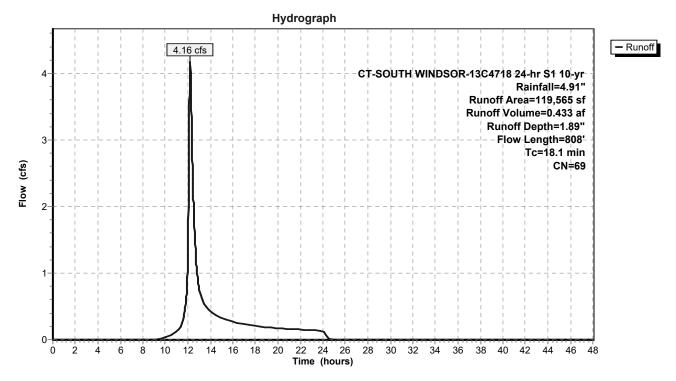
A	rea (sf)	CN D	escription					
	0	98 F	aved park	ing, HSG B	3			
	0			ing, HSG C				
	0	98 F	aved park	ing, HSG D)			
	57,679	69 5	0-75% Gra	ass cover, l	Fair, HSG B			
	26,837	79 5	0-75% Gra	ass cover, l	Fair, HSG C			
	0	84 5	50-75% Grass cover, Fair, HSG D					
	25,526	56 B	Brush, Fair, HSG B					
	9,523	70 E	rush, Fair,	, HSG C				
1	19,565	69 V	Veighted A	verage				
1	119,565			100.00% Pervious Area				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	100	0.0800	0.29		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2			
					Grassed Waterway Kv= 15.0 fps			
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3			
					Woodland Kv= 5.0 fps			
0.1	74	0.0270	19.82	194.19	Channel Flow, 4			
					Area= 9.8 sf Perim= 15.7' r= 0.62'			
					n= 0.009 Corrugated PE, smooth interior			
18.1	808	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 25

Subcatchment EDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 26

Summary for Reach DP-1: Detention Basin 7

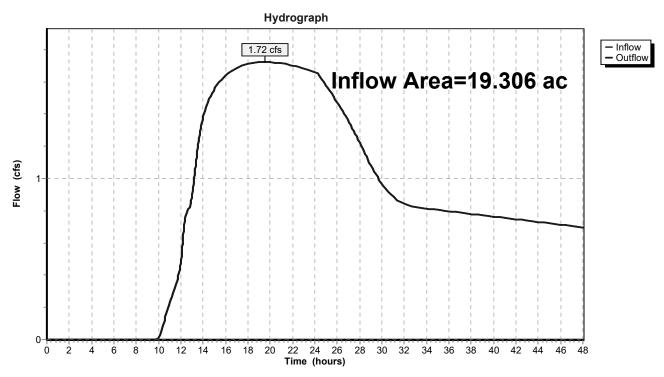
Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth > 2.11" for 10-yr event

Inflow = 1.72 cfs @ 19.53 hrs, Volume= 3.394 af

Outflow = 1.72 cfs @ 19.53 hrs, Volume= 3.394 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 27

Summary for Reach DP-2: Wetland DP-2

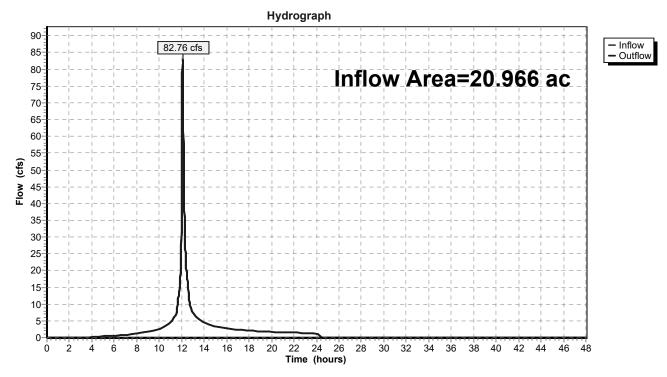
Inflow Area = 20.966 ac, 58.26% Impervious, Inflow Depth = 3.46" for 10-yr event

Inflow = 82.76 cfs @ 12.08 hrs, Volume= 6.048 af

Outflow = 82.76 cfs @ 12.08 hrs, Volume= 6.048 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 28

Summary for Reach DP-3: Wetland DP-3

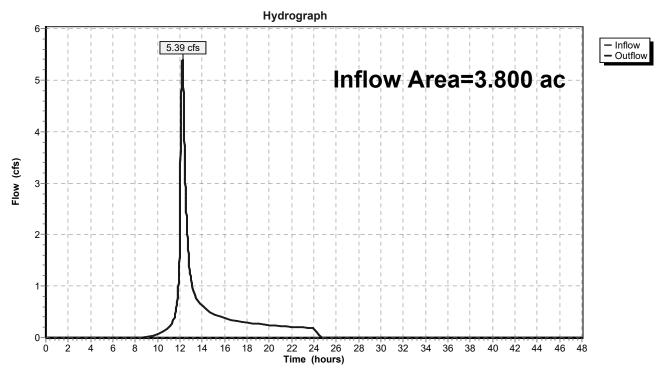
Inflow Area = 3.800 ac, 0.00% Impervious, Inflow Depth = 1.98" for 10-yr event

Inflow = 5.39 cfs @ 12.22 hrs, Volume= 0.627 af

Outflow = 5.39 cfs @ 12.22 hrs, Volume= 0.627 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 29

Summary for Reach DP-4: Wetland DP-4

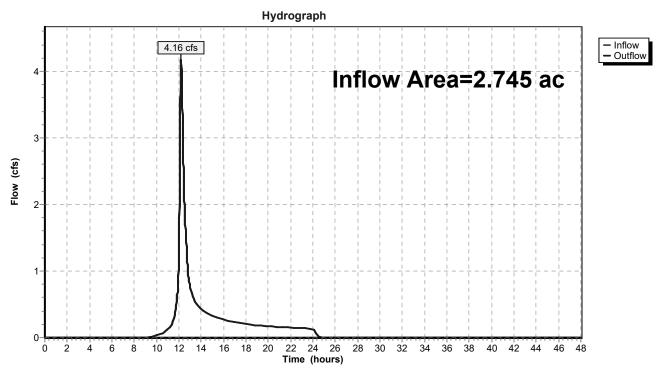
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 1.89" for 10-yr event

Inflow = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af

Outflow = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 30

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 17.167 ac, 71.15% Impervious, Inflow Depth = 3.79" for 10-yr event

Inflow = 79.37 cfs @ 12.06 hrs, Volume= 5.421 af

Outflow = 78.58 cfs @ 12.08 hrs, Volume= 5.421 af, Atten= 1%, Lag= 1.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 8.96 fps, Min. Travel Time= 0.7 min Avg. Velocity = 2.30 fps, Avg. Travel Time= 2.9 min

Peak Storage= 3,478 cf @ 12.07 hrs
Average Depth at Peak Storage= 0.78'

Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

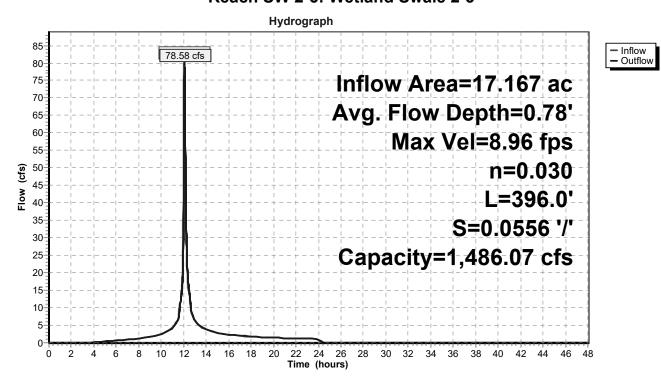
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 31

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 1.89" for 10-yr event

Inflow = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af

Outflow = 4.12 cfs @ 12.26 hrs, Volume= 0.433 af, Atten= 1%, Lag= 3.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.00 fps, Min. Travel Time= 1.9 min Avg. Velocity = 1.40 fps, Avg. Travel Time= 4.1 min

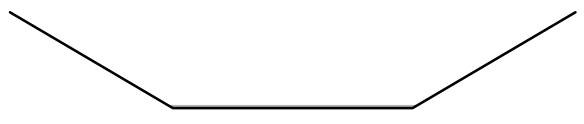
Peak Storage= 473 cf @ 12.23 hrs Average Depth at Peak Storage= 0.13' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

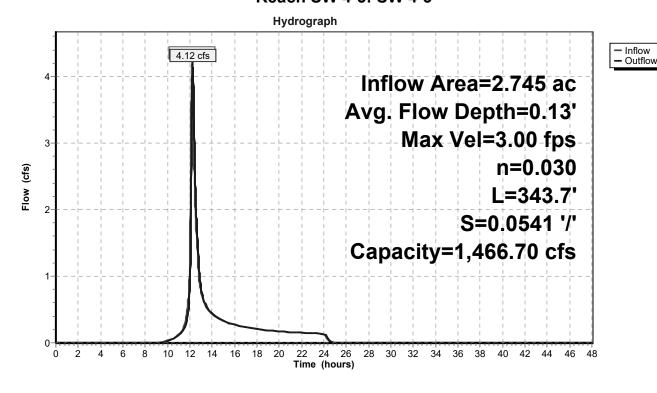
Side Slope Z-value = 1.7 '/' Top Width = 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 32

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth = 3.79" for 10-yr event

Inflow = 66.14 cfs @ 12.15 hrs, Volume= 6.097 af

Outflow = 1.72 cfs @ 19.53 hrs, Volume= 3.394 af, Atten= 97%, Lag= 442.8 min

Primary = 1.72 cfs @ 19.53 hrs, Volume= 3.394 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 95.97' @ 19.53 hrs Surf.Area= 42,837 sf Storage= 199,916 cf

Plug-Flow detention time= 937.0 min calculated for 3.394 af (56% of inflow)

Center-of-Mass det. time= 803.3 min (1,613.0 - 809.8)

Volume	Inve	ert Avail.Sto	rage Storage	Description	
#1	90.0	00' 396,47	79 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
90.0	00	19,765	0	0	
92.0	00	31,993	51,758	51,758	
94.0	00	37,305	69,298	121,056	
96.0	00	42,927	80,232	201,288	
98.0	00	48,699	91,626	292,914	
100.0	00	54,866	103,565	396,479	
Device	Routing	Invert	Outlet Device	s	
#1	Primary	88.00'	18.0" Round	l Culvert	
			L= 71.0' CM	P, square edge	headwall, Ke= 0.500
					34.50' S= 0.0493 '/' Cc= 0.900
				,	ooth interior, Flow Area= 1.77 sf
#2	Device 1			ifice/Grate C=	
#3	Device 1	95.00'	6.0" Vert. Ori	ifice/Grate C=	0.600

6.0" Vert. Orifice/Grate C= 0.600

Limited to weir flow at low heads

36.0" x 78.0" Horiz. Orifice/Grate C= 0.600

Primary OutFlow Max=1.72 cfs @ 19.53 hrs HW=95.97' (Free Discharge)

1=Culvert (Passes 1.72 cfs of 22.86 cfs potential flow)

98.00'

99.00'

2=Orifice/Grate (Orifice Controls 0.92 cfs @ 10.55 fps)

-3=Orifice/Grate (Orifice Controls 0.80 cfs @ 4.08 fps)

-4=Orifice/Grate (Controls 0.00 cfs)
-5=Orifice/Grate (Controls 0.00 cfs)

#4

#5

Device 1

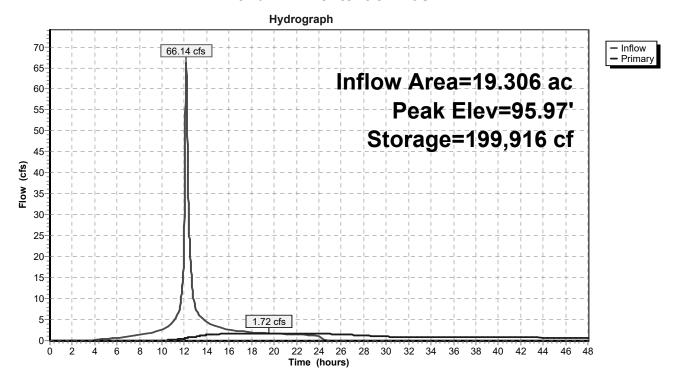
Device 1

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 33

Pond P-7: Dentention Basin 7



C-DAT-13C4718-EXISTING HY CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 34

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: Area to Detention Runoff Area=840,987 sf 70.56% Impervious Runoff Depth=4.88" Tc=15.0 min CN=90 Runoff=83.26 cfs 7.843 af

SubcatchmentEDA-2: Area to Wetland Runoff Area=747,775 sf 71.15% Impervious Runoff Depth=4.88" Flow Length=1,211' Tc=8.0 min CN=90 Runoff=99.88 cfs 6.974 af

SubcatchmentEDA-3: Area to Wetland DP-3 Runoff Area=45,946 sf 0.00% Impervious Runoff Depth=3.11" Flow Length=347' Tc=9.3 min CN=73 Runoff=3.78 cfs 0.274 af

SubcatchmentEDA-4: Area to Wetland

Runoff Area=119,565 sf 0.00% Impervious Runoff Depth=2.74"

Flow Length=808' Tc=18.1 min CN=69 Runoff=6.10 cfs 0.626 af

Reach DP-1: Detention Basin 7 Inflow=2.31 cfs 4.742 af

Outflow=2.31 cfs 4.742 af

Reach DP-2: Wetland DP-2 Inflow=105.35 cfs 7.874 af

Outflow=105.35 cfs 7.874 af

Reach DP-3: Wetland DP-3 Inflow=7.91 cfs 0.899 af

Outflow=7.91 cfs 0.899 af

Reach DP-4: Wetland DP-4 Inflow=6.10 cfs 0.626 af

Outflow=6.10 cfs 0.626 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=0.89' Max Vel=9.70 fps Inflow=99.88 cfs 6.974 af

 $n = 0.030 \quad L = 396.0' \quad S = 0.0556 \; '/' \quad Capacity = 1,486.07 \; cfs \quad Outflow = 99.04 \; cfs \; \; 6.974 \; afs \; \; 1.000 \; cfs \; \; 1.000 \; cf$

Reach SW 4-3: SW 4-3Avg. Flow Depth=0.17' Max Vel=3.47 fps Inflow=6.10 cfs 0.626 af

n=0.030 L=343.7' S=0.0541 '/' Capacity=1,466.70 cfs Outflow=6.06 cfs 0.626 af

Pond P-7: Dentention Basin 7 Peak Elev=97.11' Storage=250,580 cf Inflow=83.26 cfs 7.843 af

Outflow=2.31 cfs 4.742 af

Total Runoff Area = 40.273 ac Runoff Volume = 15.717 af Average Runoff Depth = 4.68" 35.84% Pervious = 14.435 ac 64.16% Impervious = 25.837 ac

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 35

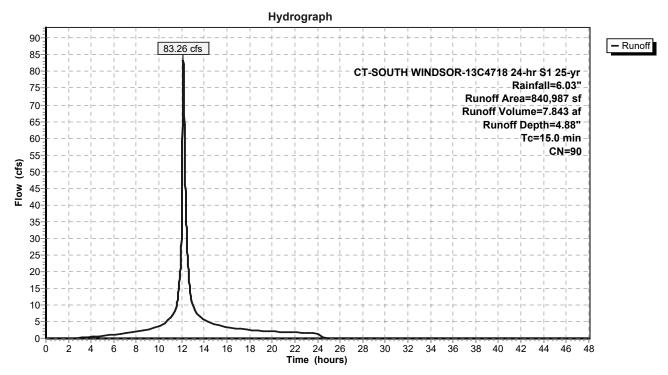
Summary for Subcatchment EDA-1: Area to Detention Basin 7

Runoff 83.26 cfs @ 12.15 hrs, Volume= 7.843 af, Depth= 4.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

A	rea (sf)	CN	Description							
2	299,131	98	98 Paved parking, HSG B							
2	282,062	98	Paved parking, HSG C							
	11,034	98	Paved parking, HSG B							
	1,191	98	Paved parking, HSG C							
1	80,158	69	·							
	59,799 79 50-75% Grass cover, Fair, HSG C									
	7,145 69 50-75% Grass cover, Fair, HSG B									
	467	79	50-75% Gra	ass cover, l	Fair, HSG C					
8	340,987	90	Weighted A	verage						
2	247,569		29.44% Per	vious Area	l					
5	93,418		70.56% Imp	ervious Ar	ea					
Tc	Length	Slop	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)						
15.0					Direct Entry, N	laster Planned Tc				

Subcatchment EDA-1: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 36

Summary for Subcatchment EDA-2: Area to Wetland DP-2

Runoff = 99.88 cfs @ 12.06 hrs, Volume= 6.974 af, Depth= 4.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

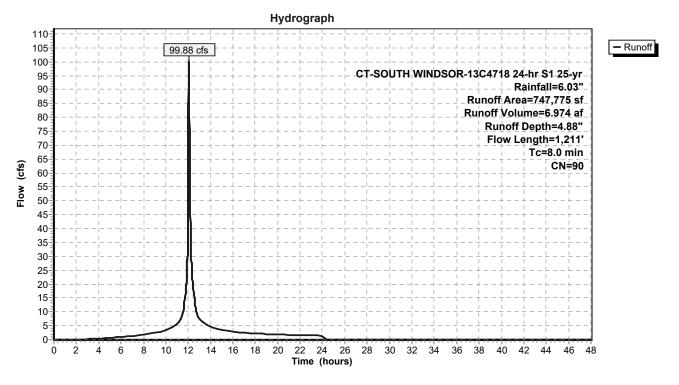
A	rea (sf)	CN E	escription			
5	17,459	98 F	aved park	ing, HSG B	}	
	10,227	98 F	aved park	ing, HSG C		
	4,362	98 F	aved park	ing, HSG D		
2	13,896	69 5	0-75% Gra	ass cover, F	Fair, HSG B	
588 79 50-75% Grass cover, Fair, HSG C						
	1,243	84 5	0-75% Gra	ass cover, F	Fair, HSG D	
7	47,775	90 V	Veighted A	verage		
2	15,727	2	8.85% Per	vious Area		
5	32,048	7	1.15% lmp	ervious Ar	ea	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
3.6	74	0.1350	0.34		Sheet Flow, 1	
					Grass: Short n= 0.150 P2= 3.11"	
0.4	26	0.0250	1.13		Sheet Flow, 2	
					Smooth surfaces n= 0.011 P2= 3.11"	
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3	
					Paved Kv= 20.3 fps	
1.7	744	0.0050	7.35	23.11	Pipe Channel, RCP_Round 24"	
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'	
4.0	4-4		0.40		n= 0.009 Corrugated PE, smooth interior	
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4	
					Grassed Waterway Kv= 15.0 fps	
8.0	1,211	Total				

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 37

Subcatchment EDA-2: Area to Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 38

Summary for Subcatchment EDA-3: Area to Wetland DP-3

Runoff = 3.78 cfs @ 12.08 hrs, Volume= 0.274 af, Depth= 3.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

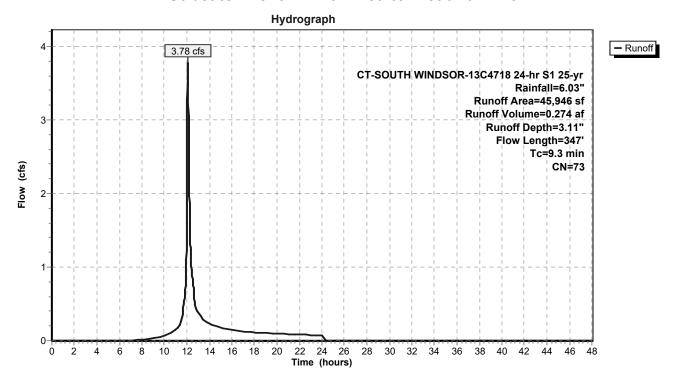
	Α	rea (sf)	CN	Description								
		0	98	Paved parking, HSG B								
		0	98	Paved parking, HSG C								
		0	98	Paved park	Paved parking, HSG D							
		21,004	69	50-75% Gra	ass cover, I	Fair, HSG B						
		0	79	50-75% Gra	ass cover, I	Fair, HSG C						
		5,451			•	Fair, HSG D						
		2,225		Brush, Fair,								
		17,266 77 Brush, Fair, HSG D										
		45,946		73 Weighted Average								
		45,946		100.00% Pe	ervious Are	ea						
	Тс	Length	Slope	•	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	7.5	100	0.0400	0.22		Sheet Flow, 1						
						Grass: Short n= 0.150 P2= 3.11"						
	0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2						
						Grassed Waterway Kv= 15.0 fps						
	1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3						
_						Woodland Kv= 5.0 fps						
	9.3	347	Total									

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 39

Subcatchment EDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 40

Summary for Subcatchment EDA-4: Area to Wetland DP-4

Runoff = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

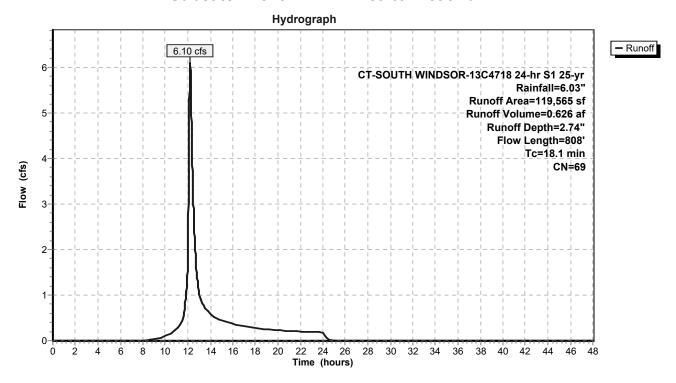
A	rea (sf)	CN D	escription					
	0	98 F	aved park	ing, HSG B	3			
	0			ing, HSG C				
	0	98 F	aved park	ing, HSG D)			
	57,679	69 5	0-75% Gra	ass cover, l	Fair, HSG B			
	26,837	79 5	0-75% Gra	ass cover, l	Fair, HSG C			
	0	84 5	50-75% Grass cover, Fair, HSG D					
	25,526	56 B	Brush, Fair, HSG B					
	9,523	70 E	rush, Fair,	, HSG C				
1	19,565	69 V	Veighted A	verage				
1	119,565			100.00% Pervious Area				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	100	0.0800	0.29		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2			
					Grassed Waterway Kv= 15.0 fps			
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3			
					Woodland Kv= 5.0 fps			
0.1	74	0.0270	19.82	194.19	Channel Flow, 4			
					Area= 9.8 sf Perim= 15.7' r= 0.62'			
					n= 0.009 Corrugated PE, smooth interior			
18.1	808	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 41

Subcatchment EDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 42

Summary for Reach DP-1: Detention Basin 7

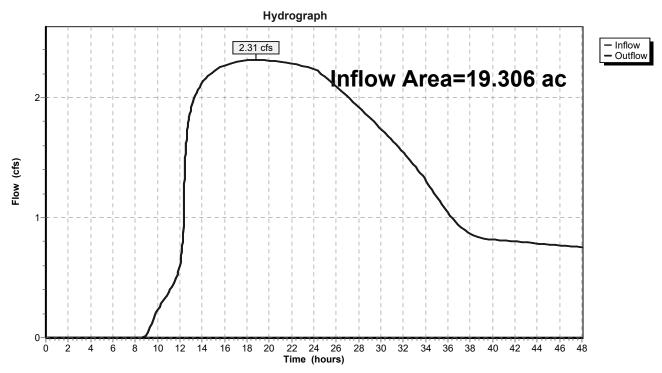
Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth > 2.95" for 25-yr event

Inflow = 2.31 cfs @ 18.82 hrs, Volume= 4.742 af

Outflow = 2.31 cfs @ 18.82 hrs, Volume= 4.742 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 43

Summary for Reach DP-2: Wetland DP-2

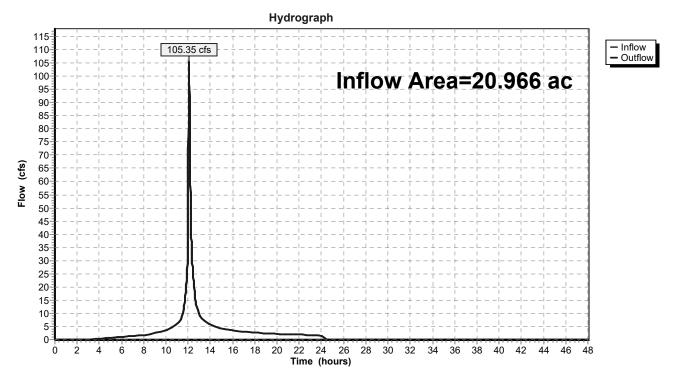
Inflow Area = 20.966 ac, 58.26% Impervious, Inflow Depth = 4.51" for 25-yr event

Inflow = 105.35 cfs @ 12.08 hrs, Volume= 7.874 af

Outflow = 105.35 cfs @ 12.08 hrs, Volume= 7.874 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

<u>Page 44</u>

Summary for Reach DP-3: Wetland DP-3

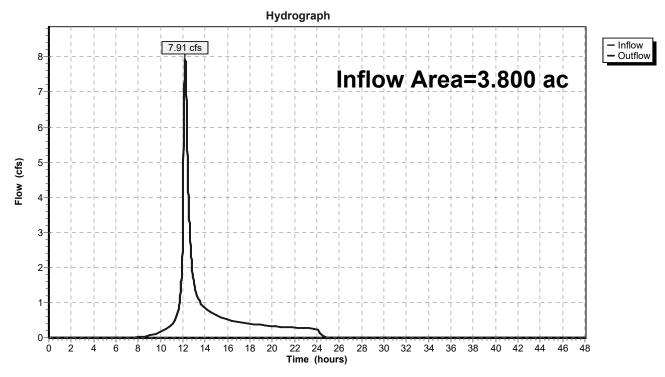
Inflow Area = 3.800 ac, 0.00% Impervious, Inflow Depth = 2.84" for 25-yr event

Inflow = 7.91 cfs @ 12.21 hrs, Volume= 0.899 af

Outflow = 7.91 cfs @ 12.21 hrs, Volume= 0.899 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 45

Summary for Reach DP-4: Wetland DP-4

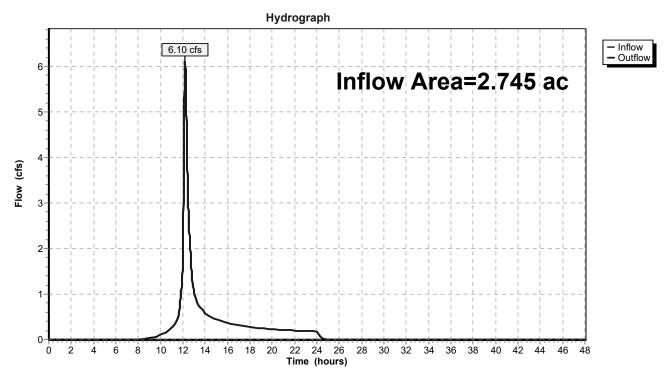
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 2.74" for 25-yr event

Inflow = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af

Outflow = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 46

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 17.167 ac, 71.15% Impervious, Inflow Depth = 4.88" for 25-yr event

Inflow = 99.88 cfs @ 12.06 hrs, Volume= 6.974 af

Outflow = 99.04 cfs @ 12.08 hrs, Volume= 6.974 af, Atten= 1%, Lag= 1.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 9.70 fps, Min. Travel Time= 0.7 min Avg. Velocity = 2.49 fps, Avg. Travel Time= 2.6 min

Peak Storage= 4,049 cf @ 12.07 hrs Average Depth at Peak Storage= 0.89' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

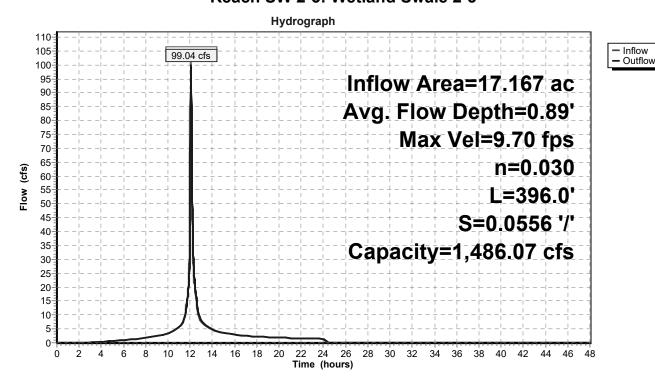
10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 1.7 $^{\prime\prime}$ Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 47

Inflow

Outflow

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 2.74" for 25-yr event

Inflow = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af

Outflow = 6.06 cfs @ 12.25 hrs, Volume= 0.626 af, Atten= 1%, Lag= 2.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.47 fps, Min. Travel Time= 1.7 min Avg. Velocity = 1.44 fps, Avg. Travel Time= 4.0 min

Peak Storage= 601 cf @ 12.22 hrs Average Depth at Peak Storage= 0.17' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

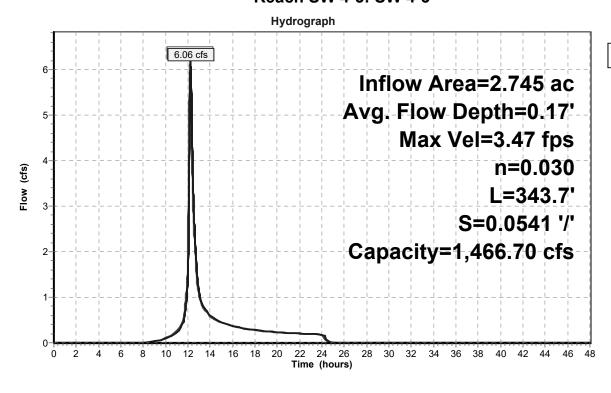
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 48

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth = 4.88" for 25-yr event

Inflow 83.26 cfs @ 12.15 hrs, Volume= 7.843 af

2.31 cfs @ 18.82 hrs, Volume= Outflow 4.742 af, Atten= 97%, Lag= 399.9 min

Primary 2.31 cfs @ 18.82 hrs, Volume= 4.742 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 97.11' @ 18.82 hrs Surf.Area= 46,122 sf Storage= 250,580 cf

Plug-Flow detention time= 894.3 min calculated for 4.741 af (60% of inflow)

Center-of-Mass det. time= 765.9 min (1,567.4 - 801.6)

Volume	Inve	ert Avail.Sto	rage Storage	Description	
#1	90.0	00' 396,47	79 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
90.0	00	19,765	0	0	
92.0	00	31,993	51,758	51,758	
94.0	00	37,305	69,298	121,056	
96.0	00	42,927	80,232	201,288	
98.0	00	48,699	91,626	292,914	
100.0	00	54,866	103,565	396,479	
Device	Routing	Invert	Outlet Device	s	
#1	Primary	88.00'	18.0" Round	l Culvert	
			L= 71.0' CM	P, square edge	headwall, Ke= 0.500
					34.50' S= 0.0493 '/' Cc= 0.900
				,	ooth interior, Flow Area= 1.77 sf
#2	Device 1			ifice/Grate C=	
#3	Device 1	95.00'	6.0" Vert. Ori	ifice/Grate C=	0.600

6.0" Vert. Orifice/Grate C= 0.600

Limited to weir flow at low heads

36.0" x 78.0" Horiz. Orifice/Grate C= 0.600

Primary OutFlow Max=2.31 cfs @ 18.82 hrs HW=97.11' (Free Discharge)

-1=Culvert (Passes 2.31 cfs of 24.60 cfs potential flow)

98.00'

99.00'

#4

#5

Device 1

Device 1

²⁼Orifice/Grate (Orifice Controls 1.02 cfs @ 11.74 fps)

⁻³⁼Orifice/Grate (Orifice Controls 1.29 cfs @ 6.56 fps)

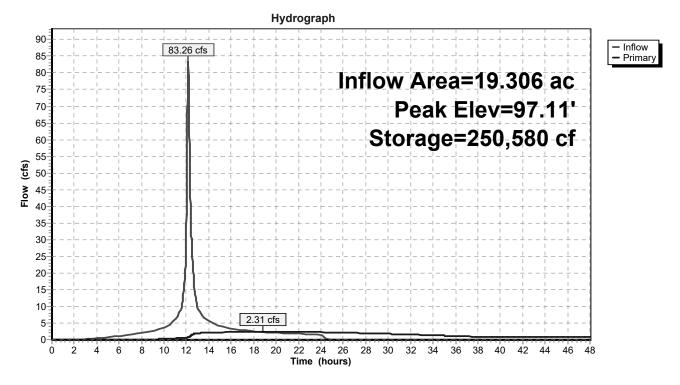
⁻⁴⁼Orifice/Grate (Controls 0.00 cfs) -5=Orifice/Grate (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 49

Pond P-7: Dentention Basin 7



C-DAT-13C4718-EXISTING HYCT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 50

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: Area to Detention Runoff Area=840,987 sf 70.56% Impervious Runoff Depth=6.58" Tc=15.0 min CN=90 Runoff=109.41 cfs 10.585 af

SubcatchmentEDA-2: Area to Wetland Runoff Area=747,775 sf 71.15% Impervious Runoff Depth=6.58" Flow Length=1,211' Tc=8.0 min CN=90 Runoff=131.15 cfs 9.412 af

SubcatchmentEDA-3: Area to Wetland DP-3 Runoff Area=45,946 sf 0.00% Impervious Runoff Depth=4.61" Flow Length=347' Tc=9.3 min CN=73 Runoff=5.55 cfs 0.405 af

SubcatchmentEDA-4: Area to Wetland

Runoff Area=119,565 sf 0.00% Impervious Runoff Depth=4.15"

Flow Length=808' Tc=18.1 min CN=69 Runoff=9.28 cfs 0.950 af

Reach DP-1: Detention Basin 7 Inflow=3.59 cfs 7.003 af

Outflow=3.59 cfs 7.003 af

Reach DP-2: Wetland DP-2 Inflow=140.06 cfs 10.767 af

Outflow=140.06 cfs 10.767 af

Reach DP-3: Wetland DP-3 Inflow=12.06 cfs 1.355 af

Outflow=12.06 cfs 1.355 af

Reach DP-4: Wetland DP-4 Inflow=9.28 cfs 0.950 af

Outflow=9.28 cfs 0.950 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=1.04' Max Vel=10.62 fps Inflow=131.15 cfs 9.412 af

 $n = 0.030 \quad L = 396.0' \quad S = 0.0556 \; \text{'/'} \quad Capacity = 1,486.07 \; \text{cfs} \quad Outflow = 130.29 \; \text{cfs} \quad 9.412 \; \text{af} \quad 10.030 \; \text{cfs} \quad 10.030 \; \text{cfs$

Reach SW 4-3: SW 4-3Avg. Flow Depth=0.22' Max Vel=4.07 fps Inflow=9.28 cfs 0.950 af

n=0.030 L=343.7' S=0.0541 '/' Capacity=1,466.70 cfs Outflow=9.23 cfs 0.950 af

Pond P-7: Dentention Basin 7 Peak Elev=98.74' Storage=329,940 cf Inflow=109.41 cfs 10.585 af

Outflow=3.59 cfs 7.003 af

Total Runoff Area = 40.273 ac Runoff Volume = 21.352 af Average Runoff Depth = 6.36" 35.84% Pervious = 14.435 ac 64.16% Impervious = 25.837 ac

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 51

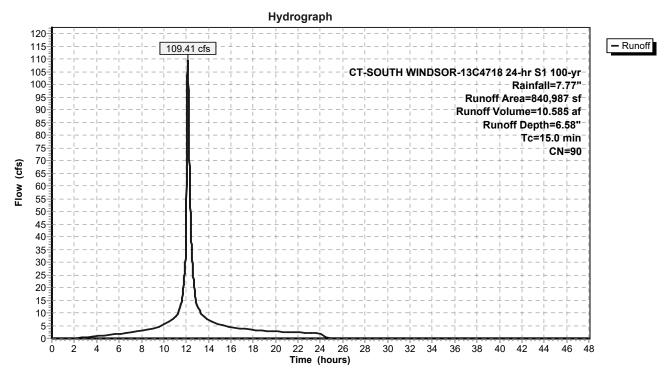
Summary for Subcatchment EDA-1: Area to Detention Basin 7

Runoff 109.41 cfs @ 12.15 hrs, Volume= 10.585 af, Depth= 6.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

A	rea (sf)	CN	<u>Description</u>							
2	299,131	98	Paved parking, HSG B							
2	282,062	98	Paved parking, HSG C							
	11,034	98	Paved parking, HSG B							
	1,191	98	Paved parking, HSG C							
1	80,158	69	50-75% Gra	ass cover, I	Fair, HSG B					
	59,799	79	50-75% Grass cover, Fair, HSG C							
	7,145	69	50-75% Grass cover, Fair, HSG B							
	467	79	50-75% Gra	ass cover, I	Fair, HSG C					
3	340,987	90	Weighted A	verage						
2	247,569		29.44% Pei	vious Area	l					
5	593,418		70.56% Imp	ervious Ar	ea					
Tc	Length	Slope	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
15.0					Direct Entry,	Master Planned Tc				

Subcatchment EDA-1: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 52

Summary for Subcatchment EDA-2: Area to Wetland DP-2

Runoff = 131.15 cfs @ 12.06 hrs, Volume= 9.412 af, Depth= 6.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

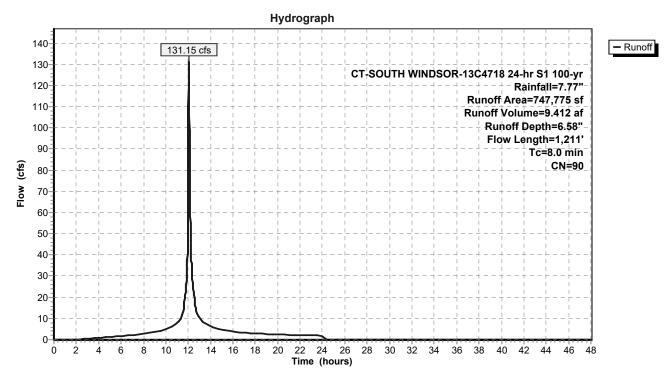
A	rea (sf)	CN E	escription			
5	17,459	98 F	aved park	ing, HSG B	}	
	10,227	98 F	aved park	ing, HSG C		
	4,362	98 F	aved park	ing, HSG D		
2	13,896	69 5	0-75% Gra	ass cover, F	Fair, HSG B	
588 79 50-75% Grass cover, Fair, HSG C						
	1,243	84 5	0-75% Gra	ass cover, F	Fair, HSG D	
7	47,775	90 V	Veighted A	verage		
2	15,727	2	8.85% Per	vious Area		
5	32,048	7	1.15% lmp	ervious Ar	ea	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
3.6	74	0.1350	0.34		Sheet Flow, 1	
					Grass: Short n= 0.150 P2= 3.11"	
0.4	26	0.0250	1.13		Sheet Flow, 2	
					Smooth surfaces n= 0.011 P2= 3.11"	
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3	
					Paved Kv= 20.3 fps	
1.7	744	0.0050	7.35	23.11	Pipe Channel, RCP_Round 24"	
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'	
4.0	4-4		0.40		n= 0.009 Corrugated PE, smooth interior	
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4	
					Grassed Waterway Kv= 15.0 fps	
8.0	1,211	Total				

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 53

Subcatchment EDA-2: Area to Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 54

Summary for Subcatchment EDA-3: Area to Wetland DP-3

Runoff = 5.55 cfs @ 12.08 hrs, Volume= 0.405 af, Depth= 4.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

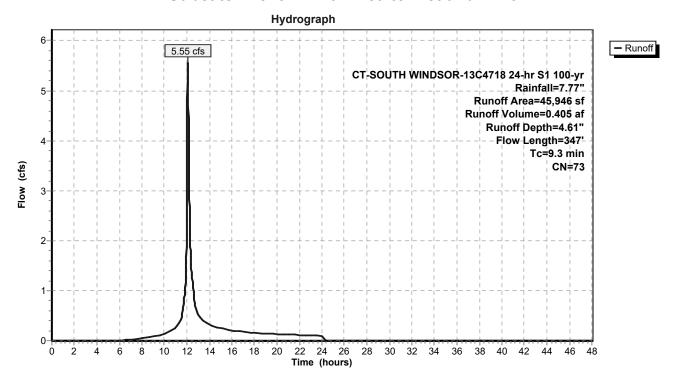
_	Α	rea (sf)	CN	CN Description								
		0	98	98 Paved parking, HSG B								
		0	98	, J								
		0	98	Paved parking, HSG D								
		21,004	69	50-75% Grass cover, Fair, HSG B								
		0	79	50-75% Gra	ass cover, l	Fair, HSG C						
		5,451	84	50-75% Gra	ass cover, l	Fair, HSG D						
		2,225		Brush, Fair,								
_		17,266	77	77 Brush, Fair, HSG D								
		45,946	73	73 Weighted Average								
		45,946		100.00% P	ervious Are	ea						
	_											
	Tc	Length	Slope			Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	7.5	100	0.0400	0.22		Sheet Flow, 1						
						Grass: Short n= 0.150 P2= 3.11"						
	0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2						
						Grassed Waterway Kv= 15.0 fps						
	1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3						
_						Woodland Kv= 5.0 fps						
	9.3	347	Total									

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 55

Subcatchment EDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 56

Summary for Subcatchment EDA-4: Area to Wetland DP-4

Runoff = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af, Depth= 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

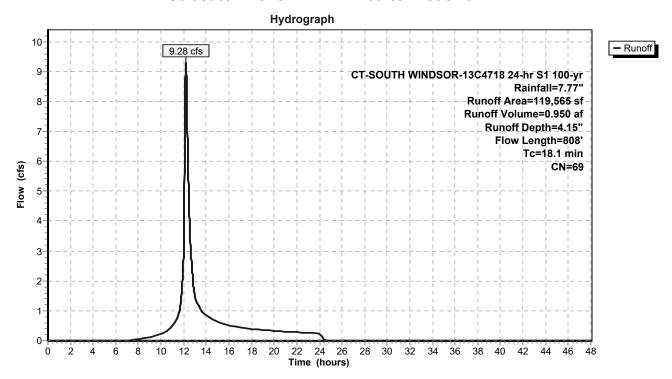
A	rea (sf)	CN E	escription					
	0	98 F	aved park	ing, HSG E	3			
	0	98 F	aved park	ing, HSG C				
	0	98 F	aved park	ing, HSG D				
	57,679	69 5	0-75% Gra	ass cover, I	Fair, HSG B			
	26,837	79 5	0-75% Gra	ass cover, I	Fair, HSG C			
	0	84 5	0-75% Gra	ass cover, I	Fair, HSG D			
	25,526	56 E	Brush, Fair,	, HSG B				
	9,523	70 E	Brush, Fair,	, HSG C				
1	19,565	69 V	Veighted A	verage				
1	19,565	1	100.00% Pervious Area					
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	100	0.0800	0.29		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2			
					Grassed Waterway Kv= 15.0 fps			
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3			
					Woodland Kv= 5.0 fps			
0.1	74	0.0270	19.82	194.19	Channel Flow, 4			
					Area= 9.8 sf Perim= 15.7' r= 0.62'			
					n= 0.009 Corrugated PE, smooth interior			
18.1	808	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 57

Subcatchment EDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 58

Summary for Reach DP-1: Detention Basin 7

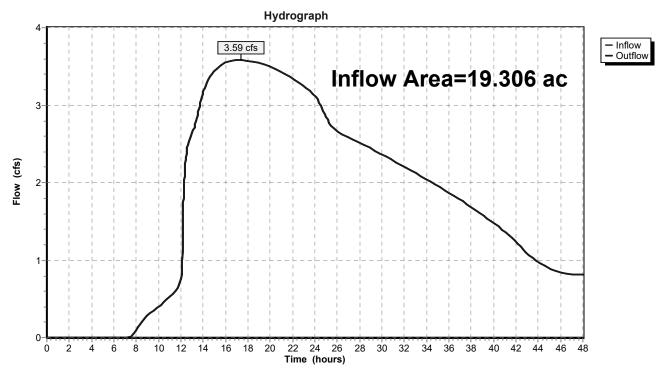
Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth > 4.35" for 100-yr event

Inflow = 3.59 cfs @ 17.35 hrs, Volume= 7.003 af

Outflow = 3.59 cfs @ 17.35 hrs, Volume= 7.003 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 59

Summary for Reach DP-2: Wetland DP-2

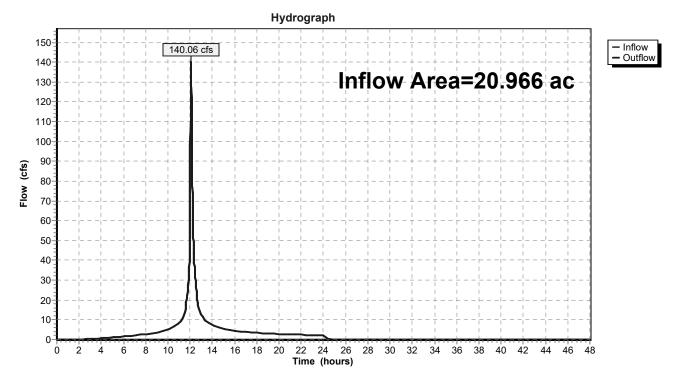
Inflow Area = 20.966 ac, 58.26% Impervious, Inflow Depth = 6.16" for 100-yr event

Inflow = 140.06 cfs @ 12.08 hrs, Volume= 10.767 af

Outflow = 140.06 cfs @ 12.08 hrs, Volume= 10.767 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 60

Summary for Reach DP-3: Wetland DP-3

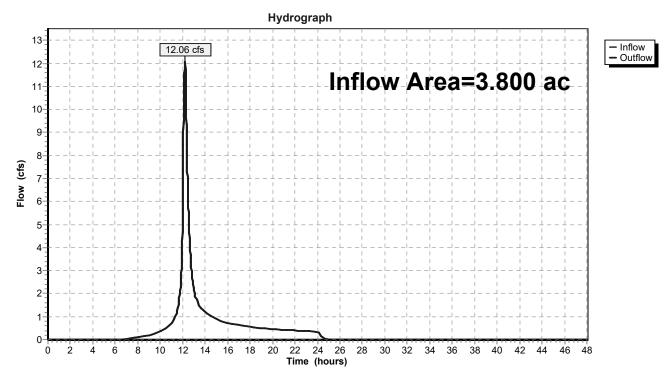
Inflow Area = 3.800 ac, 0.00% Impervious, Inflow Depth = 4.28" for 100-yr event

Inflow = 12.06 cfs @ 12.20 hrs, Volume= 1.355 af

Outflow = 12.06 cfs @ 12.20 hrs, Volume= 1.355 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 61

Summary for Reach DP-4: Wetland DP-4

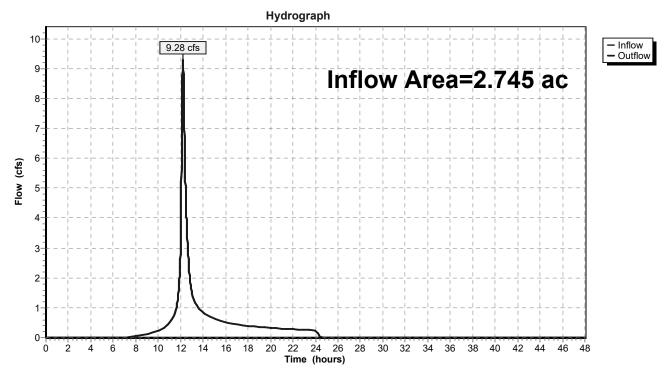
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 4.15" for 100-yr event

Inflow = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af

Outflow = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 62

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 17.167 ac, 71.15% Impervious, Inflow Depth = 6.58" for 100-yr event

Inflow = 131.15 cfs @ 12.06 hrs, Volume= 9.412 af

Outflow = 130.29 cfs @ 12.08 hrs, Volume= 9.412 af, Atten= 1%, Lag= 1.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 10.62 fps, Min. Travel Time= 0.6 min Avg. Velocity = 2.76 fps, Avg. Travel Time= 2.4 min

Peak Storage= 4,855 cf @ 12.07 hrs Average Depth at Peak Storage= 1.04' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

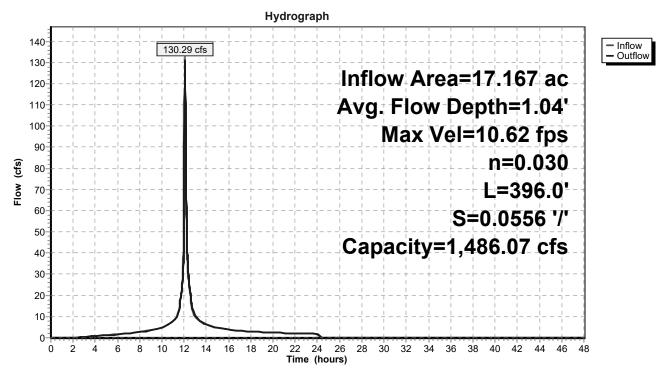
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 63

Outflow

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac. 0.00% Impervious, Inflow Depth = 4.15" for 100-yr event

Inflow 9.28 cfs @ 12.20 hrs, Volume= 0.950 af

Outflow 9.23 cfs @ 12.24 hrs, Volume= 0.950 af, Atten= 1%, Lag= 2.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Max. Velocity= 4.07 fps, Min. Travel Time= 1.4 min Avg. Velocity = 1.49 fps, Avg. Travel Time= 3.8 min

Peak Storage= 780 cf @ 12.22 hrs Average Depth at Peak Storage= 0.22' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

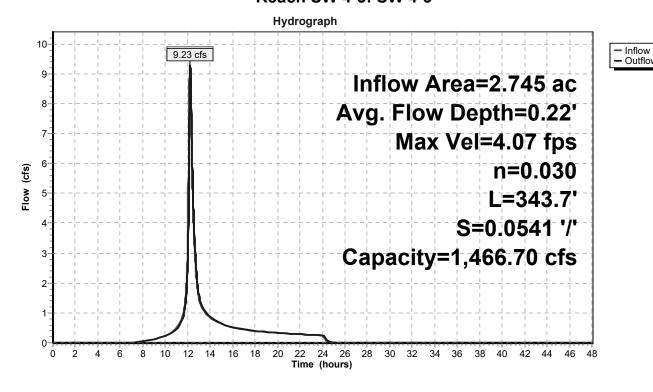
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 64

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth = 6.58" for 100-yr event

Inflow = 109.41 cfs @ 12.15 hrs, Volume= 10.585 af

Outflow = 3.59 cfs @ 17.35 hrs, Volume= 7.003 af, Atten= 97%, Lag= 312.2 min

Primary = 3.59 cfs @ 17.35 hrs, Volume= 7.003 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 98.74' @ 17.35 hrs Surf.Area= 50,990 sf Storage= 329,940 cf

Plug-Flow detention time= 885.9 min calculated for 7.001 af (66% of inflow)

Center-of-Mass det. time= 764.7 min (1,556.8 - 792.1)

Volume	Inve	ert Avail.Sto	rage Storage	Description				
#1	90.0	00' 396,47	79 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)			
		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)				
90.0	00	19,765	0	0				
92.0	00	31,993	51,758	51,758				
94.0	00	37,305	69,298	121,056				
96.0	00	42,927	80,232	201,288				
98.0	00	48,699	91,626	292,914				
100.0	00	54,866	103,565	396,479				
Device	Routing	Invert	Outlet Device	s				
#1	Primary	88.00'	18.0" Round	l Culvert				
			L= 71.0' CM	P, square edge	headwall, Ke= 0.500			
				Inlet / Outlet Invert= 88.00' / 84.50' S= 0.0493 '/' Cc= 0.900				
				n= 0.009 Corrugated PE, smooth interior, Flow Area= 1.77 sf				
#2	Device 1			ifice/Grate C=				
#3	Device 1	95.00'	6.0" Vert. Ori	ifice/Grate C=	0.600			

6.0" Vert. Orifice/Grate C= 0.600

Limited to weir flow at low heads

36.0" x 78.0" Horiz. Orifice/Grate C= 0.600

Primary OutFlow Max=3.59 cfs @ 17.35 hrs HW=98.74' (Free Discharge)

1=Culvert (Passes 3.59 cfs of 26.90 cfs potential flow)

98.00'

99.00'

2=Orifice/Grate (Orifice Controls 1.16 cfs @ 13.25 fps)

-3=Orifice/Grate (Orifice Controls 1.77 cfs @ 9.00 fps)

-4=Orifice/Grate (Orifice Controls 0.66 cfs @ 3.38 fps)

-5=Orifice/Grate (Controls 0.00 cfs)

#4

#5

Device 1

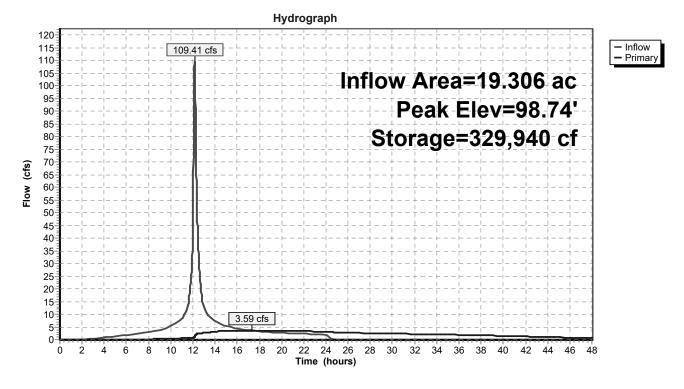
Device 1

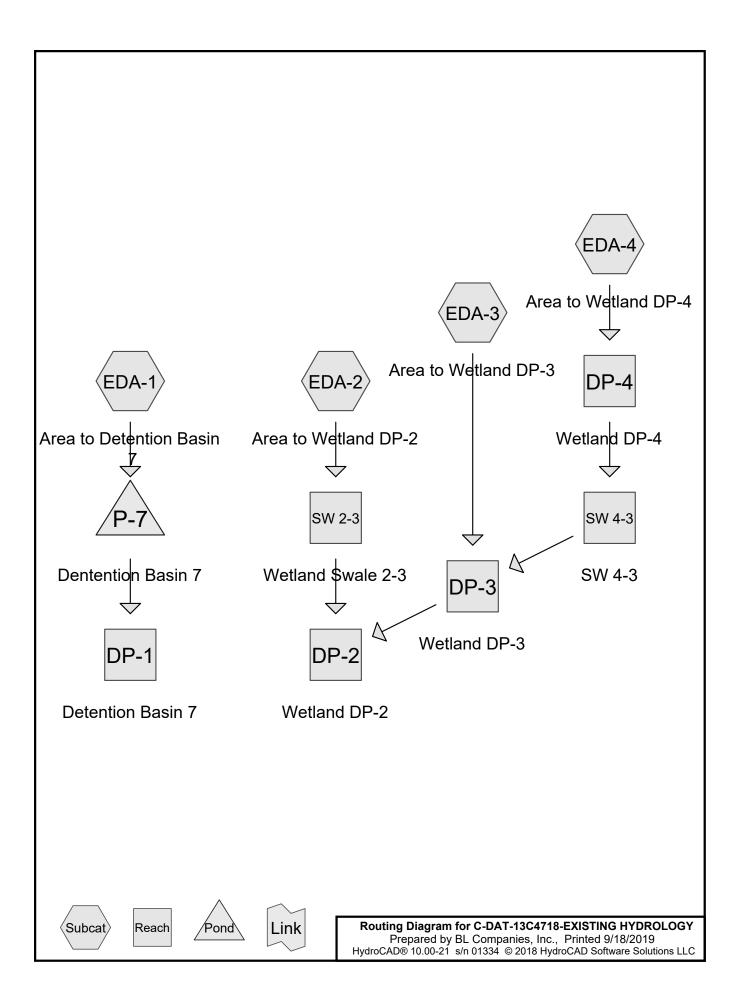
Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 65

Pond P-7: Dentention Basin 7





C-DAT-13C4718-EXISTING HYD CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 2

Time span=0.00-140.00 hrs, dt=0.01 hrs, 14001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: Area to Detention Runoff Area=840,987 sf 70.56% Impervious Runoff Depth=2.09" Tc=15.0 min CN=90 Runoff=38.21 cfs 3.355 af

SubcatchmentEDA-2: Area to Wetland Runoff Area=747,775 sf 71.15% Impervious Runoff Depth=2.09" Flow Length=1,211' Tc=8.0 min CN=90 Runoff=45.99 cfs 2.983 af

SubcatchmentEDA-3: Area to Wetland DP-3 Runoff Area=45,946 sf 0.00% Impervious Runoff Depth=0.93" Flow Length=347' Tc=9.3 min CN=73 Runoff=1.06 cfs 0.081 af

SubcatchmentEDA-4: Area to WetlandRunoff Area=119,565 sf 0.00% Impervious Runoff Depth=0.73"
Flow Length=808' Tc=18.1 min CN=69 Runoff=1.42 cfs 0.167 af

Reach DP-1: Detention Basin 7 Inflow=0.69 cfs 2.814 af

Outflow=0.69 cfs 2.814 af

Reach DP-2: Wetland DP-2 Inflow=46.65 cfs 3.231 af

Outflow=46.65 cfs 3.231 af

Reach DP-3: Wetland DP-3 Inflow=1.83 cfs 0.248 af

Outflow=1.83 cfs 0.248 af

Reach DP-4: Wetland DP-4 Inflow=1.42 cfs 0.167 af

Outflow=1.42 cfs 0.167 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=0.56' Max Vel=7.39 fps Inflow=45.99 cfs 2.983 af

 $n = 0.030 \quad L = 396.0' \quad S = 0.0556 \; '/' \quad Capacity = 1,486.07 \; cfs \quad Outflow = 45.33 \; cfs \; \; 2.983 \; afs \; \; 1.000 \; cfs \; \; 1.000 \; cf$

Reach SW 4-3: SW 4-3 Avg. Flow Depth=0.07' Max Vel=1.99 fps Inflow=1.42 cfs 0.167 af

n=0.030 L=343.7' S=0.0541 '/' Capacity=1,466.70 cfs Outflow=1.39 cfs 0.167 af

Pond P-7: Dentention Basin 7 Peak Elev=93.90' Storage=117,300 cf Inflow=38.21 cfs 3.355 af

Outflow=0.69 cfs 2.814 af

Total Runoff Area = 40.273 ac Runoff Volume = 6.587 af Average Runoff Depth = 1.96" 35.84% Pervious = 14.435 ac 64.16% Impervious = 25.837 ac

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 3

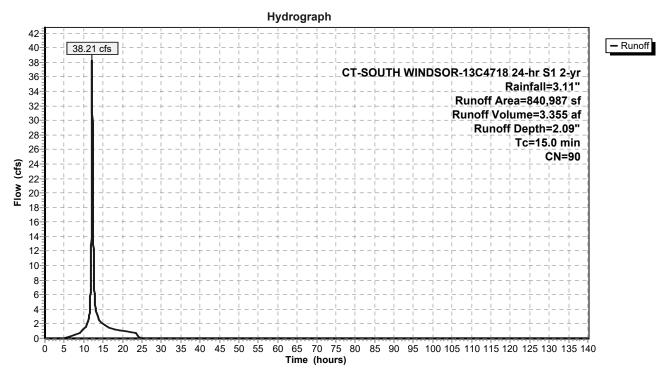
Summary for Subcatchment EDA-1: Area to Detention Basin 7

Runoff 38.21 cfs @ 12.15 hrs, Volume= 3.355 af, Depth= 2.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

A	rea (sf)	CN	Description							
2	299,131	98	Paved parking, HSG B							
2	282,062	98	Paved parking, HSG C							
	11,034	98	Paved park	ing, HSG E	3					
	1,191	98	Paved park	ing, HSG C)					
1	80,158	69	50-75% Gra	ass cover, I	Fair, HSG B					
	59,799	79	50-75% Gra	ass cover, I	Fair, HSG C					
	7,145	69	50-75% Gra	ass cover, I	Fair, HSG B					
	467	79	50-75% Gra	ass cover, I	Fair, HSG C					
3	340,987	90	Weighted A	verage						
2	247,569		29.44% Per	vious Area	l					
593,418 70.56% Impervious Area										
			-							
Tc	Length	Slope	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
15.0					Direct Entry,	Master Planned Tc				

Subcatchment EDA-1: Area to Detention Basin 7



HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 4

Summary for Subcatchment EDA-2: Area to Wetland DP-2

Runoff = 45.99 cfs @ 12.06 hrs, Volume= 2.983 af, Depth= 2.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

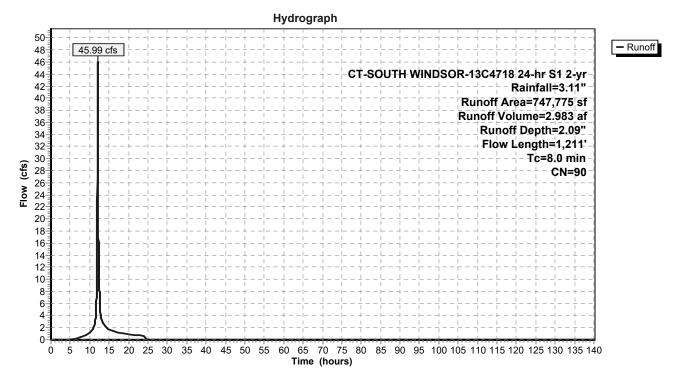
A	rea (sf)	CN E	Description						
5	17,459	98 F	98 Paved parking, HSG B						
	10,227	98 F	Paved parking, HSG C						
	4,362	98 F	Paved park	ing, HSG D					
2	13,896	69 5	0-75% Gra	ass cover, F	Fair, HSG B				
	588	79 5	0-75% Gra	ass cover, F	Fair, HSG C				
	1,243	84 5	0-75% Gra	ass cover, F	Fair, HSG D				
7	47,775	90 V	Veighted A	verage					
2	15,727	2	8.85% Per	vious Area					
5	32,048	7	'1.15% lmp	ervious Ar	ea				
Tc	Length	Slope	Velocity		Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.6	74	0.1350	0.34		Sheet Flow, 1				
					Grass: Short n= 0.150 P2= 3.11"				
0.4	26	0.0250	1.13		Sheet Flow, 2				
					Smooth surfaces n= 0.011 P2= 3.11"				
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3				
					Paved Kv= 20.3 fps				
1.7	744	0.0050	7.35	23.11	Pipe Channel, RCP_Round 24"				
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'				
4.0	454	0.0000	0.40		n= 0.009 Corrugated PE, smooth interior				
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4				
	4.04.				Grassed Waterway Kv= 15.0 fps				
8.0	1,211	Total							

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 5

Subcatchment EDA-2: Area to Wetland DP-2



HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 6

Summary for Subcatchment EDA-3: Area to Wetland DP-3

Runoff = 1.06 cfs @ 12.08 hrs, Volume= 0.081 af, Depth= 0.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

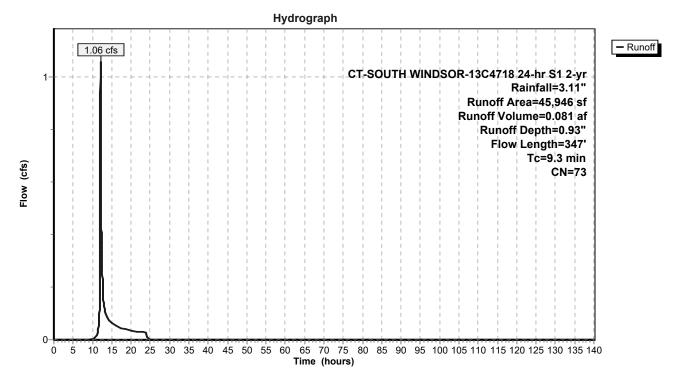
	Α	rea (sf)	CN	Description								
		0	98	Paved parking, HSG B								
		0	98	Paved parking, HSG C								
		0	98	Paved park	ing, HSG D							
		21,004	69	50-75% Gra	ass cover, I	Fair, HSG B						
		0	79	50-75% Gra	ass cover, l	Fair, HSG C						
		5,451			•	Fair, HSG D						
		2,225		Brush, Fair,								
		17,266	77	Brush, Fair,	HSG D							
		45,946		Weighted A								
		45,946		100.00% Pe	ervious Are	ea						
	Тс	Length	Slope	•	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	7.5	100	0.0400	0.22		Sheet Flow, 1						
						Grass: Short n= 0.150 P2= 3.11"						
	0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2						
						Grassed Waterway Kv= 15.0 fps						
	1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3						
_						Woodland Kv= 5.0 fps						
	9.3	347	Total									

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 7

Subcatchment EDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 8

Summary for Subcatchment EDA-4: Area to Wetland DP-4

Runoff = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af, Depth= 0.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

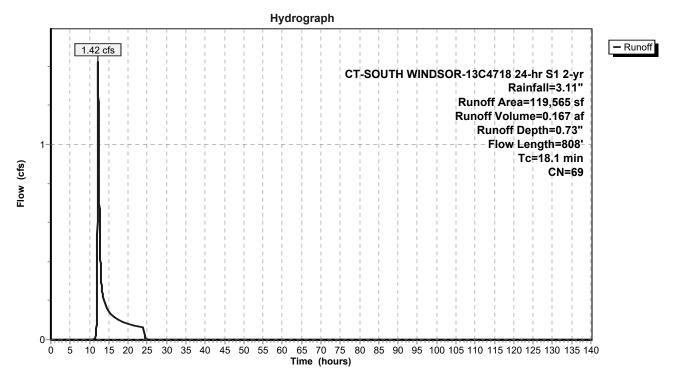
	rea (sf)	CN E	escription				
	0	98 F	aved park	ing, HSG E	3		
	0	98 F	aved park	ing, HSG C			
	0	98 F	aved park	ing, HSG D			
	57,679	69 5	0-75% Gra	ass cover, I	Fair, HSG B		
	26,837	79 5	0-75% Gra	ass cover, I	Fair, HSG C		
	0	84 5	0-75% Gra	ass cover, I	Fair, HSG D		
	25,526	56 E	Brush, Fair,	, HSG B			
	9,523	70 E	Brush, Fair,	, HSG C			
,	119,565	69 V	Veighted A	verage			
	119,565	1	100.00% Pervious Area				
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.7	100	0.0800	0.29		Sheet Flow, 1		
					Grass: Short n= 0.150 P2= 3.11"		
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2		
					Grassed Waterway Kv= 15.0 fps		
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3		
					Woodland Kv= 5.0 fps		
0.1	74	0.0270	19.82	194.19	•		
					Area= 9.8 sf Perim= 15.7' r= 0.62'		
					n= 0.009 Corrugated PE, smooth interior		
18.1	808	Total					

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 9

Subcatchment EDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 10

Summary for Reach DP-1: Detention Basin 7

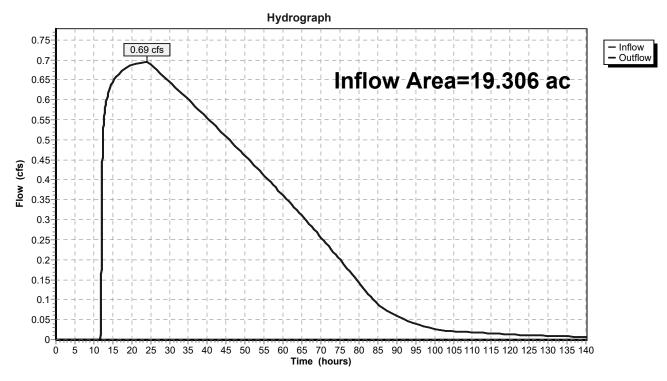
Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth > 1.75" for 2-yr event

Inflow = 0.69 cfs @ 24.10 hrs, Volume= 2.814 af

Outflow = 0.69 cfs @ 24.10 hrs, Volume= 2.814 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 11

Summary for Reach DP-2: Wetland DP-2

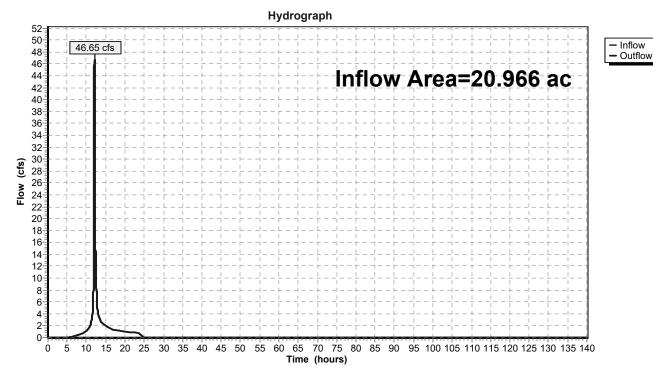
Inflow Area = 20.966 ac, 58.26% Impervious, Inflow Depth = 1.85" for 2-yr event

Inflow = 46.65 cfs @ 12.09 hrs, Volume= 3.231 af

Outflow = 46.65 cfs @ 12.09 hrs, Volume= 3.231 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 12

Summary for Reach DP-3: Wetland DP-3

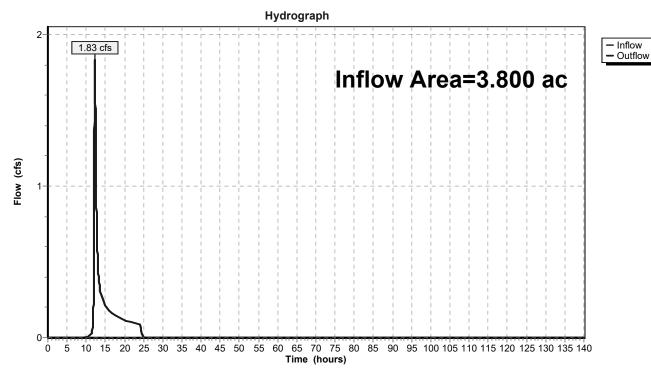
Inflow Area = 3.800 ac, 0.00% Impervious, Inflow Depth = 0.78" for 2-yr event

Inflow = 1.83 cfs @ 12.28 hrs, Volume= 0.248 af

Outflow = 1.83 cfs @ 12.28 hrs, Volume= 0.248 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 13

Summary for Reach DP-4: Wetland DP-4

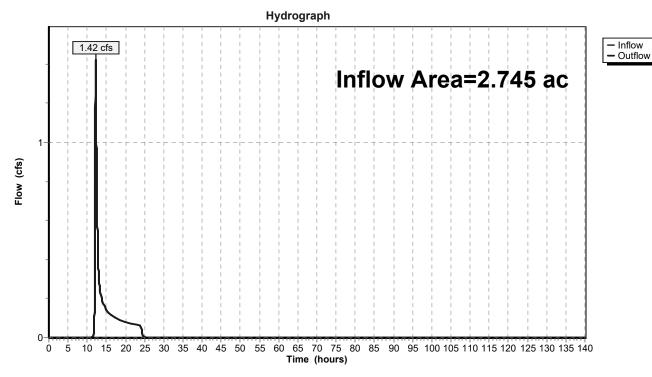
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 0.73" for 2-yr event

Inflow = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af

Outflow = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 14

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 17.167 ac, 71.15% Impervious, Inflow Depth = 2.09" for 2-yr event

Inflow = 45.99 cfs @ 12.06 hrs, Volume= 2.983 af

Outflow = 45.33 cfs @ 12.09 hrs, Volume= 2.983 af, Atten= 1%, Lag= 1.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 7.39 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.93 fps, Avg. Travel Time= 3.4 min

Peak Storage= 2,436 cf @ 12.07 hrs Average Depth at Peak Storage= 0.56' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

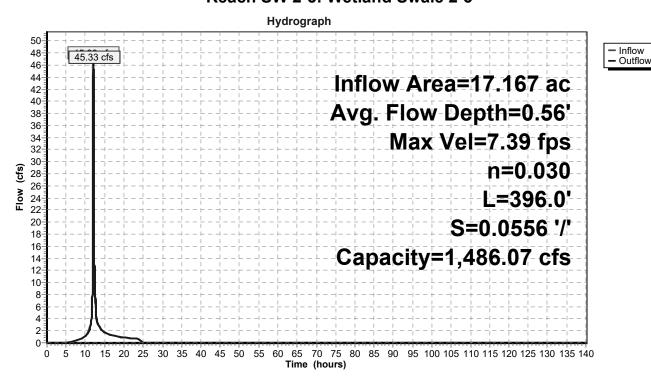
10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 15

InflowOutflow

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 0.73" for 2-yr event

Inflow = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af

Outflow = 1.39 cfs @ 12.31 hrs, Volume= 0.167 af, Atten= 3%, Lag= 5.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.99 fps, Min. Travel Time= 2.9 min Avg. Velocity = 1.36 fps, Avg. Travel Time= 4.2 min

Peak Storage= 240 cf @ 12.26 hrs Average Depth at Peak Storage= 0.07'

Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

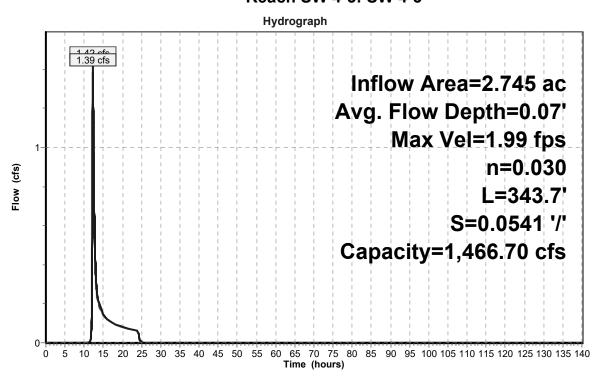
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 16

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth = 2.09" for 2-yr event

Inflow 38.21 cfs @ 12.15 hrs, Volume= 3.355 af

0.69 cfs @ 24.10 hrs, Volume= Outflow 2.814 af, Atten= 98%, Lag= 716.8 min

Primary 0.69 cfs @ 24.10 hrs, Volume= 2.814 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 93.90' @ 24.10 hrs Surf.Area= 37,037 sf Storage= 117,300 cf

Plug-Flow detention time= 1,793.4 min calculated for 2.814 af (84% of inflow)

Center-of-Mass det. time= 1,717.2 min (2,546.8 - 829.5)

Volume	Inve	ert Avail.Sto	rage Storage	Description					
#1	90.0	0' 396,47	79 cf Custom	9 cf Custom Stage Data (Prismatic)Listed below (Recalc)					
	Elevation Surf.Area (feet) (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)					
90.0	00	19,765	0	0					
92.0	00	31,993	51,758	51,758					
94.0	00	37,305	69,298	121,056					
96.0	00	42,927	80,232	201,288					
98.0	00	48,699	91,626	292,914					
100.0	00	54,866	103,565	396,479					
Device	Routing	Invert	Outlet Device	es					
#1	Primary	88.00'	18.0" Round	d Culvert					
				L= 71.0' CMP, square edge headwall, Ke= 0.500					
			Inlet / Outlet I	Inlet / Outlet Invert = 88.00' / 84.50' S= 0.0493 '/' Cc= 0.900					
					ooth interior, Flow Area= 1.77 sf				
#2	Device 1	91.00'		ifice/Grate C=					
#3	Device 1	95.00' 6.0" Vert. Orifice/Grate C= 0.600							

6.0" Vert. Orifice/Grate C= 0.600

Limited to weir flow at low heads

36.0" x 78.0" Horiz. Orifice/Grate C= 0.600

Primary OutFlow Max=0.69 cfs @ 24.10 hrs HW=93.90' (Free Discharge)

-1=Culvert (Passes 0.69 cfs of 19.31 cfs potential flow)

98.00'

99.00'

2=Orifice/Grate (Orifice Controls 0.69 cfs @ 7.96 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

#4

#5

Device 1

Device 1

-4=Orifice/Grate (Controls 0.00 cfs)

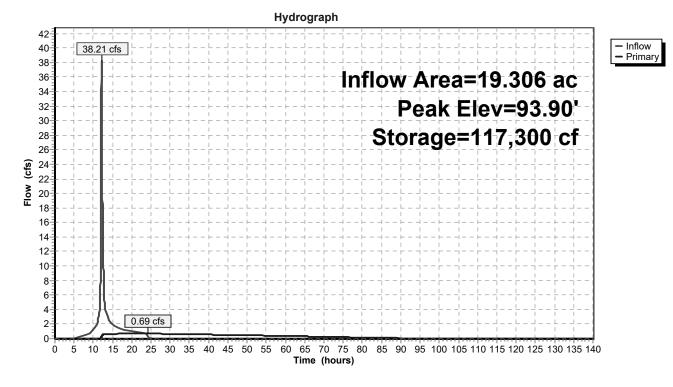
-5=Orifice/Grate (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 17

Pond P-7: Dentention Basin 7



C-DAT-13C4718-EXISTING HY CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 18

Time span=0.00-140.00 hrs, dt=0.01 hrs, 14001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: Area to Detention Runoff Area=840,987 sf 70.56% Impervious Runoff Depth=3.79" Tc=15.0 min CN=90 Runoff=66.14 cfs 6.097 af

SubcatchmentEDA-2: Area to Wetland Runoff Area=747,775 sf 71.15% Impervious Runoff Depth=3.79" Flow Length=1,211' Tc=8.0 min CN=90 Runoff=79.37 cfs 5.421 af

SubcatchmentEDA-3: Area to Wetland DP-3 Runoff Area=45,946 sf 0.00% Impervious Runoff Depth=2.21" Flow Length=347' Tc=9.3 min CN=73 Runoff=2.67 cfs 0.194 af

SubcatchmentEDA-4: Area to Wetland

Runoff Area=119,565 sf 0.00% Impervious Runoff Depth=1.89"
Flow Length=808' Tc=18.1 min CN=69 Runoff=4.16 cfs 0.433 af

Reach DP-1: Detention Basin 7 Inflow=1.72 cfs 5.536 af

Outflow=1.72 cfs 5.536 af

Reach DP-2: Wetland DP-2 Inflow=82.76 cfs 6.048 af

Outflow=82.76 cfs 6.048 af

Reach DP-3: Wetland DP-3 Inflow=5.39 cfs 0.627 af

Outflow=5.39 cfs 0.627 af

Reach DP-4: Wetland DP-4 Inflow=4.16 cfs 0.433 af

Outflow=4.16 cfs 0.433 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=0.78' Max Vel=8.96 fps Inflow=79.37 cfs 5.421 af

 $n = 0.030 \quad L = 396.0' \quad S = 0.0556 \; '/' \quad Capacity = 1,486.07 \; cfs \quad Outflow = 78.58 \; cfs \; \; 5.421 \; afs \; (1.000 \pm 0.000 \pm 0.0000 \pm 0.000 \pm 0.$

Reach SW 4-3: SW 4-3 Avg. Flow Depth=0.13' Max Vel=3.00 fps Inflow=4.16 cfs 0.433 af

n=0.030 L=343.7' S=0.0541 '/' Capacity=1,466.70 cfs Outflow=4.12 cfs 0.433 af

Pond P-7: Dentention Basin 7 Peak Elev=95.97' Storage=199,916 cf Inflow=66.14 cfs 6.097 af

Outflow=1.72 cfs 5.536 af

Total Runoff Area = 40.273 ac Runoff Volume = 12.145 af Average Runoff Depth = 3.62" 35.84% Pervious = 14.435 ac 64.16% Impervious = 25.837 ac

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 19

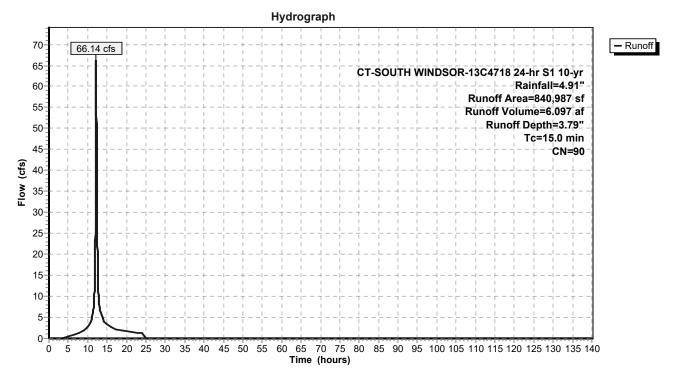
Summary for Subcatchment EDA-1: Area to Detention Basin 7

Runoff = 66.14 cfs @ 12.15 hrs, Volume= 6.097 af, Depth= 3.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

A	rea (sf)	CN	Description								
2	99,131	98	Paved parking, HSG B								
2	82,062	98	Paved park	Paved parking, HSG C							
	11,034	98	Paved park	Paved parking, HSG B							
	1,191	98	Paved park	ing, HSG C)						
1	80,158	69	50-75% Gra	ass cover, I	Fair, HSG B						
	59,799	79	50-75% Gra	ass cover, I	Fair, HSG C						
	7,145	69	50-75% Gra	ass cover, I	Fair, HSG B						
	467	79	50-75% Gra	ass cover, l	Fair, HSG C						
8	40,987	90	Weighted A	verage							
2	47,569		29.44% Per	rvious Area	1						
593,418 70.56% Impervious Area											
Tc	Length	Slop	e Velocity	Capacity	Description						
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)							
15.0					Direct Entry,	Master Planned Tc					

Subcatchment EDA-1: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 20

Summary for Subcatchment EDA-2: Area to Wetland DP-2

Runoff = 79.37 cfs @ 12.06 hrs, Volume= 5.421 af, Depth= 3.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

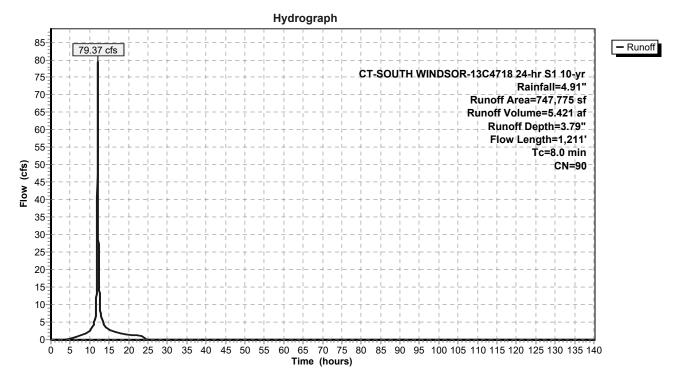
A	rea (sf)	CN D	escription						
5	517,459	98 F	aved park	ing, HSG B	}				
	10,227	98 F	Paved parking, HSG C						
	4,362	98 F	aved park	ing, HSG D					
2	213,896	69 5	0-75% Gra	ass cover, I	Fair, HSG B				
	588				Fair, HSG C				
	1,243	84 5	0-75% Gra	ass cover, l	Fair, HSG D				
7	47,775	90 V	Veighted A	verage					
2	215,727	2	8.85% Per	vious Area					
5	32,048	7	1.15% lmp	ervious Ar	ea				
Tc	Length	Slope	Velocity		Description				
(min)_	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.6	74	0.1350	0.34		Sheet Flow, 1				
					Grass: Short n= 0.150 P2= 3.11"				
0.4	26	0.0250	1.13		Sheet Flow, 2				
					Smooth surfaces n= 0.011 P2= 3.11"				
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3				
4 7	744	0.0050	7.05	00.44	Paved Kv= 20.3 fps				
1.7	744	0.0050	7.35	23.11	Pipe Channel, RCP_Round 24"				
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'				
1.0	151	0.0000	2.42		n= 0.009 Corrugated PE, smooth interior				
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4				
	4.044	T.4.1			Grassed Waterway Kv= 15.0 fps				
8.0	1,211	Total							

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 21

Subcatchment EDA-2: Area to Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 22

Summary for Subcatchment EDA-3: Area to Wetland DP-3

Runoff = 2.67 cfs @ 12.08 hrs, Volume= 0.194 af, Depth= 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

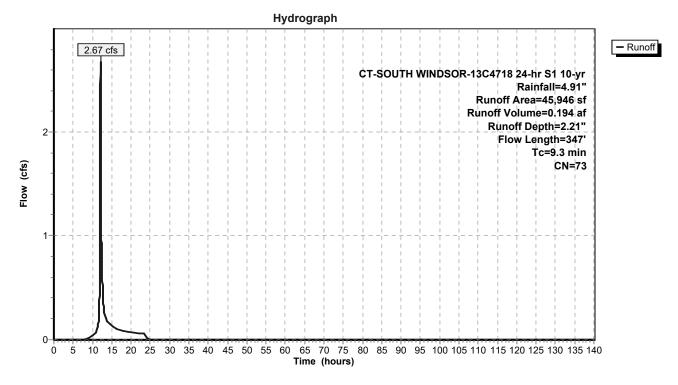
_	Α	rea (sf)	CN	Description								
		0	98	8 Paved parking, HSG B								
		0	98	Paved park	Paved parking, HSG C							
		0	98	Paved park	ing, HSG D)						
		21,004	69	50-75% Gr	ass cover, l	Fair, HSG B						
		0	79	50-75% Gr	ass cover, l	Fair, HSG C						
		5,451	84	50-75% Gr	ass cover, l	Fair, HSG D						
		2,225	56	Brush, Fair	•							
_		17,266	77	Brush, Fair	, HSG D							
		45,946	73	Weighted A	•							
		45,946		100.00% P	ervious Are	ea						
	_											
	Tc	Length	Slop	•	Capacity	Description						
_	(min)	(feet)	(ft/f		(cfs)							
	7.5	100	0.040	0 0.22		Sheet Flow, 1						
						Grass: Short n= 0.150 P2= 3.11"						
	0.6	125	0.048	0 3.29		Shallow Concentrated Flow, 2						
		4.0.5				Grassed Waterway Kv= 15.0 fps						
	1.2	122	0.106	0 1.63		Shallow Concentrated Flow, 3						
_						Woodland Kv= 5.0 fps						
	93	347	Total									

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 23

Subcatchment EDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 24

Summary for Subcatchment EDA-4: Area to Wetland DP-4

Runoff = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af, Depth= 1.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

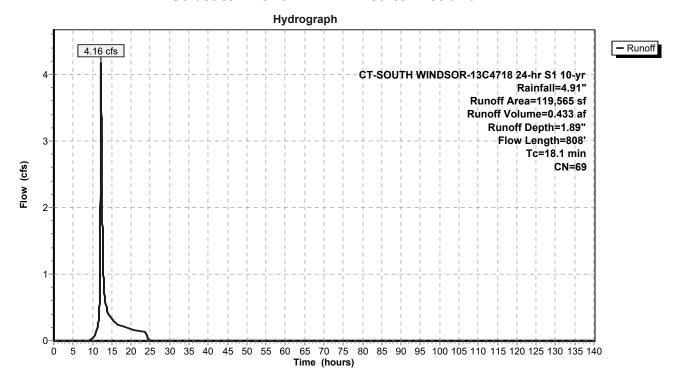
	rea (sf)	CN E	escription				
	0	98 F	aved park	ing, HSG E	3		
	0	98 F	aved park	ing, HSG C			
	0	98 F	aved park	ing, HSG D			
	57,679	69 5	0-75% Gra	ass cover, I	Fair, HSG B		
	26,837	79 5	0-75% Gra	ass cover, I	Fair, HSG C		
	0	84 5	0-75% Gra	ass cover, I	Fair, HSG D		
	25,526	56 E	Brush, Fair,	, HSG B			
	9,523	70 E	Brush, Fair,	, HSG C			
,	119,565	69 V	Veighted A	verage			
	119,565	1	100.00% Pervious Area				
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.7	100	0.0800	0.29		Sheet Flow, 1		
					Grass: Short n= 0.150 P2= 3.11"		
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2		
					Grassed Waterway Kv= 15.0 fps		
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3		
					Woodland Kv= 5.0 fps		
0.1	74	0.0270	19.82	194.19	•		
					Area= 9.8 sf Perim= 15.7' r= 0.62'		
					n= 0.009 Corrugated PE, smooth interior		
18.1	808	Total					

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 25

Subcatchment EDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 26

Summary for Reach DP-1: Detention Basin 7

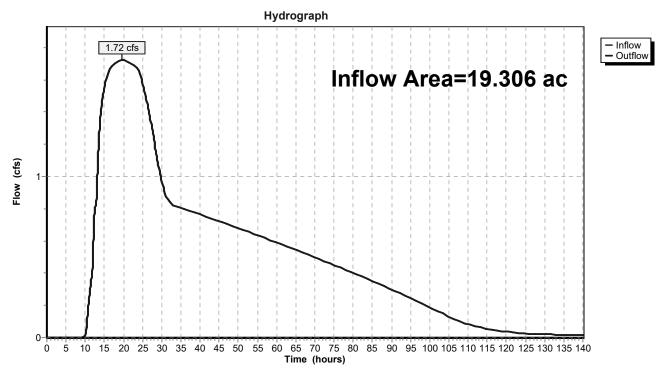
Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth > 3.44" for 10-yr event

Inflow = 1.72 cfs @ 19.53 hrs, Volume= 5.536 af

Outflow = 1.72 cfs @ 19.53 hrs, Volume= 5.536 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 27

Summary for Reach DP-2: Wetland DP-2

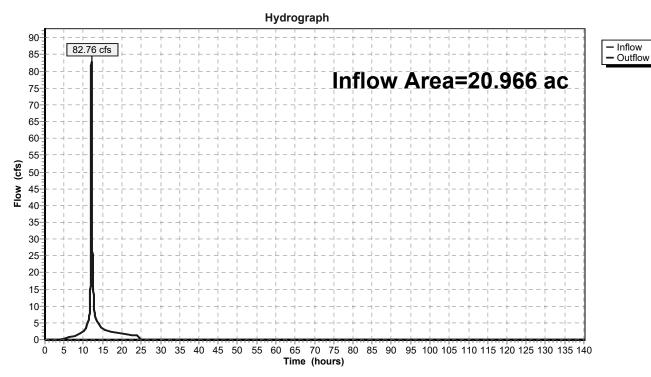
Inflow Area = 20.966 ac, 58.26% Impervious, Inflow Depth = 3.46" for 10-yr event

Inflow = 82.76 cfs @ 12.08 hrs, Volume= 6.048 af

Outflow = 82.76 cfs @ 12.08 hrs, Volume= 6.048 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 28

Summary for Reach DP-3: Wetland DP-3

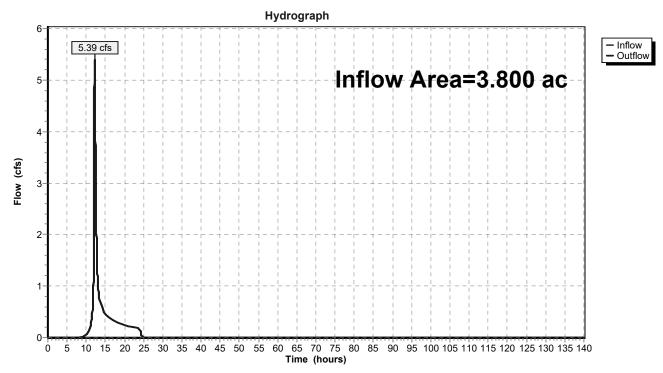
Inflow Area = 3.800 ac, 0.00% Impervious, Inflow Depth = 1.98" for 10-yr event

Inflow = 5.39 cfs @ 12.22 hrs, Volume= 0.627 af

Outflow = 5.39 cfs @ 12.22 hrs, Volume= 0.627 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 29

Summary for Reach DP-4: Wetland DP-4

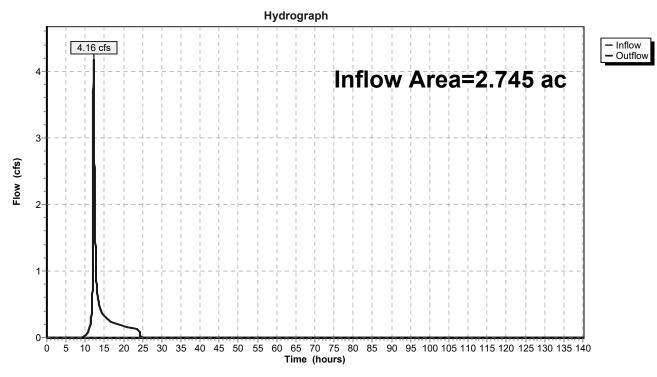
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 1.89" for 10-yr event

Inflow = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af

Outflow = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 30

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 17.167 ac, 71.15% Impervious, Inflow Depth = 3.79" for 10-yr event

Inflow = 79.37 cfs @ 12.06 hrs, Volume= 5.421 af

Outflow = 78.58 cfs @ 12.08 hrs, Volume= 5.421 af, Atten= 1%, Lag= 1.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 8.96 fps, Min. Travel Time= 0.7 min Avg. Velocity = 2.30 fps, Avg. Travel Time= 2.9 min

Peak Storage= 3,478 cf @ 12.07 hrs Average Depth at Peak Storage= 0.78'

Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

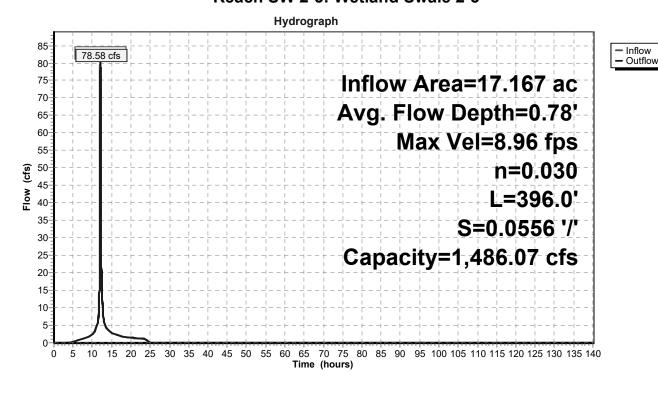
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 31

Inflow

Outflow

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 1.89" for 10-yr event

Inflow = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af

Outflow = 4.12 cfs @ 12.26 hrs, Volume= 0.433 af, Atten= 1%, Lag= 3.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.00 fps, Min. Travel Time= 1.9 min Avg. Velocity = 1.40 fps, Avg. Travel Time= 4.1 min

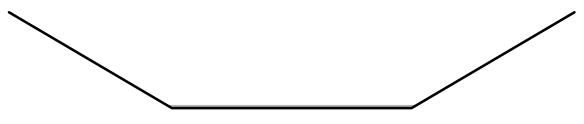
Peak Storage= 473 cf @ 12.23 hrs Average Depth at Peak Storage= 0.13' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

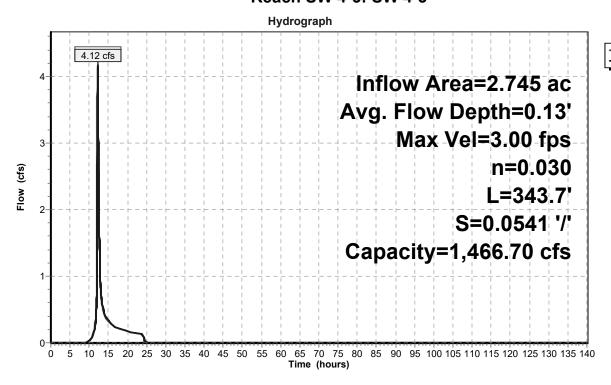
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 32

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth = 3.79" for 10-yr event

Inflow 66.14 cfs @ 12.15 hrs, Volume= 6.097 af

1.72 cfs @ 19.53 hrs, Volume= Outflow 5.536 af, Atten= 97%, Lag= 442.8 min

Primary 1.72 cfs @ 19.53 hrs, Volume= 5.536 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 95.97' @ 19.53 hrs Surf.Area= 42,837 sf Storage= 199,916 cf

Plug-Flow detention time= 1,923.0 min calculated for 5.536 af (91% of inflow)

Center-of-Mass det. time= 1,872.7 min (2,682.5 - 809.8)

Volume	Inve	ert Avail.Sto	rage Storage	e Description				
#1	90.0	00' 396,4°	79 cf Custor	m Stage Data (P	rismatic)Listed below (Recalc)			
Elevation	on	Surf.Area	Inc.Store	Cum.Store				
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)				
90.0	00	19,765	0	0				
92.0	00	31,993	51,758	51,758				
94.0	00	37,305	69,298	121,056				
96.0	00	42,927	80,232	201,288				
98.0	00	48,699	91,626	292,914				
100.0	00	54,866	103,565	396,479				
Device	Routing	Invert	Outlet Devic	es				
#1	Primary	88.00'	18.0" Roun	d Culvert				
	•		L= 71.0' CN	MP, square edge	headwall, Ke= 0.500			
			Inlet / Outlet	Invert= 88.00' / 8	34.50' S= 0.0493 '/' Cc= 0.900			
			n= 0.009 Cd	orrugated PE, sm	ooth interior, Flow Area= 1.77 sf			
#2	Device 1	91.00'	4.0" Vert. O	rifice/Grate C=	0.600			
#3	Device 1	95.00'	6.0" Vert. O	rifice/Grate C=	0.600			
#4	Device 1	98.00'	6.0" Vert. O	6.0" Vert. Orifice/Grate C= 0.600				

36.0" x 78.0" Horiz. Orifice/Grate C= 0.600

Limited to weir flow at low heads

Primary OutFlow Max=1.72 cfs @ 19.53 hrs HW=95.97' (Free Discharge)

-1=Culvert (Passes 1.72 cfs of 22.86 cfs potential flow)

99.00'

#5

Device 1

²⁼Orifice/Grate (Orifice Controls 0.92 cfs @ 10.55 fps)

⁻³⁼Orifice/Grate (Orifice Controls 0.80 cfs @ 4.08 fps)

⁻⁴⁼Orifice/Grate (Controls 0.00 cfs)

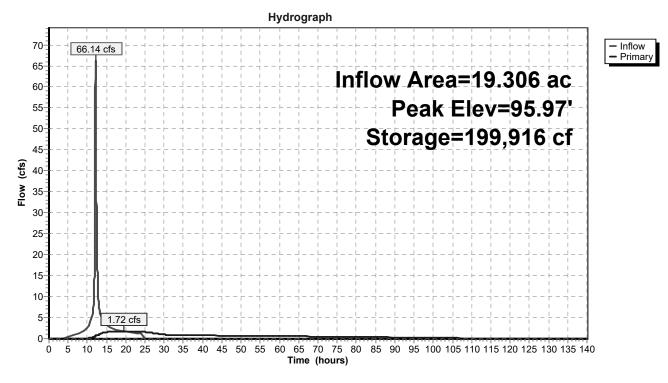
⁻⁵⁼Orifice/Grate (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 33

Pond P-7: Dentention Basin 7



C-DAT-13C4718-EXISTING HY CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 34

Time span=0.00-140.00 hrs, dt=0.01 hrs, 14001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: Area to Detention Runoff Area=840,987 sf 70.56% Impervious Runoff Depth=4.88" Tc=15.0 min CN=90 Runoff=83.26 cfs 7.843 af

SubcatchmentEDA-2: Area to Wetland Runoff Area=747,775 sf 71.15% Impervious Runoff Depth=4.88" Flow Length=1,211' Tc=8.0 min CN=90 Runoff=99.88 cfs 6.974 af

SubcatchmentEDA-3: Area to Wetland DP-3 Runoff Area=45,946 sf 0.00% Impervious Runoff Depth=3.11" Flow Length=347' Tc=9.3 min CN=73 Runoff=3.78 cfs 0.274 af

SubcatchmentEDA-4: Area to WetlandRunoff Area=119,565 sf 0.00% Impervious Runoff Depth=2.74"
Flow Length=808' Tc=18.1 min CN=69 Runoff=6.10 cfs 0.626 af

Reach DP-1: Detention Basin 7 Inflow=2.31 cfs 7.273 af

Outflow=2.31 cfs 7.273 af

Reach DP-2: Wetland DP-2 Inflow=105.35 cfs 7.874 af

Outflow=105.35 cfs 7.874 af

Reach DP-3: Wetland DP-3 Inflow=7.91 cfs 0.899 af

Outflow=7.91 cfs 0.899 af

Reach DP-4: Wetland DP-4 Inflow=6.10 cfs 0.626 af

Outflow=6.10 cfs 0.626 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=0.89' Max Vel=9.70 fps Inflow=99.88 cfs 6.974 af

 $n = 0.030 \quad L = 396.0' \quad S = 0.0556 \; '/' \quad Capacity = 1,486.07 \; cfs \quad Outflow = 99.04 \; cfs \; \; 6.974 \; afs \; \; 1.000 \; cfs \; \; 1.000 \; cf$

Reach SW 4-3: SW 4-3Avg. Flow Depth=0.17' Max Vel=3.47 fps Inflow=6.10 cfs 0.626 af

n=0.030 L=343.7' S=0.0541 '/' Capacity=1,466.70 cfs Outflow=6.06 cfs 0.626 af

Pond P-7: Dentention Basin 7 Peak Elev=97.11' Storage=250,580 cf Inflow=83.26 cfs 7.843 af

Outflow=2.31 cfs 7.273 af

Total Runoff Area = 40.273 ac Runoff Volume = 15.717 af Average Runoff Depth = 4.68" 35.84% Pervious = 14.435 ac 64.16% Impervious = 25.837 ac

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 35

Summary for Subcatchment EDA-1: Area to Detention Basin 7

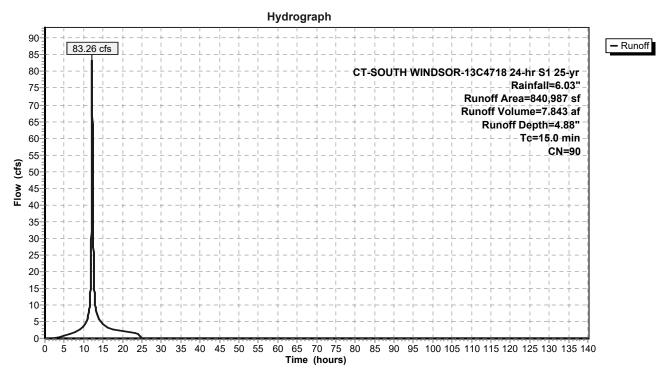
Runoff 83.26 cfs @ 12.15 hrs, Volume= 7.843 af, Depth= 4.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

A	rea (sf)	CN	Description							
2	299,131	98	Paved parking, HSG B							
2	282,062	98	Paved parking, HSG C							
	11,034	98	Paved parking, HSG B							
	1,191	98	Paved park	ing, HSG C)					
1	80,158	69	50-75% Gra	ass cover, I	Fair, HSG B					
	59,799	79	50-75% Gra	ass cover, I	Fair, HSG C					
	7,145	69	50-75% Grass cover, Fair, HSG B							
	467	79	50-75% Grass cover, Fair, HSG C							
3	840,987 90 Weighted Average									
2	247,569 29.44% Pervious Area									
5	593,418 70.56% Impervious Area									
			-							
Tc	Length	Slope	e Velocity	Capacity	Description					
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)						
15.0					Direct Entry,	Master Planned Tc				

Direct Entry, Master Planned Tc

Subcatchment EDA-1: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 36

Summary for Subcatchment EDA-2: Area to Wetland DP-2

Runoff = 99.88 cfs @ 12.06 hrs, Volume= 6.974 af, Depth= 4.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

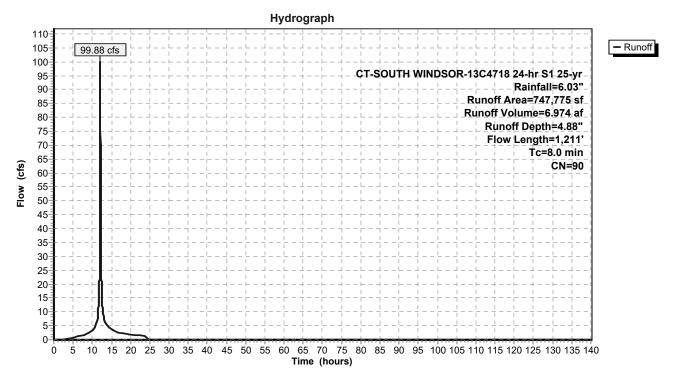
A	rea (sf)	CN E	escription						
5	17,459	98 F	98 Paved parking, HSG B						
	10,227								
	4,362	98 F	aved park	ing, HSG D					
2	13,896	69 5	0-75% Gra	ass cover, I	Fair, HSG B				
	588				Fair, HSG C				
	1,243	84 5	0-75% Gra	ass cover, l	Fair, HSG D				
7	47,775		Veighted A						
	15,727	2	8.85% Per	vious Area					
5	32,048	7	'1.15% lmp	pervious Ar	ea				
_		٥.							
Tc	Length	Slope	Velocity		Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.6	74	0.1350	0.34		Sheet Flow, 1				
			4.40		Grass: Short n= 0.150 P2= 3.11"				
0.4	26	0.0250	1.13		Sheet Flow, 2				
4.4	040	0.0050	2.04		Smooth surfaces n= 0.011 P2= 3.11"				
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3				
1.7	744	0.0050	7.35	23.11	Paved Kv= 20.3 fps Pipe Channel, RCP_Round 24"				
1.7	144	0.0030	7.35	23.11	24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'				
					n= 0.009 Corrugated PE, smooth interior				
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4				
1.2	101	0.0200	2.12		Grassed Waterway Kv= 15.0 fps				
8.0	1,211	Total			5.45554				
0.0	1,411	i Otai							

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 37

Subcatchment EDA-2: Area to Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 38

Summary for Subcatchment EDA-3: Area to Wetland DP-3

Runoff = 3.78 cfs @ 12.08 hrs, Volume= 0.274 af, Depth= 3.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

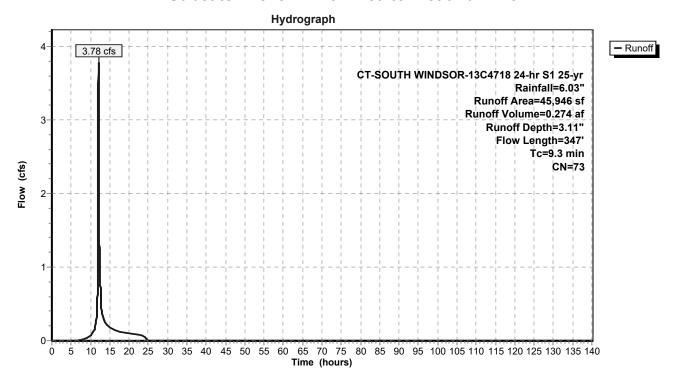
	Α	rea (sf)	CN	Description								
		0	98	Paved parking, HSG B								
		0	98	Paved parking, HSG C								
		0	98	Paved park	ing, HSG D							
		21,004	69	50-75% Gra	ass cover, I	Fair, HSG B						
		0	79	50-75% Gra	ass cover, l	Fair, HSG C						
		5,451			•	Fair, HSG D						
		2,225		Brush, Fair,								
		17,266	77	Brush, Fair,	HSG D							
		45,946		Weighted A								
		45,946		100.00% Pe	ervious Are	ea						
	Тс	Length	Slope	•	Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	7.5	100	0.0400	0.22		Sheet Flow, 1						
						Grass: Short n= 0.150 P2= 3.11"						
	0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2						
						Grassed Waterway Kv= 15.0 fps						
	1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3						
_						Woodland Kv= 5.0 fps						
	9.3	347	Total									

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 39

Subcatchment EDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 40

Summary for Subcatchment EDA-4: Area to Wetland DP-4

Runoff = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

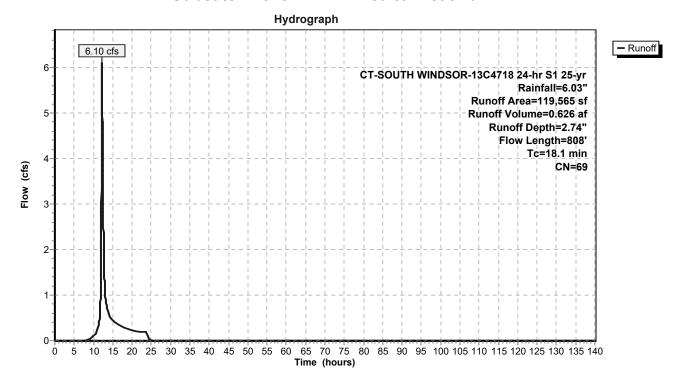
A	rea (sf)	CN D	escription				
	0	98 F	aved park	ing, HSG B	3		
	0			ing, HSG C			
	0	98 F	aved park	ing, HSG D			
	57,679	69 5	0-75% Gra	ass cover, l	Fair, HSG B		
	26,837	79 5	0-75% Gra	ass cover, l	Fair, HSG C		
	0	84 5	0-75% Gra	ass cover, I	Fair, HSG D		
	25,526	56 E	rush, Fair,	, HSG B			
	9,523	70 B	rush, Fair,	, HSG C			
1	19,565	69 V	Veighted A	verage			
1	19,565	1	100.00% Pervious Area				
Tc	Length	Slope	Velocity		Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.7	100	0.0800	0.29		Sheet Flow, 1		
					Grass: Short n= 0.150 P2= 3.11"		
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2		
					Grassed Waterway Kv= 15.0 fps		
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3		
					Woodland Kv= 5.0 fps		
0.1	74	0.0270	19.82	194.19	•		
					Area= 9.8 sf Perim= 15.7' r= 0.62'		
					n= 0.009 Corrugated PE, smooth interior		
18.1	808	Total					

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 41

Subcatchment EDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 42

Summary for Reach DP-1: Detention Basin 7

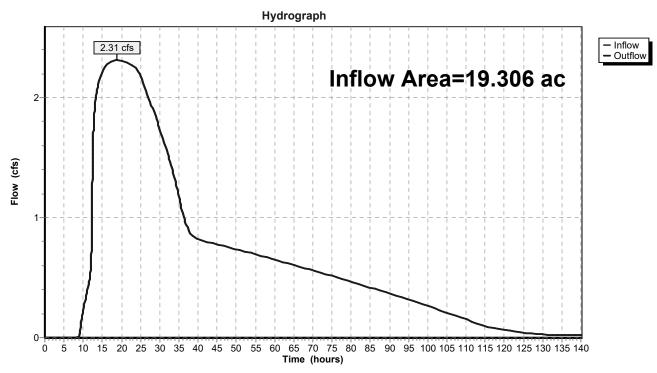
Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth > 4.52" for 25-yr event

Inflow = 2.31 cfs @ 18.82 hrs, Volume= 7.273 af

Outflow = 2.31 cfs @ 18.82 hrs, Volume= 7.273 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



C-DAT-13C4718-EXISTING HY CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 43

Summary for Reach DP-2: Wetland DP-2

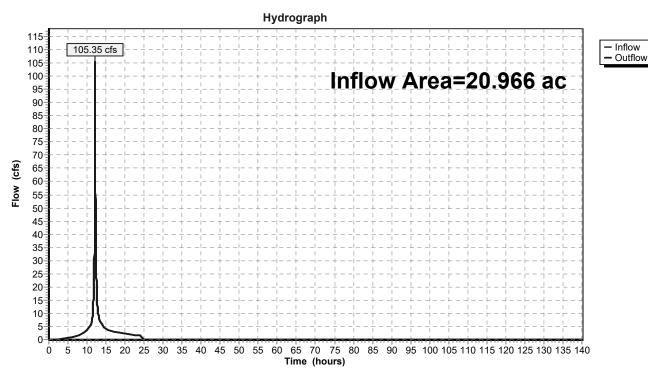
Inflow Area = 20.966 ac, 58.26% Impervious, Inflow Depth = 4.51" for 25-yr event

Inflow = 105.35 cfs @ 12.08 hrs, Volume= 7.874 af

Outflow = 105.35 cfs @ 12.08 hrs, Volume= 7.874 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 44

Summary for Reach DP-3: Wetland DP-3

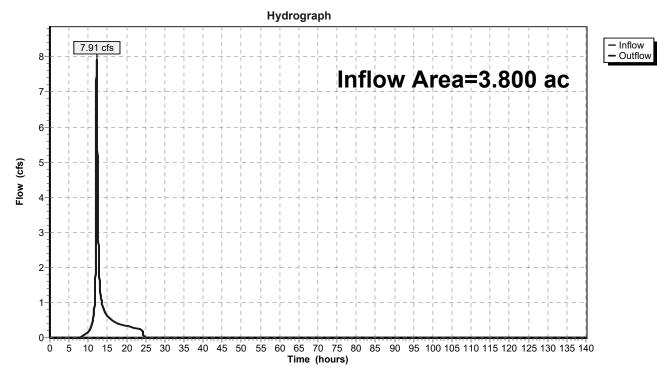
Inflow Area = 3.800 ac, 0.00% Impervious, Inflow Depth = 2.84" for 25-yr event

Inflow = 7.91 cfs @ 12.21 hrs, Volume= 0.899 af

Outflow = 7.91 cfs @ 12.21 hrs, Volume= 0.899 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 45

Summary for Reach DP-4: Wetland DP-4

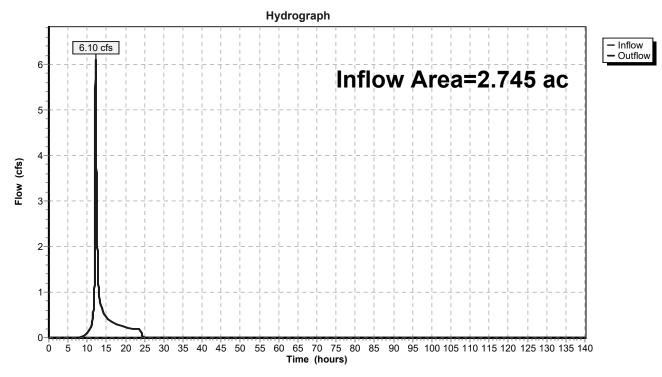
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 2.74" for 25-yr event

Inflow = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af

Outflow = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 46

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 17.167 ac, 71.15% Impervious, Inflow Depth = 4.88" for 25-yr event

Inflow = 99.88 cfs @ 12.06 hrs, Volume= 6.974 af

Outflow = 99.04 cfs @ 12.08 hrs, Volume= 6.974 af, Atten= 1%, Lag= 1.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 9.70 fps, Min. Travel Time= 0.7 min Avg. Velocity = 2.49 fps, Avg. Travel Time= 2.6 min

Peak Storage= 4,049 cf @ 12.07 hrs Average Depth at Peak Storage= 0.89' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

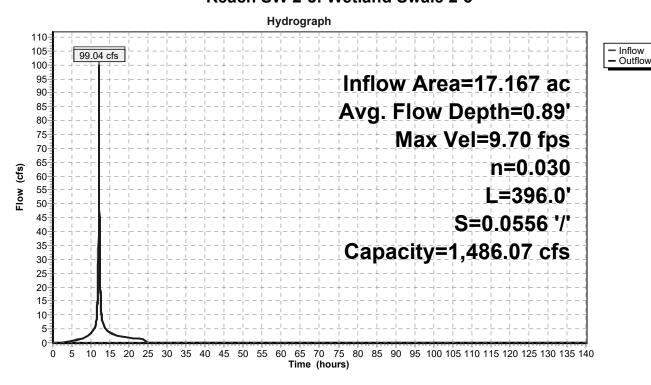
10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



C-DAT-13C4718-EXISTING HY CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 47

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 2.74" for 25-yr event

Inflow = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af

Outflow = 6.06 cfs @ 12.25 hrs, Volume= 0.626 af, Atten= 1%, Lag= 2.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.47 fps, Min. Travel Time= 1.7 min Avg. Velocity = 1.44 fps, Avg. Travel Time= 4.0 min

Peak Storage= 601 cf @ 12.22 hrs Average Depth at Peak Storage= 0.17' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

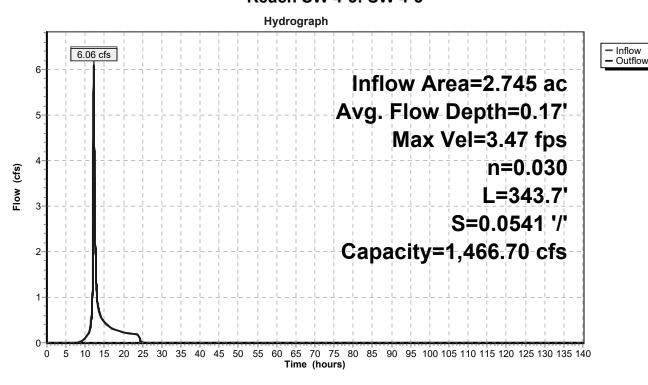
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 48

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth = 4.88" for 25-yr event

Inflow 83.26 cfs @ 12.15 hrs, Volume= 7.843 af

2.31 cfs @ 18.82 hrs, Volume= Outflow 7.273 af, Atten= 97%, Lag= 399.9 min

Primary 2.31 cfs @ 18.82 hrs, Volume= 7.273 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 97.11' @ 18.82 hrs Surf.Area= 46,122 sf Storage= 250,580 cf

Plug-Flow detention time= 1,826.0 min calculated for 7.272 af (93% of inflow)

Center-of-Mass det. time= 1,784.8 min (2,586.4 - 801.6)

Volume	Inve	ert Avail.Sto	rage Storage	Description			
#1	90.0	0' 396,47	79 cf Custom	n Stage Data (P	rismatic)Listed below (Recalc)		
Elevation (feet)		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)			
90.0	00	19,765	0	0			
92.0	00	31,993	51,758	51,758			
94.0	00	37,305	69,298	121,056			
96.0	00	42,927	80,232	201,288			
98.0	00	48,699	91,626	292,914			
100.0	00	54,866	103,565	396,479			
Device	Routing	Invert	Outlet Device	es			
#1	Primary	88.00'	18.0" Round	d Culvert			
					headwall, Ke= 0.500		
			Inlet / Outlet I	Invert= 88.00' / 8	34.50' S= 0.0493 '/' Cc= 0.900		
					ooth interior, Flow Area= 1.77 sf		
#2	Device 1	91.00'		ifice/Grate C=			
#3	Device 1	95.00'	6.0" Vert. Orifice/Grate C= 0.600				

6.0" Vert. Orifice/Grate C= 0.600

Limited to weir flow at low heads

36.0" x 78.0" Horiz. Orifice/Grate C= 0.600

Primary OutFlow Max=2.31 cfs @ 18.82 hrs HW=97.11' (Free Discharge)

-1=Culvert (Passes 2.31 cfs of 24.60 cfs potential flow)

98.00'

99.00'

#4

#5

Device 1

Device 1

²⁼Orifice/Grate (Orifice Controls 1.02 cfs @ 11.74 fps)

⁻³⁼Orifice/Grate (Orifice Controls 1.29 cfs @ 6.56 fps)

⁻⁴⁼Orifice/Grate (Controls 0.00 cfs)

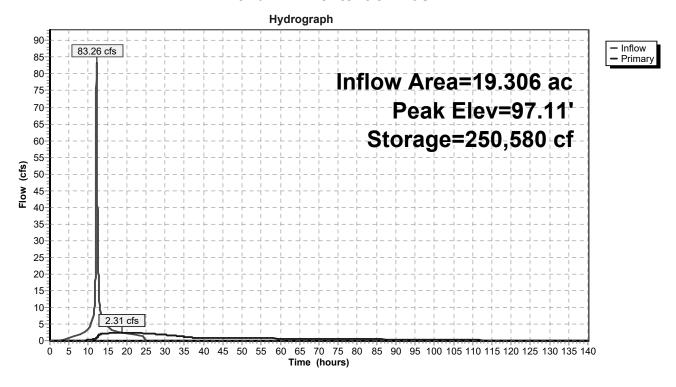
⁻⁵⁼Orifice/Grate (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 49

Pond P-7: Dentention Basin 7



C-DAT-13C4718-EXISTING HYCT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 50

Time span=0.00-140.00 hrs, dt=0.01 hrs, 14001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentEDA-1: Area to Detention Runoff Area=840,987 sf 70.56% Impervious Runoff Depth=6.58" Tc=15.0 min CN=90 Runoff=109.41 cfs 10.585 af

SubcatchmentEDA-2: Area to Wetland Runoff Area=747,775 sf 71.15% Impervious Runoff Depth=6.58" Flow Length=1,211' Tc=8.0 min CN=90 Runoff=131.15 cfs 9.412 af

SubcatchmentEDA-3: Area to Wetland DP-3 Runoff Area=45,946 sf 0.00% Impervious Runoff Depth=4.61" Flow Length=347' Tc=9.3 min CN=73 Runoff=5.55 cfs 0.405 af

SubcatchmentEDA-4: Area to Wetland

Runoff Area=119,565 sf 0.00% Impervious Runoff Depth=4.15"
Flow Length=808' Tc=18.1 min CN=69 Runoff=9.28 cfs 0.950 af

Reach DP-1: Detention Basin 7 Inflow=3.59 cfs 10.002 af

Outflow=3.59 cfs 10.002 af

Reach DP-2: Wetland DP-2 Inflow=140.06 cfs 10.767 af

Outflow=140.06 cfs 10.767 af

Reach DP-3: Wetland DP-3 Inflow=12.06 cfs 1.355 af

Outflow=12.06 cfs 1.355 af

Reach DP-4: Wetland DP-4 Inflow=9.28 cfs 0.950 af

Outflow=9.28 cfs 0.950 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=1.04' Max Vel=10.62 fps Inflow=131.15 cfs 9.412 af n=0.030 L=396.0' S=0.0556 '/' Capacity=1,486.07 cfs Outflow=130.29 cfs 9.412 af

Reach SW 4-3: SW 4-3Avg. Flow Depth=0.22' Max Vel=4.07 fps Inflow=9.28 cfs 0.950 af

n=0.030 L=343.7' S=0.0541 '/' Capacity=1,466.70 cfs Outflow=9.23 cfs 0.950 af

Pond P-7: Dentention Basin 7 Peak Elev=98.74' Storage=329,940 cf Inflow=109.41 cfs 10.585 af

Outflow=3.59 cfs 10.002 af

Total Runoff Area = 40.273 ac Runoff Volume = 21.352 af Average Runoff Depth = 6.36" 35.84% Pervious = 14.435 ac 64.16% Impervious = 25.837 ac

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 51

Summary for Subcatchment EDA-1: Area to Detention Basin 7

Runoff 109.41 cfs @ 12.15 hrs, Volume= 10.585 af, Depth= 6.58"

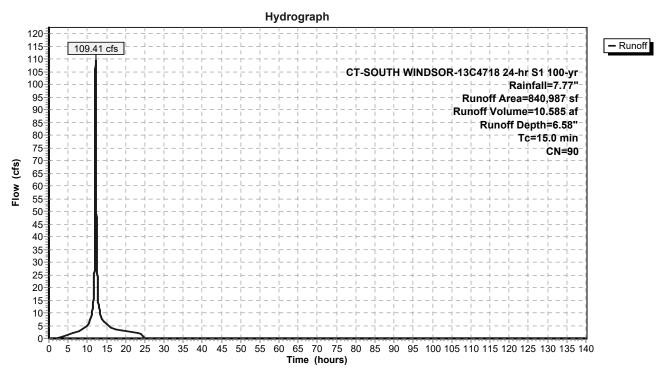
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

	Area (sf)	CN	Description							
	299,131	98	Paved parking, HSG B							
	282,062	98	Paved parking, HSG C							
	11,034	98	Paved parking, HSG B							
	1,191	98	Paved parking, HSG C							
	180,158	69	50-75% Grass cover, Fair, HSG B							
	59,799	79	50-75% Grass cover, Fair, HSG C							
	7,145	69	50-75% Grass cover, Fair, HSG B							
	467	79	50-75% Grass cover, Fair, HSG C							
	840,987 90 Weighted Average									
247,569 29.44% Pervious Area										
593,418 70.56% Impervious Area										
	Tc Length	Slop	pe Velocity Capacity Description							
(m	in) (feet)	(ft/	ft) (ft/sec) (cfs)							

15.0

Direct Entry, Master Planned Tc

Subcatchment EDA-1: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 52

Summary for Subcatchment EDA-2: Area to Wetland DP-2

Runoff = 131.15 cfs @ 12.06 hrs, Volume= 9.412 af, Depth= 6.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

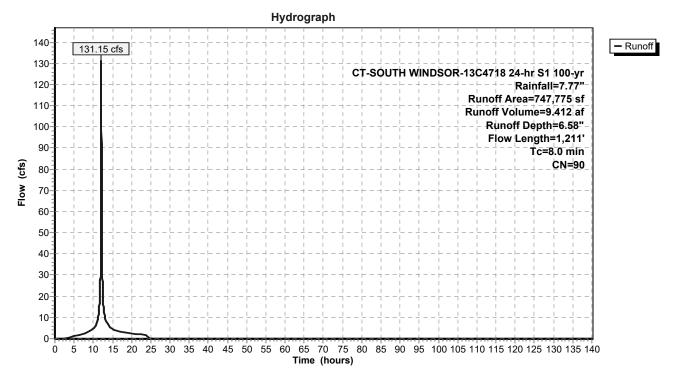
A	rea (sf)	CN E	escription						
5	17,459	98 F	98 Paved parking, HSG B						
	10,227	98 F							
	4,362	98 F	aved park	ing, HSG D					
2	13,896	69 5	0-75% Gra	ass cover, I	Fair, HSG B				
	588	79 5	0-75% Gra	ass cover, I	Fair, HSG C				
	1,243	84 5	0-75% Gra	ass cover, l	Fair, HSG D				
7	47,775	90 V	Veighted A	verage					
2	15,727	2	8.85% Per	vious Area					
5	32,048	7	1.15% lmp	pervious Ar	ea				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
3.6	74	0.1350	0.34		Sheet Flow, 1				
					Grass: Short n= 0.150 P2= 3.11"				
0.4	26	0.0250	1.13		Sheet Flow, 2				
					Smooth surfaces n= 0.011 P2= 3.11"				
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3				
					Paved Kv= 20.3 fps				
1.7	744	0.0050	7.35	23.11	• • • • • • • • • • • • • • • • • • •				
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'				
4.0	454	0.0000	0.40		n= 0.009 Corrugated PE, smooth interior				
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4				
					Grassed Waterway Kv= 15.0 fps				
8.0	1,211	Total							

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 53

Subcatchment EDA-2: Area to Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 54

Summary for Subcatchment EDA-3: Area to Wetland DP-3

Runoff = 5.55 cfs @ 12.08 hrs, Volume= 0.405 af, Depth= 4.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

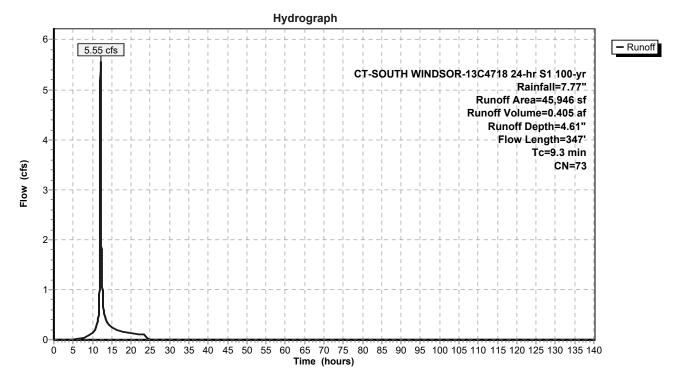
_	Α	rea (sf)	CN	CN Description							
		0	98	98 Paved parking, HSG B							
		0	98	Paved park	ing, HSG C						
		0	98	Paved park	ing, HSG D						
		21,004	69	50-75% Grass cover, Fair, HSG B							
		0	79	9 50-75% Grass cover, Fair, HSG C							
		5,451	84	84 50-75% Grass cover, Fair, HSG D							
		2,225		Brush, Fair,							
_		17,266	77	Brush, Fair,	HSG D						
		45,946	73	Weighted A	verage						
		45,946		100.00% Pe	ervious Are	a					
	_										
	Tc	Length	Slope			Description					
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	7.5	100	0.0400	0.22		Sheet Flow, 1					
						Grass: Short n= 0.150 P2= 3.11"					
	0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2					
						Grassed Waterway Kv= 15.0 fps					
	1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3					
_						Woodland Kv= 5.0 fps					
	9.3	347	Total								

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 55

Subcatchment EDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 56

Summary for Subcatchment EDA-4: Area to Wetland DP-4

Runoff = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af, Depth= 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

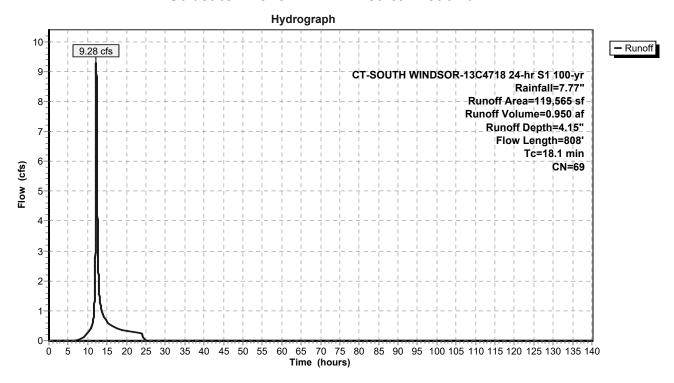
	rea (sf)	CN E	escription				
	0	98 F	aved park	ing, HSG E	3		
	0	98 F	aved park	ing, HSG C			
	0	98 F	aved park	ing, HSG D			
	57,679	69 5	0-75% Gra	ass cover, I	Fair, HSG B		
	26,837	79 5	0-75% Gra	ass cover, I	Fair, HSG C		
	0	84 5	0-75% Gra	ass cover, I	Fair, HSG D		
	25,526	56 E	Brush, Fair,	, HSG B			
	9,523	70 E	Brush, Fair,	, HSG C			
,	119,565	69 V	Veighted A	verage			
	119,565	1	100.00% Pervious Area				
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.7	100	0.0800	0.29		Sheet Flow, 1		
					Grass: Short n= 0.150 P2= 3.11"		
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2		
					Grassed Waterway Kv= 15.0 fps		
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3		
					Woodland Kv= 5.0 fps		
0.1	74	0.0270	19.82	194.19	•		
					Area= 9.8 sf Perim= 15.7' r= 0.62'		
					n= 0.009 Corrugated PE, smooth interior		
18.1	808	Total					

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 57

Subcatchment EDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 58

Summary for Reach DP-1: Detention Basin 7

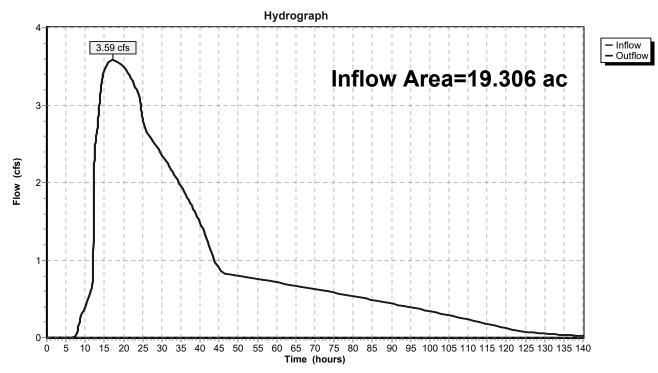
Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth > 6.22" for 100-yr event

Inflow = 3.59 cfs @ 17.35 hrs, Volume= 10.002 af

Outflow = 3.59 cfs @ 17.35 hrs, Volume= 10.002 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 59

Summary for Reach DP-2: Wetland DP-2

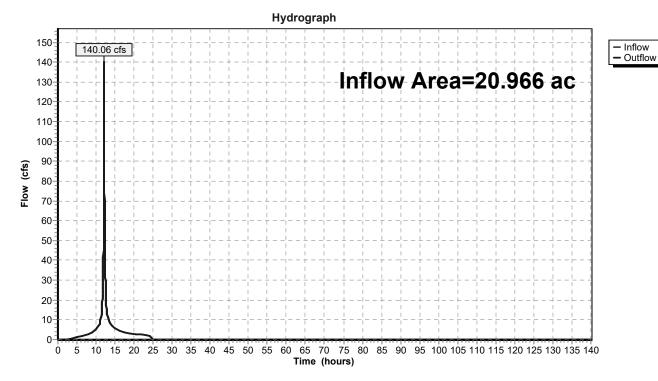
Inflow Area = 20.966 ac, 58.26% Impervious, Inflow Depth = 6.16" for 100-yr event

Inflow = 140.06 cfs @ 12.08 hrs, Volume= 10.767 af

Outflow = 140.06 cfs @ 12.08 hrs, Volume= 10.767 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 60

Summary for Reach DP-3: Wetland DP-3

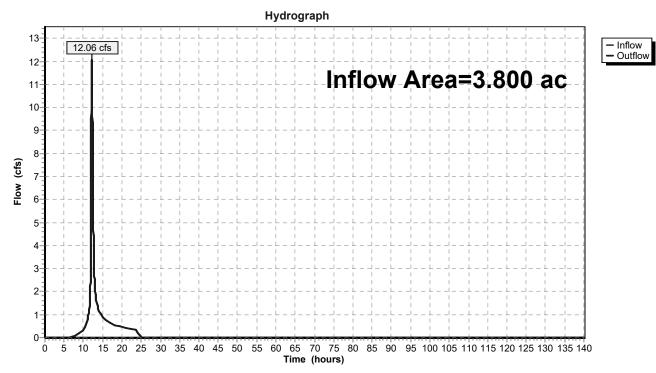
Inflow Area = 3.800 ac, 0.00% Impervious, Inflow Depth = 4.28" for 100-yr event

Inflow = 12.06 cfs @ 12.20 hrs, Volume= 1.355 af

Outflow = 12.06 cfs @ 12.20 hrs, Volume= 1.355 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 61

Summary for Reach DP-4: Wetland DP-4

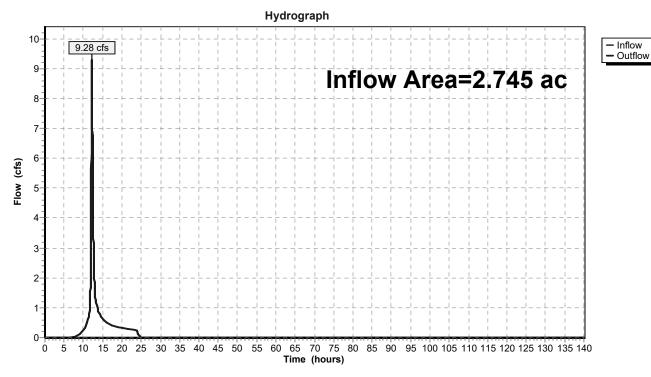
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 4.15" for 100-yr event

Inflow = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af

Outflow = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 62

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 17.167 ac, 71.15% Impervious, Inflow Depth = 6.58" for 100-yr event

Inflow = 131.15 cfs @ 12.06 hrs, Volume= 9.412 af

Outflow = 130.29 cfs @ 12.08 hrs, Volume= 9.412 af, Atten= 1%, Lag= 1.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 10.62 fps, Min. Travel Time= 0.6 min Avg. Velocity = 2.76 fps, Avg. Travel Time= 2.4 min

Peak Storage= 4,855 cf @ 12.07 hrs Average Depth at Peak Storage= 1.04' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

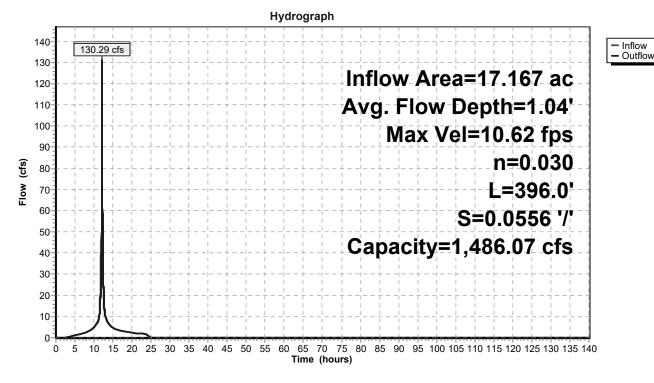
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 63

Inflow

Outflow

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 4.15" for 100-yr event

Inflow = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af

Outflow = 9.23 cfs @ 12.24 hrs, Volume= 0.950 af, Atten= 1%, Lag= 2.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 4.07 fps, Min. Travel Time= 1.4 min Avg. Velocity = 1.49 fps, Avg. Travel Time= 3.8 min

Peak Storage= 780 cf @ 12.22 hrs Average Depth at Peak Storage= 0.22' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

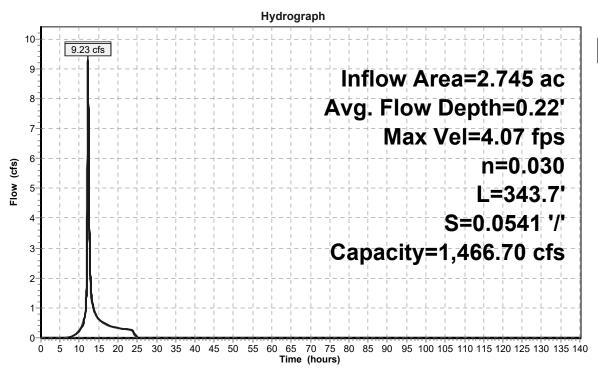
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 64

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.306 ac, 70.56% Impervious, Inflow Depth = 6.58" for 100-yr event

Inflow 109.41 cfs @ 12.15 hrs, Volume= 10.585 af

3.59 cfs @ 17.35 hrs, Volume= Outflow 10.002 af, Atten= 97%, Lag= 312.2 min

Primary 3.59 cfs @ 17.35 hrs, Volume= 10.002 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 98.74' @ 17.35 hrs Surf.Area= 50,990 sf Storage= 329,940 cf

Plug-Flow detention time= 1,719.0 min calculated for 10.001 af (94% of inflow)

Center-of-Mass det. time= 1,686.5 min (2,478.6 - 792.1)

Volume	ln۱	∕ert Avai	l.Storage	Storage	Description	
#1	90.	.00' 3	96,479 cf	Custom	n Stage Data (Pi	rismatic)Listed below (Recalc)
Elevation (fee		Surf.Area (sq-ft)		Store c-feet)	Cum.Store (cubic-feet)	
90.0	00	19,765		0	0	
92.0	00	31,993	Į	51,758	51,758	
94.0	00	37,305	(39,298	121,056	
96.0	00	42,927	8	30,232	201,288	
98.0	00	48,699	(91,626	292,914	
100.0	00	54,866	10	03,565	396,479	
Device	Routing	ı İn	vert Outl	et Device	es	
#1	Primary	88			d Culvert IP, square edge l	headwall, Ke= 0.500

DEVICE	rtouting	IIIVEIL	Outlet Devices
#1	Primary	88.00'	18.0" Round Culvert
	•		L= 71.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 88.00' / 84.50' S= 0.0493 '/' Cc= 0.900
			n= 0.009 Corrugated PE, smooth interior, Flow Area= 1.77 sf
#2	Device 1	91.00'	4.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	95.00'	6.0" Vert. Orifice/Grate C= 0.600
#4	Device 1	98.00'	6.0" Vert. Orifice/Grate C= 0.600
#5	Device 1	99.00'	36.0" x 78.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=3.59 cfs @ 17.35 hrs HW=98.74' (Free Discharge)

-1=Culvert (Passes 3.59 cfs of 26.90 cfs potential flow)

2=Orifice/Grate (Orifice Controls 1.16 cfs @ 13.25 fps)

-3=Orifice/Grate (Orifice Controls 1.77 cfs @ 9.00 fps)

-4=Orifice/Grate (Orifice Controls 0.66 cfs @ 3.38 fps)

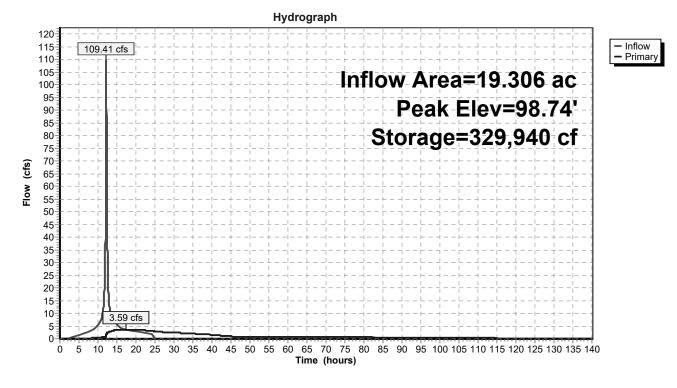
-5=Orifice/Grate (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 65

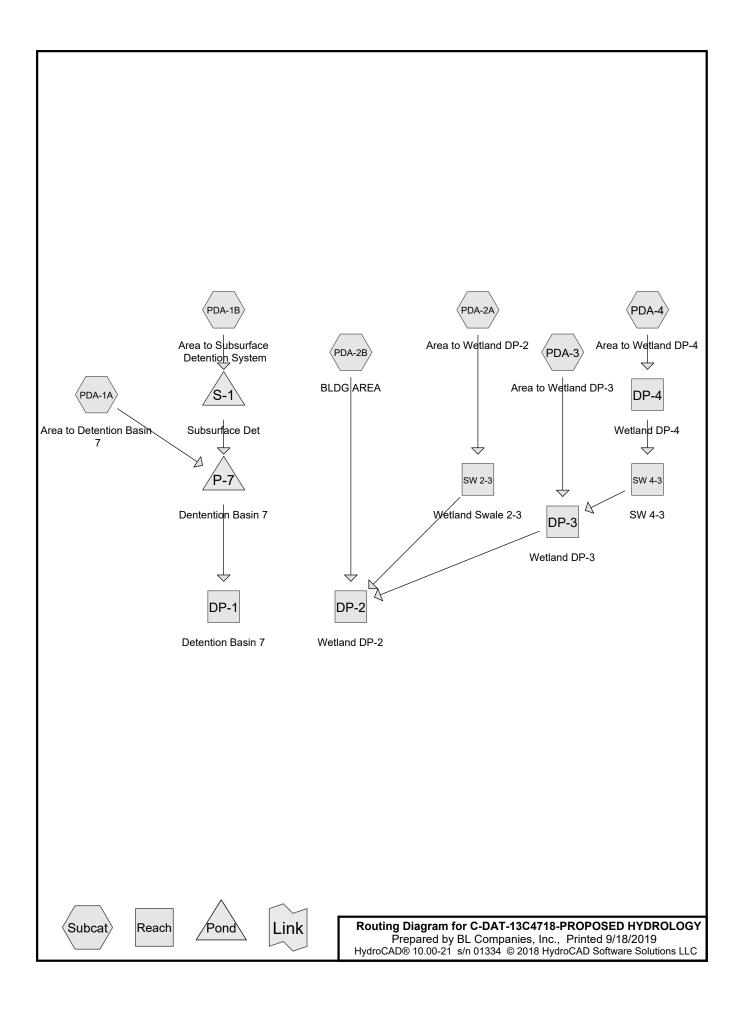
Pond P-7: Dentention Basin 7





APPENDIX C

POST-DEVELOPMENT HYDROLOGY



C-DAT-13C4718-PROPOSED HYCT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 2

Time span=0.00-140.00 hrs, dt=0.01 hrs, 14001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPDA-1A: Area to Detention Runoff Area=505,152 sf 73.61% Impervious Runoff Depth=2.17" Flow Length=1,379' Tc=15.9 min CN=91 Runoff=22.94 cfs 2.101 af

SubcatchmentPDA-1B: Area to Runoff Area=339,526 sf 85.88% Impervious Runoff Depth=2.46"

Tc=5.0 min CN=94 Runoff=28.14 cfs 1.596 af

SubcatchmentPDA-2A: Area to Wetland Runoff Area=470,105 sf 67.01% Impervious Runoff Depth=1.92" Flow Length=1,211' Tc=8.0 min CN=88 Runoff=26.68 cfs 1.723 af

SubcatchmentPDA-2B: BLDG AREA Runoff Area=287,020 sf 85.22% Impervious Runoff Depth=2.46"

Tc=5.0 min CN=94 Runoff=23.79 cfs 1.349 af

SubcatchmentPDA-3: Area to Wetland DP-3 Runoff Area=47,497 sf 0.13% Impervious Runoff Depth=0.98" Flow Length=347' Tc=9.3 min CN=74 Runoff=1.18 cfs 0.089 af

SubcatchmentPDA-4: Area to WetlandRunoff Area=119,565 sf 0.00% Impervious Runoff Depth=0.73"
Flow Length=808' Tc=18.1 min CN=69 Runoff=1.42 cfs 0.167 af

Reach DP-1: Detention Basin 7 Inflow=0.68 cfs 2.769 af

Outflow=0.68 cfs 2.769 af

Reach DP-2: Wetland DP-2 Inflow=44.30 cfs 3.328 af

Outflow=44.30 cfs 3.328 af

Reach DP-3: Wetland DP-3 Inflow=1.88 cfs 0.256 af

Outflow=1.88 cfs 0.256 af

Reach DP-4: Wetland DP-4 Inflow=1.42 cfs 0.167 af

Outflow=1.42 cfs 0.167 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=0.41' Max Vel=6.06 fps Inflow=26.68 cfs 1.723 af

n=0.030 L=396.0' S=0.0556'/' Capacity=1,486.07 cfs Outflow=26.22 cfs 1.723 af

Reach SW 4-3: SW 4-3 Avg. Flow Depth=0.07' Max Vel=1.99 fps Inflow=1.42 cfs 0.167 af

n=0.030 L=343.7' S=0.0541'/' Capacity=1,466.70 cfs Outflow=1.39 cfs 0.167 af

Pond P-7: Dentention Basin 7 Peak Elev=93.82' Storage=79,955 cf Inflow=23.14 cfs 3.128 af

Outflow=0.68 cfs 2.769 af

Pond S-1: Subsurface Det Peak Elev=102.68' Storage=53,965 cf Inflow=28.14 cfs 1.596 af

Outflow=0.39 cfs 1.028 af

Total Runoff Area = 40.608 ac Runoff Volume = 7.025 af Average Runoff Depth = 2.08" 30.85% Pervious = 12.529 ac 69.15% Impervious = 28.078 ac

Page 3

Summary for Subcatchment PDA-1A: Area to Detention Basin 7

Runoff = 22.94 cfs @ 12.17 hrs, Volume= 2.101 af, Depth= 2.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

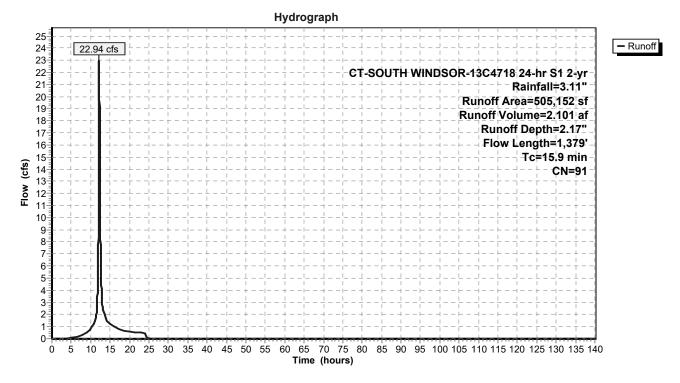
A	rea (sf)	CN E	escription		
	82,516	98 F	aved park	ing, HSG B	
2	75,911	98 F	aved park	ing, HSG C	
	12,219	98 F	aved park	ing, HSG B	
	1,191	98 F	Paved park	ing, HSG C	
	78,680	69 5	0-75% Gra	ass cover, F	Fair, HSG B
	41,252	79 5	0-75% Gra	ass cover, F	Fair, HSG C
	10,535				Fair, HSG B
	467		0-75% Gra	ass cover, F	Fair, HSG C
	0		Gravel road		
	389		Fravel road		
	1,992		Gravel road		
	0		Gravel road		
	05,152		Veighted A		
	33,315			vious Area	
3	71,837	7	3.61% lmp	ervious Ar	ea
_		01			D 10
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.9	100	0.0900	0.21		Sheet Flow,
۰-	0.40	0.0400	4 = 0		Grass: Dense n= 0.240 P2= 3.11"
2.7	249	0.0480	1.53		Shallow Concentrated Flow,
4.0	000	0.0070	4.00		Short Grass Pasture Kv= 7.0 fps
4.8	383	0.0078	1.32		Shallow Concentrated Flow, swale
0.5	0.47	0.0400	00.00	05.05	Grassed Waterway Kv= 15.0 fps
0.5	647	0.0400	20.80	65.35	Pipe Channel,
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
45.0	4.070	T.4.1			n= 0.009 Corrugated PE, smooth interior
15.9	1,379	Total			

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 4

Subcatchment PDA-1A: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 5

Summary for Subcatchment PDA-1B: Area to Subsurface Detention System

Runoff = 28.14 cfs @ 12.03 hrs, Volume= 1.596 af, Depth= 2.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

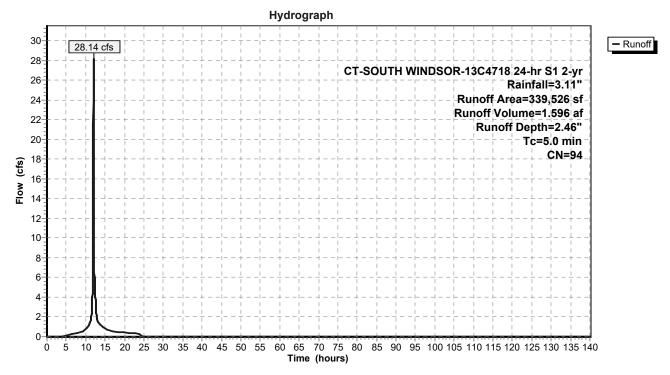
Area (s	f) CN	Description						
269,48	3 98	Paved parking, HSG B						
22,10	0 98	Paved parking, HSG C						
	0 98	Paved parking, HSG B						
	0 98	Paved parking, HSG C						
43,11	3 69	50-75% Grass cover, Fair, HSG B						
4,83	0 79	50-75% Grass cover, Fair, HSG C						
	0 69	50-75% Grass cover, Fair, HSG B						
	0 79	50-75% Grass cover, Fair, HSG C						
	0 85	Gravel roads, HSG B						
	0 89	Gravel roads, HSG C						
	0 85	Gravel roads, HSG B						
	0 89	Gravel roads, HSG C						
339,52	6 94	Weighted Average						
47,94	3	14.12% Pervious Area						
291,58	3	85.88% Impervious Area						
Tc Leng	gth Slo	pe Velocity Capacity Description						
(min) (fee	et) (ft/	ft) (ft/sec) (cfs)						
5.0	•	Direct Entry						

5.0

Direct Entry,

Page 6

Subcatchment PDA-1B: Area to Subsurface Detention System



Page 7

Summary for Subcatchment PDA-2A: Area to Wetland DP-2

Runoff = 26.68 cfs @ 12.06 hrs, Volume= 1.723 af, Depth= 1.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

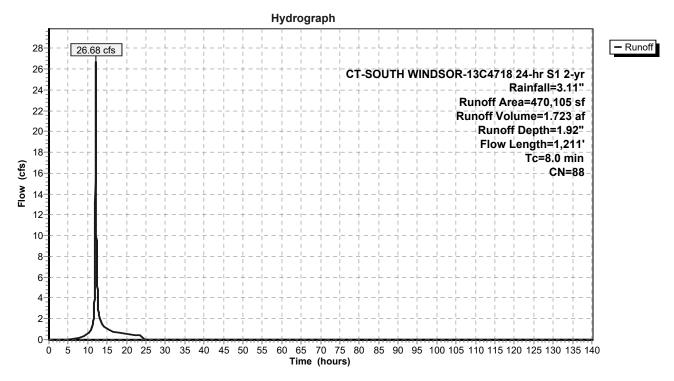
A	rea (sf)	CN E	escription					
3	04,766	98 F	Paved parking, HSG B					
	10,251	98 F	aved park	ing, HSG C				
	0	98 F	aved park	ing, HSG D				
1	54,500	69 5	0-75% Gra	ass cover, F	Fair, HSG B			
	588			,	Fair, HSG C			
	0	84 5	0-75% Gra	ass cover, F	Fair, HSG D			
4	70,105		Veighted A					
	55,088			vious Area				
3	15,017	6	7.01% lmp	ervious Ar	ea			
_				_				
Tc	Length	Slope	Velocity		Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.6	74	0.1350	0.34		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.4	26	0.0250	1.13		Sheet Flow, 2			
	040	0.0050	0.04		Smooth surfaces n= 0.011 P2= 3.11"			
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3			
17	711	0.0050	7.05	00.44	Paved Kv= 20.3 fps			
1.7	744	0.0050	7.35	23.11	Pipe Channel, RCP_Round 24" 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'			
					n= 0.009 Corrugated PE, smooth interior			
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4			
1.2	101	0.0200	2.12		Grassed Waterway Kv= 15.0 fps			
8.0	1,211	Total			5.45554 **A.51**4y 1(* 15.5 lpc			
0.0	1,411	i Otai						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 8

Subcatchment PDA-2A: Area to Wetland DP-2



Page 9

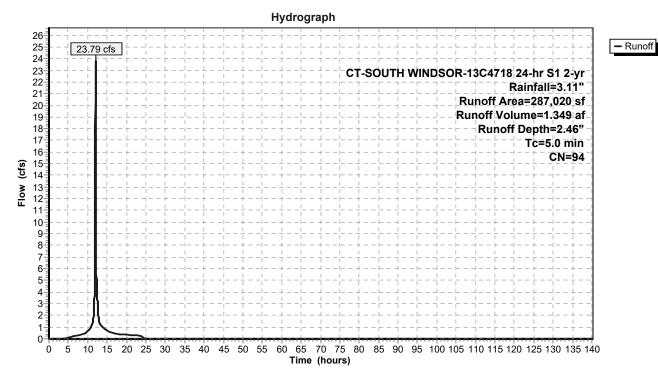
Summary for Subcatchment PDA-2B: BLDG AREA

Runoff = 23.79 cfs @ 12.03 hrs, Volume= 1.349 af, Depth= 2.46"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

Are	ea (sf)	CN	Description				
24	0,618	98	Paved park	ing, HSG B	3		
	0	98	Paved park	ing, HSG C			
;	3,973	98	Paved park	ing, HSG D			
39	9,809	69	50-75% Gra	ass cover, F	Fair, HSG B		
	0	79	50-75% Gra	ass cover, F	Fair, HSG C		
	2,620	84	50-75% Gra	ass cover, F	Fair, HSG D		
28	7,020	94	Weighted A	verage			
4:	2,429		14.78% Per	vious Area	a e e e e e e e e e e e e e e e e e e e		
24	4,591	85.22% Impervious Area					
Tc I	Length	Slope	,	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.0					Direct Entry,		

Subcatchment PDA-2B: BLDG AREA



Page 10

Summary for Subcatchment PDA-3: Area to Wetland DP-3

Runoff = 1.18 cfs @ 12.08 hrs, Volume= 0.089 af, Depth= 0.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

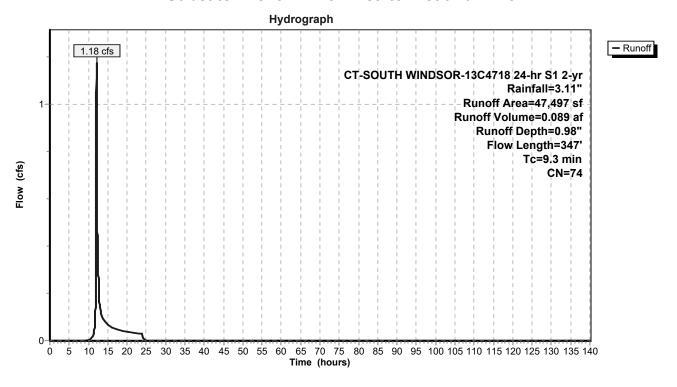
A	rea (sf)	CN [Description		
	63	98 F	Paved park	ing, HSG E	3
	0	98 F	Paved park	ing, HSG C	
	0	98 F	Paved park	ing, HSG D	
	20,845	69 5	0-75% Gra	ass cover, I	Fair, HSG B
	0	79 5	0-75% Gra	ass cover, I	Fair, HSG C
	7,654	84 5	0-75% Gra	ass cover, I	Fair, HSG D
	2,047	56 E	Brush, Fair,	HSG B	
	16,888	77 E	Brush, Fair,	HSG D	
	47,497	74 V	Veighted A	verage	
	47,434	g	9.87% Per	vious Area	l
	63	C).13% Impe	ervious Are	a
_				_	
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.5	100	0.0400	0.22		Sheet Flow, 1
					Grass: Short n= 0.150 P2= 3.11"
0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2
					Grassed Waterway Kv= 15.0 fps
1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3
					Woodland Kv= 5.0 fps
9.3	347	Total			

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 11

Subcatchment PDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 12

Summary for Subcatchment PDA-4: Area to Wetland DP-4

Runoff = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af, Depth= 0.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

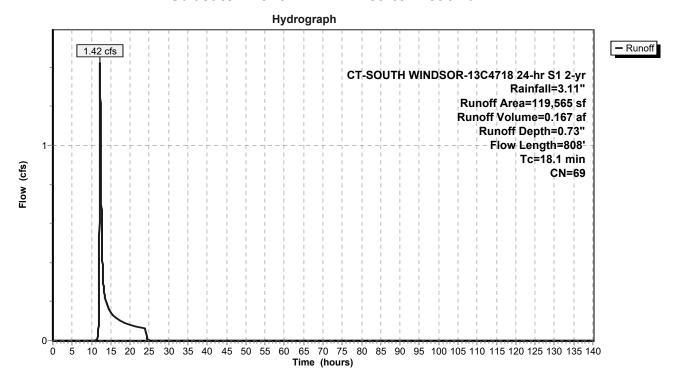
_	Aı	rea (sf)	CN D	escription					
		0	98 P	98 Paved parking, HSG B					
		0			ing, HSG C				
		0	98 P	aved park	ing, HSG D)			
		57,679				Fair, HSG B			
		26,837				Fair, HSG C			
		0				Fair, HSG D			
		25,526		rush, Fair		, -			
		9,523		rush, Fair,					
_	1	19,565		Veighted A					
		19,565		•	ervious Are	a a			
	•	10,000	•	00.00701	31 11000 7 110				
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	2			
_	5.7	100	0.0800	0.29	, ,	Sheet Flow, 1			
	0.7	100	0.0000	0.20		Grass: Short n= 0.150 P2= 3.11"			
	0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2			
	0.2	00	0.0000	7.27		Grassed Waterway Kv= 15.0 fps			
	12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3			
	12.1	000	0.0270	0.02		Woodland Kv= 5.0 fps			
	0.1	74	0.0270	19.82	194.19	• • • • • • • • • • • • • • • • • • •			
	0.1	, ,	0.0210	10.02	104.10	Area= 9.8 sf Perim= 15.7' r= 0.62'			
						n= 0.009 Corrugated PE, smooth interior			
_	18.1	909	Total			11- 0.000 Corragated FE, Sillootti litterioi			
	10.1	808	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 13

Subcatchment PDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 14

Summary for Reach DP-1: Detention Basin 7

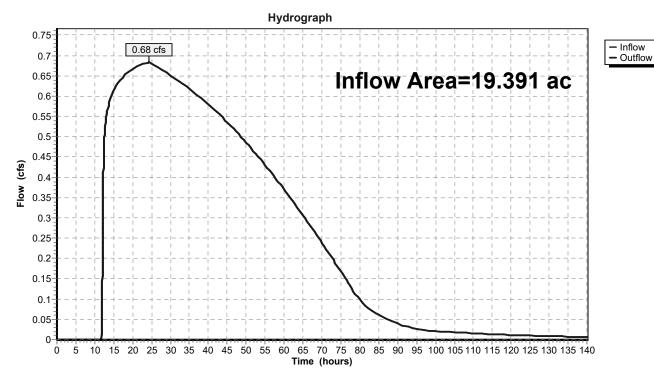
Inflow Area = 19.391 ac, 78.54% Impervious, Inflow Depth > 1.71" for 2-yr event

Inflow = 0.68 cfs @ 24.19 hrs, Volume= 2.769 af

Outflow = 0.68 cfs @ 24.19 hrs, Volume= 2.769 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



C-DAT-13C4718-PROPOSED HYCT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 15

Summary for Reach DP-2: Wetland DP-2

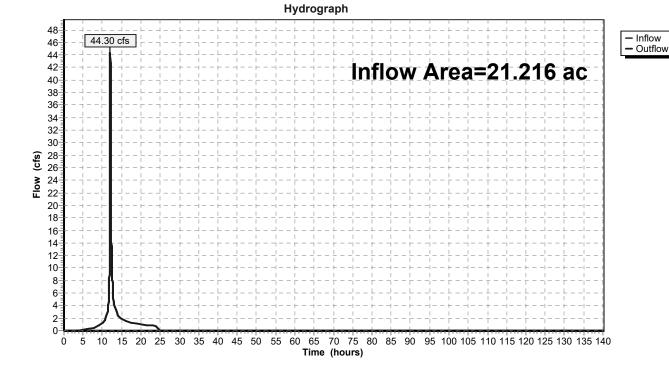
Inflow Area = 21.216 ac, 60.56% Impervious, Inflow Depth = 1.88" for 2-yr event

Inflow = 44.30 cfs @ 12.05 hrs, Volume= 3.328 af

Outflow = 44.30 cfs @ 12.05 hrs, Volume= 3.328 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 16

Summary for Reach DP-3: Wetland DP-3

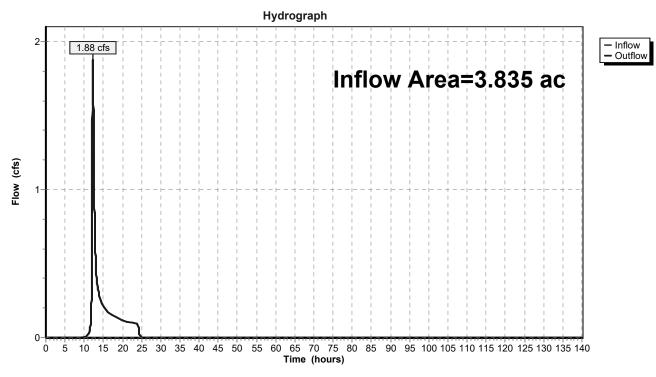
Inflow Area = 3.835 ac, 0.04% Impervious, Inflow Depth = 0.80" for 2-yr event

Inflow = 1.88 cfs @ 12.27 hrs, Volume= 0.256 af

Outflow = 1.88 cfs @ 12.27 hrs, Volume= 0.256 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 17

Summary for Reach DP-4: Wetland DP-4

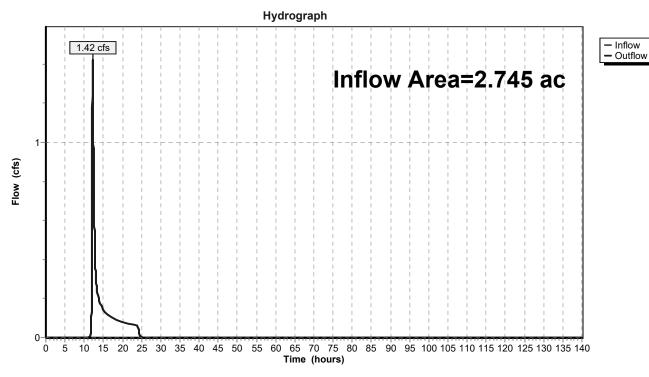
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 0.73" for 2-yr event

Inflow = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af

Outflow = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 18

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 10.792 ac, 67.01% Impervious, Inflow Depth = 1.92" for 2-yr event

Inflow = 26.68 cfs @ 12.06 hrs, Volume= 1.723 af

Outflow = 26.22 cfs @ 12.09 hrs, Volume= 1.723 af, Atten= 2%, Lag= 1.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 6.06 fps, Min. Travel Time= 1.1 min Avg. Velocity = 1.66 fps, Avg. Travel Time= 4.0 min

Peak Storage= 1,716 of @ 12.07 hrs Average Depth at Peak Storage= 0.41'

Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

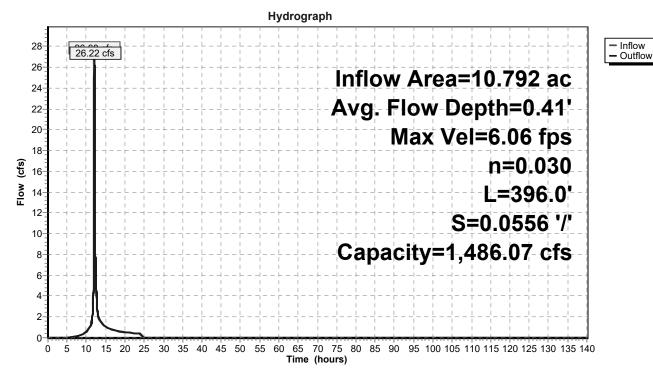
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 19

InflowOutflow

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 0.73" for 2-yr event

Inflow = 1.42 cfs @ 12.23 hrs, Volume= 0.167 af

Outflow = 1.39 cfs @ 12.31 hrs, Volume= 0.167 af, Atten= 3%, Lag= 5.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 1.99 fps, Min. Travel Time= 2.9 min Avg. Velocity = 1.36 fps, Avg. Travel Time= 4.2 min

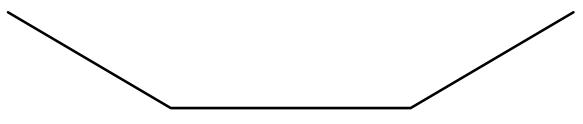
Peak Storage= 240 cf @ 12.26 hrs Average Depth at Peak Storage= 0.07' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

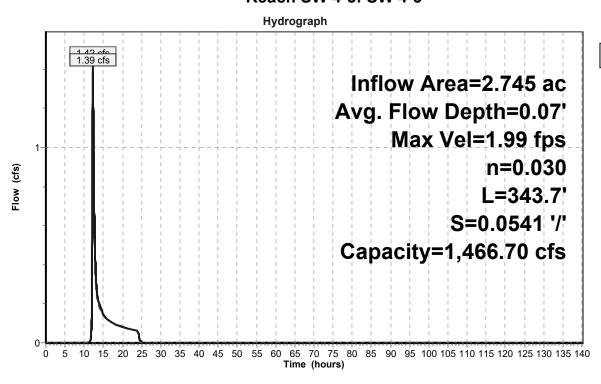
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 20

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.391 ac, 78.54% Impervious, Inflow Depth > 1.94" for 2-yr event

Inflow = 23.14 cfs @ 12.17 hrs, Volume= 3.128 af

Outflow = 0.68 cfs @ 24.19 hrs, Volume= 2.769 af, Atten= 97%, Lag= 721.1 min

Primary = 0.68 cfs @ 24.19 hrs, Volume= 2.769 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 93.82' @ 24.19 hrs Surf.Area= 26,229 sf Storage= 79,955 cf

Plug-Flow detention time= 1,535.6 min calculated for 2.769 af (89% of inflow)

Center-of-Mass det. time= 1,283.0 min (2,497.4 - 1,214.5)

Volume	ln۱	vert Ava	I.Storage	Storage	Description	
#1	90.	00' 2	80,770 cf	Custon	n Stage Data (Pi	rismatic)Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)		c.Store c-feet)	Cum.Store (cubic-feet)	
90.0	00	12,621		0	0	
92.0	00	22,814	;	35,435	35,435	
94.0	94.00 26,576		4	49,390	84,825	
96.0	00	30,501	:	57,077	141,902	
98.0	00	34,640		35,141	207,043	
100.0	00	39,087	•	73,727	280,770	
Device	Routing	ı İn	vert Out	et Device	es	
#1	Primary	88	.00' 18.0	" Round	d Culvert	
	-		Inle	t / Outlet	Invert= 88.00' / 8	headwall, Ke= 0.500 4.50' S= 0.0493'/' Cc= 0.900 ooth interior, Flow Area= 1.77 sf

91.00' **4.0" Vert. Orifice/Grate** C= 0.600 #2 Device 1 **6.0" Vert. Orifice/Grate** C= 0.600 #3 Device 1 95.00' #4 Device 1 98.00' **6.0" Vert. Orifice/Grate** C= 0.600 #5 **36.0" x 78.0" Horiz. Orifice/Grate** C= 0.600 Device 1 99.00' Limited to weir flow at low heads

Primary OutFlow Max=0.68 cfs @ 24.19 hrs HW=93.82' (Free Discharge)

1=Culvert (Passes 0.68 cfs of 19.15 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.68 cfs @ 7.84 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Orifice/Grate (Controls 0.00 cfs)

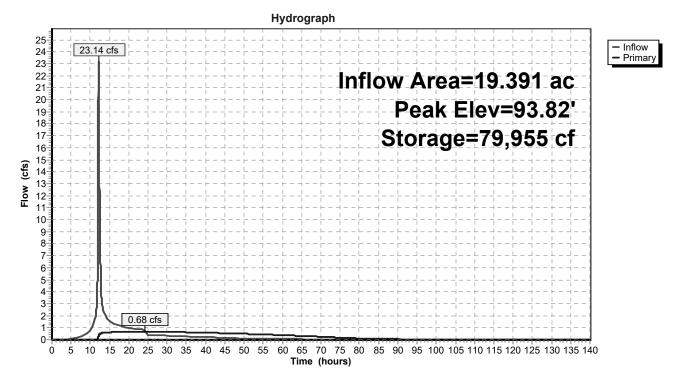
-5=Orifice/Grate (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 21

Pond P-7: Dentention Basin 7



C-DAT-13C4718-PROPOSED HYCT-SOUTH WINDSOR-13C4718 24-hr S1 2-yr Rainfall=3.11"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

<u>Page 22</u>

Summary for Pond S-1: Subsurface Det

Inflow Area = 7.794 ac, 85.88% Impervious, Inflow Depth = 2.46" for 2-yr event

Inflow = 28.14 cfs @ 12.03 hrs, Volume= 1.596 af

Outflow = 0.39 cfs @ 20.56 hrs, Volume= 1.028 af, Atten= 99%, Lag= 512.2 min

Primary = 0.39 cfs @ 20.56 hrs, Volume= 1.028 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 102.68' @ 20.56 hrs Surf.Area= 31,034 sf Storage= 53,965 cf

Plug-Flow detention time= 1,334.7 min calculated for 1.027 af (64% of inflow)

Center-of-Mass det. time= 1,215.4 min (2,011.0 - 795.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	100.25'	49,002 cf	228.33'W x 135.92'L x 6.75'H Field A
			209,482 cf Overall - 86,977 cf Embedded = 122,504 cf x 40.0% Voids
#2A	101.00'	86,977 cf	ADS_StormTech MC-4500 +Capx 800 Inside #1
			Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf
			Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap
			25 Rows of 32 Chambers
			Cap Storage= +35.7 cf x 2 x 25 rows = 1,785.0 cf
		125 070 of	Total Available Ctarage

135,979 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	101.50'	24.0" Round Culvert
	•		L= 200.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 101.50' / 99.50' S= 0.0100 '/' Cc= 0.900
			n= 0.009 Corrugated PE, smooth interior, Flow Area= 3.14 sf
#2	Device 1	101.55'	3.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
#3	Device 1	103.40'	5.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
#4	Device 1	106.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.39 cfs @ 20.56 hrs HW=102.68' (Free Discharge)

1=Culvert (Passes 0.39 cfs of 7.12 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.39 cfs @ 4.72 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 23

Pond S-1: Subsurface Det - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-4500 +Cap (ADS StormTech® MC-4500 with cap volume)

Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap Cap Storage= +35.7 cf x 2 x 25 rows = 1,785.0 cf

100.0" Wide + 9.0" Spacing = 109.0" C-C Row Spacing

32 Chambers/Row x 4.02' Long +2.56' Cap Length x 2 = 133.92' Row Length +12.0" End Stone x 2 = 135.92' Base Length

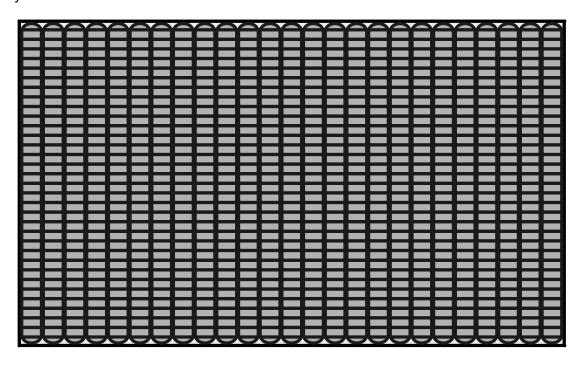
25 Rows x 100.0" Wide + 9.0" Spacing x 24 + 12.0" Side Stone x 2 = 228.33' Base Width 9.0" Base + 60.0" Chamber Height + 12.0" Cover = 6.75' Field Height

800 Chambers x 106.5 cf + 35.7 cf Cap Volume x 2 x 25 Rows = 86,977.3 cf Chamber Storage

209,481.6 cf Field - 86,977.3 cf Chambers = 122,504.2 cf Stone x 40.0% Voids = 49,001.7 cf Stone Storage

Chamber Storage + Stone Storage = 135,979.0 cf = 3.122 af Overall Storage Efficiency = 64.9% Overall System Size = 135.92' x 228.33' x 6.75'

800 Chambers 7,758.6 cy Field 4,537.2 cy Stone



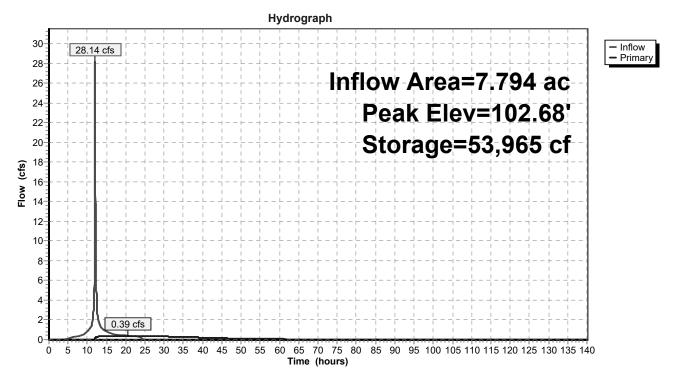


Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 24

Pond S-1: Subsurface Det



C-DAT-13C4718-PROPOSED HCT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 25

Time span=0.00-140.00 hrs. dt=0.01 hrs. 14001 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPDA-1A: Area to Detention Runoff Area=505,152 sf 73.61% Impervious Runoff Depth=3.89" Flow Length=1,379' Tc=15.9 min CN=91 Runoff=39.04 cfs 3.764 af

SubcatchmentPDA-1B: Area to Runoff Area=339,526 sf 85.88% Impervious Runoff Depth=4.22"

Tc=5.0 min CN=94 Runoff=45.45 cfs 2.741 af

SubcatchmentPDA-2A: Area to Wetland Runoff Area=470,105 sf 67.01% Impervious Runoff Depth=3.58" Flow Length=1,211' Tc=8.0 min CN=88 Runoff=47.69 cfs 3.223 af

Runoff Area=287,020 sf 85.22% Impervious Runoff Depth=4.22" SubcatchmentPDA-2B: BLDG AREA

Tc=5.0 min CN=94 Runoff=38.42 cfs 2.317 af

SubcatchmentPDA-3: Area to Wetland DP-3 Runoff Area=47,497 sf 0.13% Impervious Runoff Depth=2.29" Flow Length=347' Tc=9.3 min CN=74 Runoff=2.88 cfs 0.208 af

Runoff Area=119,565 sf 0.00% Impervious Runoff Depth=1.89" SubcatchmentPDA-4: Area to Wetland Flow Length=808' Tc=18.1 min CN=69 Runoff=4.16 cfs 0.433 af

Reach DP-1: Detention Basin 7 Inflow=1.71 cfs 5.564 af Outflow=1.71 cfs 5.564 af

Reach DP-2: Wetland DP-2 Inflow=79.63 cfs 6.181 af Outflow=79.63 cfs 6.181 af

Reach DP-3: Wetland DP-3 Inflow=5.49 cfs 0.641 af Outflow=5.49 cfs 0.641 af

Reach DP-4: Wetland DP-4 Inflow=4.16 cfs 0.433 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=0.57' Max Vel=7.49 fps Inflow=47.69 cfs 3.223 af n=0.030 L=396.0' S=0.0556'/' Capacity=1,486.07 cfs Outflow=47.06 cfs 3.223 af

Reach SW 4-3: SW 4-3 Avg. Flow Depth=0.13' Max Vel=3.00 fps Inflow=4.16 cfs 0.433 af

n=0.030 L=343.7' S=0.0541'/ Capacity=1,466.70 cfs Outflow=4.12 cfs 0.433 af

Pond P-7: Dentention Basin 7 Peak Elev=95.95' Storage=140,381 cf Inflow=39.50 cfs 5.932 af

Outflow=1.71 cfs 5.564 af

Outflow=4.16 cfs 0.433 af

Pond S-1: Subsurface Det Peak Elev=103.98' Storage=85,981 cf Inflow=45.45 cfs 2.741 af

Outflow=1.03 cfs 2.168 af

Total Runoff Area = 40.608 ac Runoff Volume = 12.685 af Average Runoff Depth = 3.75" 30.85% Pervious = 12.529 ac 69.15% Impervious = 28.078 ac

Page 26

Summary for Subcatchment PDA-1A: Area to Detention Basin 7

Runoff = 39.04 cfs @ 12.17 hrs, Volume= 3.764 af, Depth= 3.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

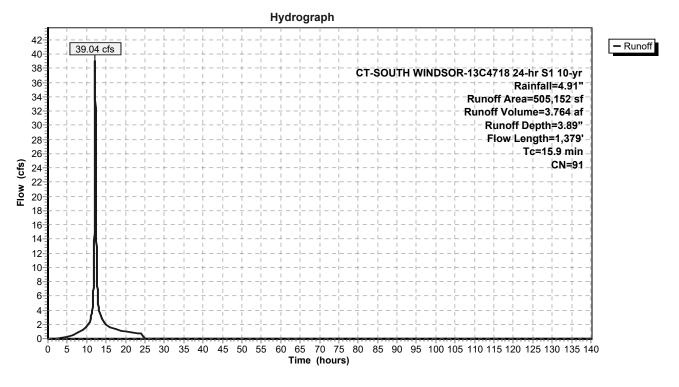
A	rea (sf)	CN D	escription		
	82,516	98 F	aved park	ing, HSG B	
2	275,911	98 F	aved park	ing, HSG C	
	12,219	98 F	aved park	ing, HSG B	
	1,191	98 F	aved park	ing, HSG C	
	78,680	69 5	0-75% Gra	ass cover, I	Fair, HSG B
	41,252	79 5	0-75% Gra	ass cover, f	Fair, HSG C
	10,535				Fair, HSG B
	467				Fair, HSG C
	0		Fravel road		
	389		Fravel road		
	1,992		Fravel road		
	0		Fravel road		
	505,152		Veighted A		
	33,315			vious Area	
3	371,837	7	3.61% Imp	ervious Ar	ea
То	Longth	Clana	\/alaait\	Canacity	Description
Tc (min)	Length (feet)	Slope	Velocity (ft/sec)	Capacity	Description
(min)		(ft/ft)		(cfs)	Oh and Elass
7.9	100	0.0900	0.21		Sheet Flow,
2.7	240	0.0400	4.50		Grass: Dense n= 0.240 P2= 3.11"
2.1	249	0.0480	1.53		Shallow Concentrated Flow,
4.8	383	0.0078	1.32		Short Grass Pasture Kv= 7.0 fps
4.0	303	0.0076	1.32		Shallow Concentrated Flow, swale Grassed Waterway Kv= 15.0 fps
0.5	647	0.0400	20.80	65.35	Pipe Channel,
0.5	047	0.0400	20.00	00.00	24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'
					n= 0.009 Corrugated PE, smooth interior
15.9	1,379	Total			11 0.000 Confugatou i E, omoon monor
10.9	1,019	i Otai			

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 27

Subcatchment PDA-1A: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 28

Summary for Subcatchment PDA-1B: Area to Subsurface Detention System

Runoff = 45.45 cfs @ 12.03 hrs, Volume= 2.741 af, Depth= 4.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

Area (sf)	CN	Description						
269,483	98	Paved parking, HSG B						
22,100	98	Paved parking, HSG C						
0	98	Paved parking, HSG B						
0	98	Paved parking, HSG C						
43,113	69	50-75% Grass cover, Fair, HSG B						
4,830	79	50-75% Grass cover, Fair, HSG C						
0	69	50-75% Grass cover, Fair, HSG B						
0	79	50-75% Grass cover, Fair, HSG C						
0	85	Gravel roads, HSG B						
0	89	Gravel roads, HSG C						
0	85	Gravel roads, HSG B						
0	89	Gravel roads, HSG C						
339,526	94	94 Weighted Average						
47,943		14.12% Pervious Area						
291,583		85.88% Impervious Area						
		·						
Tc Length	Slo	pe Velocity Capacity Description						
(min) (feet)								
		Direct Entry						

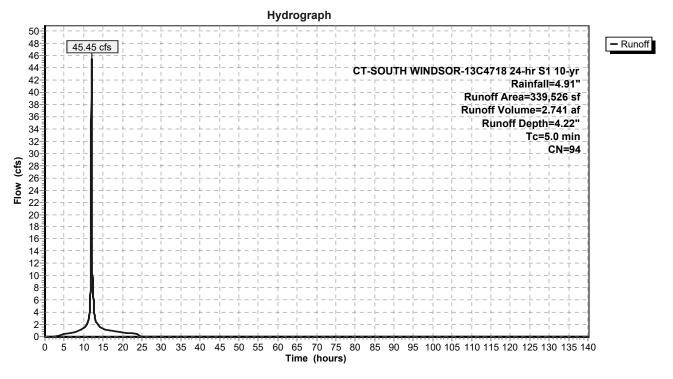
5.0 Direct Entry,

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 29

Subcatchment PDA-1B: Area to Subsurface Detention System



Page 30

Summary for Subcatchment PDA-2A: Area to Wetland DP-2

Runoff = 47.69 cfs @ 12.06 hrs, Volume= 3.223 af, Depth= 3.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

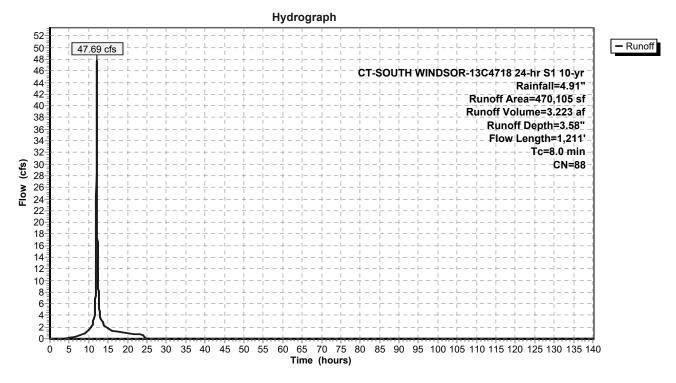
A	rea (sf)	CN E	escription					
3	04,766	98 F	Paved parking, HSG B					
	10,251	98 F	Paved parking, HSG C					
	0	98 F	Paved parking, HSG D					
1	54,500	69 5	50-75% Grass cover, Fair, HSG B					
	588	79 5	50-75% Grass cover, Fair, HSG C					
	0	84 5	50-75% Grass cover, Fair, HSG D					
4	70,105	88 Weighted Average						
1	55,088	3	2.99% Per	vious Area				
3	15,017	67.01% Impervious Area						
Тс	Length	Slope	Velocity		Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
3.6	74	0.1350	0.34		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.4	26	0.0250	1.13		Sheet Flow, 2			
					Smooth surfaces n= 0.011 P2= 3.11"			
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3			
					Paved Kv= 20.3 fps			
1.7	744	0.0050	7.35	23.11	Pipe Channel, RCP_Round 24"			
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'			
					n= 0.009 Corrugated PE, smooth interior			
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4			
					Grassed Waterway Kv= 15.0 fps			
8.0	1,211	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 31

Subcatchment PDA-2A: Area to Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 32

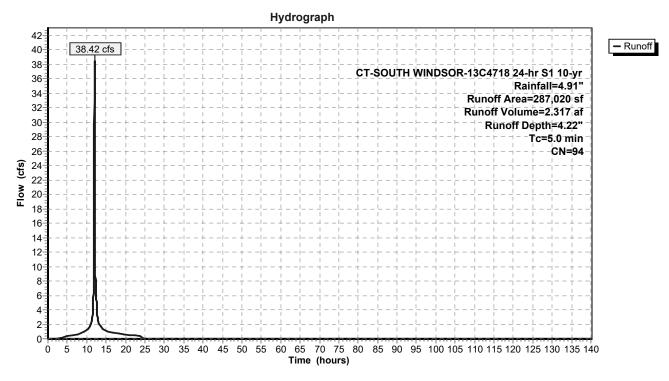
Summary for Subcatchment PDA-2B: BLDG AREA

Runoff = 38.42 cfs @ 12.03 hrs, Volume= 2.317 af, Depth= 4.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

Ar	ea (sf)	CN	Description					
24	40,618	98	Paved parking, HSG B					
	0	98	Paved parking, HSG C					
	3,973	98	Paved parking, HSG D					
	39,809	69	50-75% Grass cover, Fair, HSG B					
	0	79	50-75% Grass cover, Fair, HSG C					
	2,620	84	50-75% Grass cover, Fair, HSG D					
28	87,020	94	94 Weighted Average					
4	42,429	14.78% Pervious Area						
24	44,591	85.22% Impervious Area						
Тс	Length	Slope	,	Capacity	·			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
5.0					Direct Entry,			

Subcatchment PDA-2B: BLDG AREA



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 33

Summary for Subcatchment PDA-3: Area to Wetland DP-3

Runoff = 2.88 cfs @ 12.08 hrs, Volume= 0.208 af, Depth= 2.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

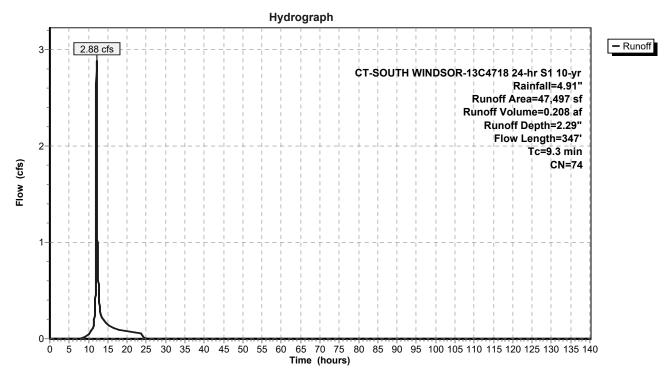
A	rea (sf)	CN E	Description						
	63	98 F	Paved parking, HSG B						
	0	98 F	Paved parking, HSG C						
	0	98 F	Paved parking, HSG D						
	20,845	69 5	50-75% Grass cover, Fair, HSG B						
	0	79 5	50-75% Grass cover, Fair, HSG C						
	7,654	84 5	50-75% Grass cover, Fair, HSG D						
	2,047	56 E	Brush, Fair, HSG B						
	16,888	77 E	77 Brush, Fair, HSG D						
	47,497	74 V	74 Weighted Average						
	47,434	99.87% Pervious Area							
	63	0	0.13% Impervious Area						
Tc	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
7.5	100	0.0400	0.22		Sheet Flow, 1				
					Grass: Short n= 0.150 P2= 3.11"				
0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2				
					Grassed Waterway Kv= 15.0 fps				
1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3				
					Woodland Kv= 5.0 fps				
9.3	347	Total							

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 34

Subcatchment PDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 35

Summary for Subcatchment PDA-4: Area to Wetland DP-4

Runoff = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af, Depth= 1.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

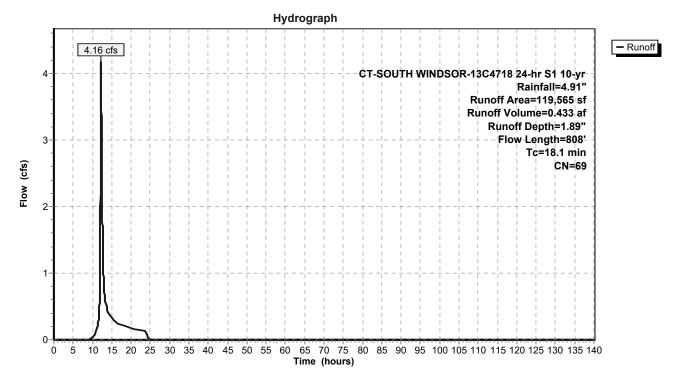
A	rea (sf)	CN E	escription					
	0	98 F	98 Paved parking, HSG B					
	0	98 F	aved park	ing, HSG C				
	0	98 F	aved park	ing, HSG D				
	57,679	69 5	0-75% Gra	ass cover, I	Fair, HSG B			
	26,837	79 5	0-75% Gra	ass cover, I	Fair, HSG C			
	0	84 5	0-75% Gra	ass cover, I	Fair, HSG D			
	25,526	56 E	Brush, Fair,	, HSG B				
	9,523	70 E	Brush, Fair,	, HSG C				
1	19,565	69 V	Veighted A	verage				
1	19,565	1	00.00% P	ervious Are	a			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	100	0.0800	0.29		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2			
					Grassed Waterway Kv= 15.0 fps			
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3			
					Woodland Kv= 5.0 fps			
0.1	74	0.0270	19.82	194.19	Channel Flow, 4			
					Area= 9.8 sf Perim= 15.7' r= 0.62'			
					n= 0.009 Corrugated PE, smooth interior			
18.1	808	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 36

Subcatchment PDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 37

Summary for Reach DP-1: Detention Basin 7

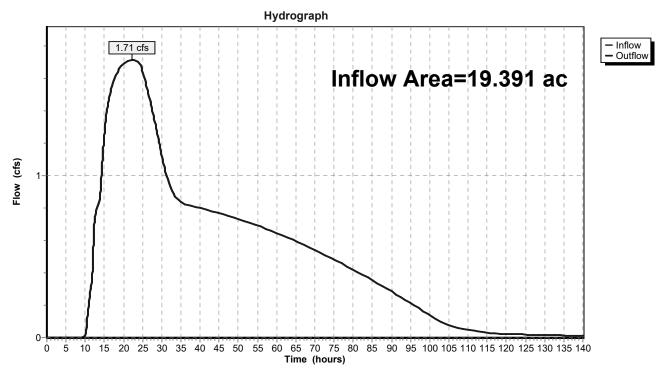
Inflow Area = 19.391 ac, 78.54% Impervious, Inflow Depth > 3.44" for 10-yr event

Inflow = 1.71 cfs @ 22.32 hrs, Volume= 5.564 af

Outflow = 1.71 cfs @ 22.32 hrs, Volume= 5.564 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



C-DAT-13C4718-PROPOSED HCT-SOUTH WINDSOR-13C4718 24-hr S1 10-yr Rainfall=4.91"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 38

Summary for Reach DP-2: Wetland DP-2

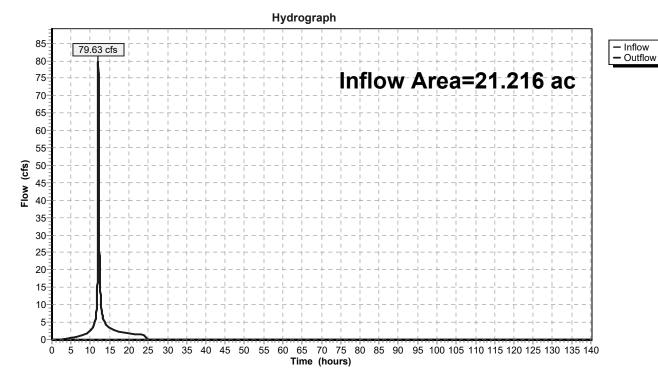
Inflow Area = 21.216 ac, 60.56% Impervious, Inflow Depth = 3.50" for 10-yr event

Inflow = 79.63 cfs @ 12.05 hrs, Volume= 6.181 af

Outflow = 79.63 cfs @ 12.05 hrs, Volume= 6.181 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 39

Summary for Reach DP-3: Wetland DP-3

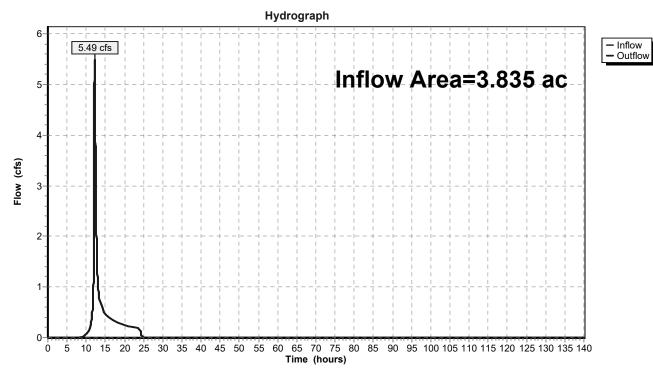
Inflow Area = 3.835 ac, 0.04% Impervious, Inflow Depth = 2.01" for 10-yr event

Inflow = 5.49 cfs @ 12.22 hrs, Volume= 0.641 af

Outflow = 5.49 cfs @ 12.22 hrs, Volume= 0.641 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 40

Summary for Reach DP-4: Wetland DP-4

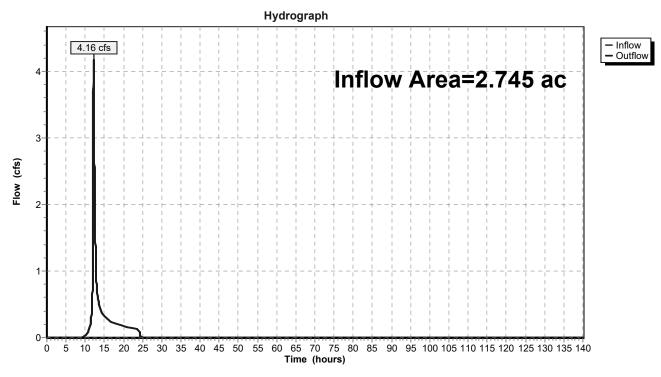
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 1.89" for 10-yr event

Inflow = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af

Outflow = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 41

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 10.792 ac, 67.01% Impervious, Inflow Depth = 3.58" for 10-yr event

Inflow = 47.69 cfs @ 12.06 hrs, Volume= 3.223 af

Outflow = 47.06 cfs @ 12.08 hrs, Volume= 3.223 af, Atten= 1%, Lag= 1.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 7.49 fps, Min. Travel Time= 0.9 min Avg. Velocity = 1.95 fps, Avg. Travel Time= 3.4 min

Peak Storage= 2,494 cf @ 12.07 hrs Average Depth at Peak Storage= 0.57'

Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

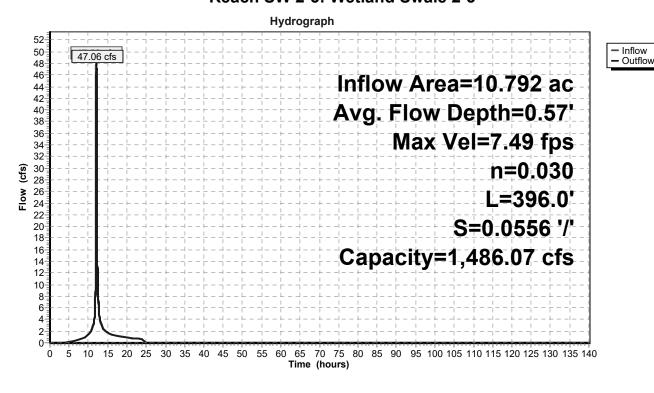
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 42

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 1.89" for 10-yr event

Inflow = 4.16 cfs @ 12.21 hrs, Volume= 0.433 af

Outflow = 4.12 cfs @ 12.26 hrs, Volume= 0.433 af, Atten= 1%, Lag= 3.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.00 fps, Min. Travel Time= 1.9 min Avg. Velocity = 1.40 fps, Avg. Travel Time= 4.1 min

Peak Storage= 473 cf @ 12.23 hrs Average Depth at Peak Storage= 0.13' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

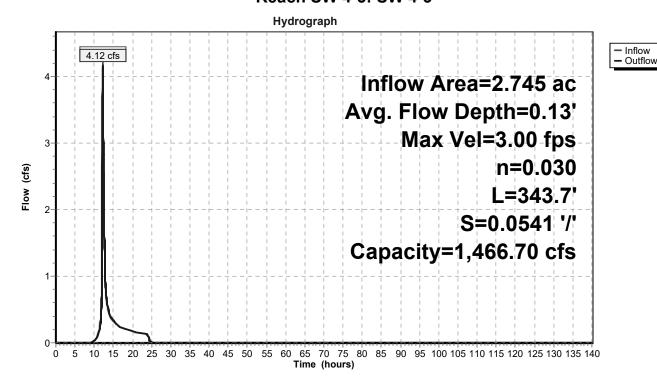
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 43

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.391 ac, 78.54% Impervious, Inflow Depth = 3.67" for 10-yr event

Inflow = 39.50 cfs @ 12.17 hrs, Volume= 5.932 af

Outflow = 1.71 cfs @ 22.32 hrs, Volume= 5.564 af, Atten= 96%, Lag= 609.1 min

Primary = 1.71 cfs @ 22.32 hrs, Volume= 5.564 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 95.95' @ 22.32 hrs Surf.Area= 30,403 sf Storage= 140,381 cf

Plug-Flow detention time= 1,607.0 min calculated for 5.564 af (94% of inflow)

Center-of-Mass det. time= 1,427.6 min (2,662.5 - 1,235.0)

Volume	Inve	ert Avail.Sto	rage Storage	Description	
#1	90.0	0' 280,77	70 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevation	on	Surf.Area	Inc.Store	Cum.Store	
(fee		(sq-ft)	(cubic-feet)	(cubic-feet)	
90.0	00	12,621	0	0	
92.0	00	22,814	35,435	35,435	
94.0	00	26,576	49,390	84,825	
96.0	00	30,501	57,077	141,902	
98.0	00	34,640	65,141	207,043	
100.0	00	39,087	73,727	280,770	
Device	Routing	Invert	Outlet Devices	S	
#1	Primary	88.00'	18.0" Round	Culvert	
	•		L= 71.0' CMF	⊃, square edge	headwall, Ke= 0.500
			Inlet / Outlet Ir	nvert= 88.00' / 8	4.50' S= 0.0493 '/' Cc= 0.900
			n= 0.009 Cori	rugated PE, sm	ooth interior, Flow Area= 1.77 sf
#2	Device 1	91.00'	4.0" Vert. Ori	fice/Grate C=	0.600
#3	Device 1	95.00'	6.0" Vert. Ori	fice/Grate C=	0.600

6.0" Vert. Orifice/Grate C= 0.600

Limited to weir flow at low heads

36.0" x 78.0" Horiz. Orifice/Grate C= 0.600

Primary OutFlow Max=1.71 cfs @ 22.32 hrs HW=95.95' (Free Discharge)

1=Culvert (Passes 1.71 cfs of 22.83 cfs potential flow)

98.00'

99.00'

2=Orifice/Grate (Orifice Controls 0.92 cfs @ 10.53 fps) **3=Orifice/Grate** (Orifice Controls 0.79 cfs @ 4.03 fps)

-4=Orifice/Grate (Controls 0.00 cfs)

#4

#5

Device 1

Device 1

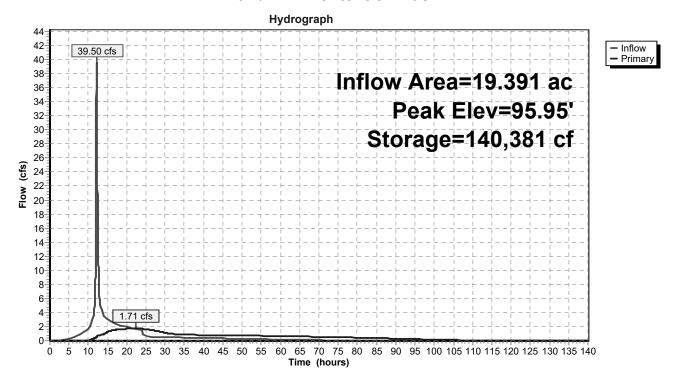
-5=Orifice/Grate (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 44

Pond P-7: Dentention Basin 7



C-DAT-13C4718-PROPOSED H*CT-SOUTH WINDSOR-13C4718 24-hr* S1 10-yr Rainfall=4.91"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 45

Summary for Pond S-1: Subsurface Det

Inflow Area = 7.794 ac, 85.88% Impervious, Inflow Depth = 4.22" for 10-yr event

Inflow = 45.45 cfs @ 12.03 hrs, Volume= 2.741 af

Outflow = 1.03 cfs @ 16.27 hrs, Volume= 2.168 af, Atten= 98%, Lag= 254.9 min

Primary = 1.03 cfs @ 16.27 hrs, Volume= 2.168 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 103.98' @ 16.27 hrs Surf.Area= 31,034 sf Storage= 85,981 cf

Plug-Flow detention time= 1,293.5 min calculated for 2.168 af (79% of inflow)

Center-of-Mass det. time= 1,201.0 min (1,980.0 - 779.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	100.25'	49,002 cf	228.33'W x 135.92'L x 6.75'H Field A
			209,482 cf Overall - 86,977 cf Embedded = 122,504 cf x 40.0% Voids
#2A	101.00'	86,977 cf	ADS_StormTech MC-4500 +Capx 800 Inside #1
			Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf
			Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap
			25 Rows of 32 Chambers
			Cap Storage= +35.7 cf x 2 x 25 rows = 1,785.0 cf
-		125 070 of	Total Available Storage

135,979 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	101.50'	24.0" Round Culvert
	-		L= 200.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 101.50' / 99.50' S= 0.0100 '/' Cc= 0.900
			n= 0.009 Corrugated PE, smooth interior, Flow Area= 3.14 sf
#2	Device 1	101.55'	3.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
#3	Device 1	103.40'	5.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
#4	Device 1	106.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=1.03 cfs @ 16.27 hrs HW=103.98' (Free Discharge)

-1=Culvert (Passes 1.03 cfs of 18.43 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.60 cfs @ 7.25 fps)

-3=Orifice/Grate (Orifice Controls 0.43 cfs @ 3.09 fps)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 46

Pond S-1: Subsurface Det - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-4500 + Cap (ADS StormTech® MC-4500 with cap volume)

Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap Cap Storage= +35.7 cf x 2 x 25 rows = 1,785.0 cf

100.0" Wide + 9.0" Spacing = 109.0" C-C Row Spacing

32 Chambers/Row x 4.02' Long +2.56' Cap Length x 2 = 133.92' Row Length +12.0" End Stone x 2 = 135.92' Base Length

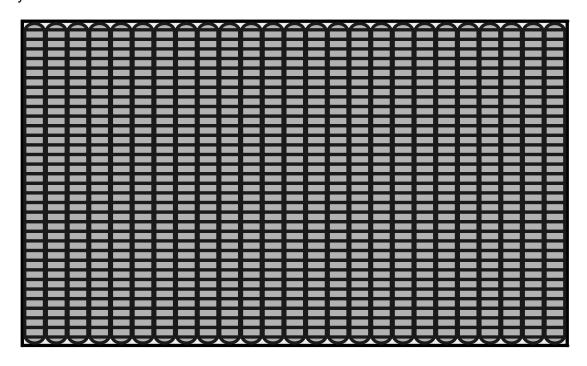
25 Rows x 100.0" Wide + 9.0" Spacing x 24 + 12.0" Side Stone x 2 = 228.33' Base Width 9.0" Base + 60.0" Chamber Height + 12.0" Cover = 6.75' Field Height

800 Chambers x 106.5 cf + 35.7 cf Cap Volume x 2 x 25 Rows = 86,977.3 cf Chamber Storage

209,481.6 cf Field - 86,977.3 cf Chambers = 122,504.2 cf Stone x 40.0% Voids = 49,001.7 cf Stone Storage

Chamber Storage + Stone Storage = 135,979.0 cf = 3.122 af Overall Storage Efficiency = 64.9% Overall System Size = 135.92' x 228.33' x 6.75'

800 Chambers 7,758.6 cy Field 4,537.2 cy Stone



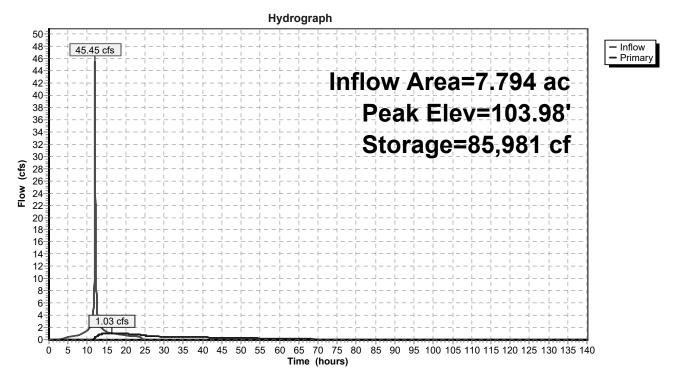


Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 47

Pond S-1: Subsurface Det



C-DAT-13C4718-PROPOSED HCT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 48

Time span=0.00-140.00 hrs, dt=0.01 hrs, 14001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPDA-1A: Area to Detention Runoff Area=505,152 sf 73.61% Impervious Runoff Depth=4.99" Flow Length=1,379' Tc=15.9 min CN=91 Runoff=48.88 cfs 4.819 af

SubcatchmentPDA-1B: Area to Runoff Area=339,526 sf 85.88% Impervious Runoff Depth=5.33"

Tc=5.0 min CN=94 Runoff=56.09 cfs 3.460 af

SubcatchmentPDA-2A: Area to Wetland Runoff Area=470,105 sf 67.01% Impervious Runoff Depth=4.65" Flow Length=1,211' Tc=8.0 min CN=88 Runoff=60.69 cfs 4.186 af

SubcatchmentPDA-2B: BLDG AREA Runoff Area=287,020 sf 85.22% Impervious Runoff Depth=5.33"

Tc=5.0 min CN=94 Runoff=47.42 cfs 2.925 af

SubcatchmentPDA-3: Area to Wetland DP-3 Runoff Area=47,497 sf 0.13% Impervious Runoff Depth=3.21" Flow Length=347' Tc=9.3 min CN=74 Runoff=4.03 cfs 0.292 af

SubcatchmentPDA-4: Area to WetlandRunoff Area=119,565 sf 0.00% Impervious Runoff Depth=2.74"
Flow Length=808' Tc=18.1 min CN=69 Runoff=6.10 cfs 0.626 af

Reach DP-1: Detention Basin 7 Inflow=2.30 cfs 7.335 af

Outflow=2.30 cfs 7.335 af

Reach DP-2: Wetland DP-2 Inflow=101.82 cfs 8.028 af

Outflow=101.82 cfs 8.028 af

Reach DP-3: Wetland DP-3 Inflow=8.04 cfs 0.918 af

Outflow=8.04 cfs 0.918 af

Reach DP-4: Wetland DP-4 Inflow=6.10 cfs 0.626 af

Outflow=6.10 cfs 0.626 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=0.66' Max Vel=8.16 fps Inflow=60.69 cfs 4.186 af

n=0.030 L=396.0' S=0.0556'/' Capacity=1,486.07 cfs Outflow=59.98 cfs 4.186 af

Reach SW 4-3: SW 4-3 Avg. Flow Depth=0.17' Max Vel=3.47 fps Inflow=6.10 cfs 0.626 af

n=0.030 L=343.7' S=0.0541 '/' Capacity=1,466.70 cfs Outflow=6.06 cfs 0.626 af

Pond P-7: Dentention Basin 7 Peak Elev=97.07' Storage=175,878 cf Inflow=49.74 cfs 7.705 af

Outflow=2.30 cfs 7.335 af

Pond S-1: Subsurface Det Peak Elev=104.85' Storage=104,765 cf Inflow=56.09 cfs 3.460 af

Outflow=1.47 cfs 2.886 af

Total Runoff Area = 40.608 ac Runoff Volume = 16.307 af Average Runoff Depth = 4.82" 30.85% Pervious = 12.529 ac 69.15% Impervious = 28.078 ac HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 49

Summary for Subcatchment PDA-1A: Area to Detention Basin 7

Runoff = 48.88 cfs @ 12.17 hrs, Volume= 4.819 af, Depth= 4.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

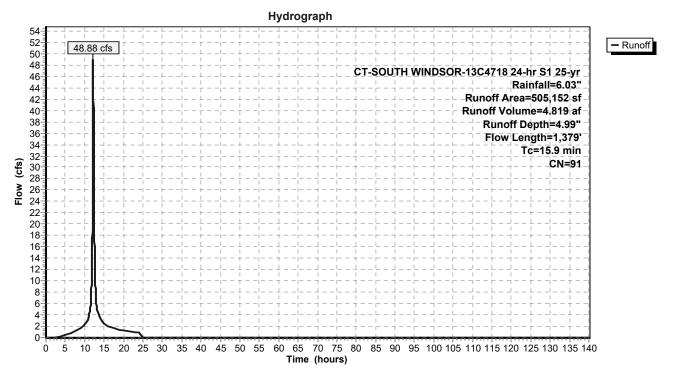
A	rea (sf)	CN D	escription					
	82,516	98 P	98 Paved parking, HSG B					
2	75,911			ing, HSG C				
	12,219	98 P	aved park	ing, HSG B	}			
	1,191	98 P	aved park	ing, HSG C				
	78,680	69 5	0-75% Gra	ass cover, l	Fair, HSG B			
	41,252	79 5	0-75% Gra	ass cover, f	Fair, HSG C			
	10,535	69 5	0-75% Gra	ass cover, f	Fair, HSG B			
	467				Fair, HSG C			
	0		Fravel road					
	389		Gravel road	•				
	1,992		Fravel road					
	0		Fravel road					
	05,152		Veighted A	-				
	33,315	_		vious Area				
3	71,837	7	3.61% Imp	ervious Ar	ea			
т.	1 41-	Ol	\/-1:4	0	Description			
Tc	Length	Slope	Velocity	. ,	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
7.9	100	0.0900	0.21		Sheet Flow,			
0.7	0.40	0.0400	4.50		Grass: Dense n= 0.240 P2= 3.11"			
2.7	249	0.0480	1.53		Shallow Concentrated Flow,			
4.0	202	0.0070	4.00		Short Grass Pasture Kv= 7.0 fps			
4.8	383	0.0078	1.32		Shallow Concentrated Flow, swale			
0.5	647	0.0400	20.80	65.35	Grassed Waterway Kv= 15.0 fps			
0.5	047	0.0400	20.00	05.35	Pipe Channel , 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'			
					n= 0.009 Corrugated PE, smooth interior			
15.0	1 270	Total			11- 0.000 Corrugated I L, Sillootti liiterioi			
15.9	1,379	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 50

Subcatchment PDA-1A: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 51

Summary for Subcatchment PDA-1B: Area to Subsurface Detention System

Runoff = 56.09 cfs @ 12.03 hrs, Volume= 3.460 af, Depth= 5.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

Area (sf)	CN	I Description						
269,483	98	B Paved parking, HSG B						
22,100	98	Paved parking, HSG C						
0	98	Paved parking, HSG B						
0	98	Paved parking, HSG C						
43,113	69	50-75% Grass cover, Fair, HSG B						
4,830	79	50-75% Grass cover, Fair, HSG C						
0	69	50-75% Grass cover, Fair, HSG B						
0	79	50-75% Grass cover, Fair, HSG C						
0	85	Gravel roads, HSG B						
0	89	Gravel roads, HSG C						
0	85	Gravel roads, HSG B						
0	89	Gravel roads, HSG C						
339,526	94	94 Weighted Average						
47,943		14.12% Pervious Area						
291,583		85.88% Impervious Area						
		·						
Tc Length	Slo	pe Velocity Capacity Description						
(min) (feet)								
		Direct Entry						

5.0

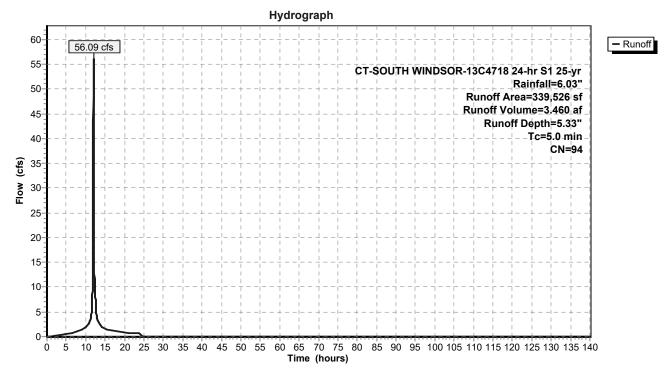
Direct Entry,

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 52

Subcatchment PDA-1B: Area to Subsurface Detention System



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 53

Summary for Subcatchment PDA-2A: Area to Wetland DP-2

Runoff = 60.69 cfs @ 12.06 hrs, Volume= 4.186 af, Depth= 4.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

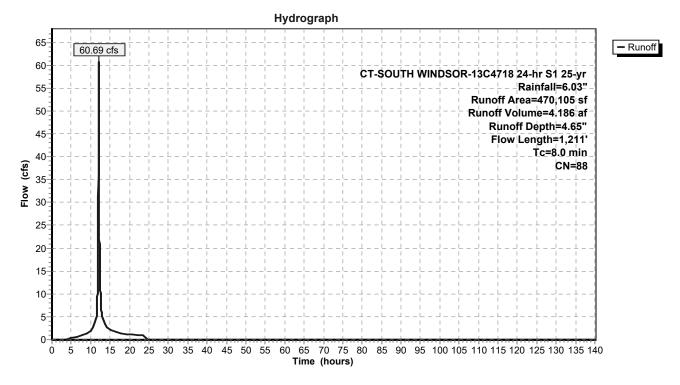
A	rea (sf)	CN E	escription				
3	04,766	98 F	98 Paved parking, HSG B				
	10,251			ing, HSG C			
	0	98 F	aved park	ing, HSG D			
1	54,500	69 5	0-75% Gra	ass cover, l	Fair, HSG B		
	588				Fair, HSG C		
	0	84 5	0-75% Gra	ass cover, l	Fair, HSG D		
4	70,105		Veighted A				
	55,088	_		vious Area			
3	315,017	6	7.01% lmp	ervious Ar	ea		
_		01					
Tc	Length	Slope	Velocity		Description		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
3.6	74	0.1350	0.34		Sheet Flow, 1		
0.4	00	0.0050	4.40		Grass: Short n= 0.150 P2= 3.11"		
0.4	26	0.0250	1.13		Sheet Flow, 2		
1 1	216	0.0250	2 24		Smooth surfaces n= 0.011 P2= 3.11"		
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3 Paved Kv= 20.3 fps		
1.7	744	0.0050	7.35	23.11	·		
1.7	744	0.0030	7.55	25.11	24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'		
					n= 0.009 Corrugated PE, smooth interior		
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4		
	.01	3.0230			Grassed Waterway Kv= 15.0 fps		
8.0	1,211	Total			,		

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 54

Subcatchment PDA-2A: Area to Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 55

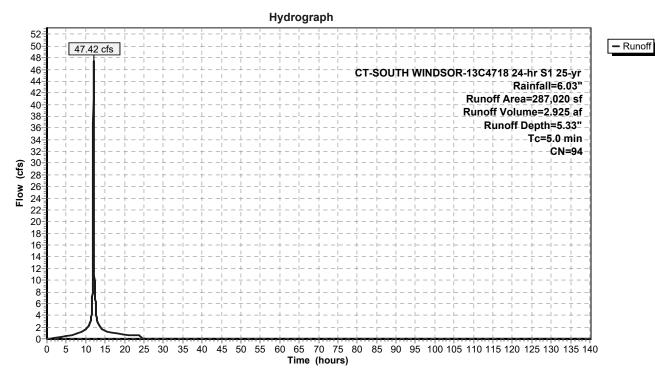
Summary for Subcatchment PDA-2B: BLDG AREA

Runoff = 47.42 cfs @ 12.03 hrs, Volume= 2.925 af, Depth= 5.33"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

Are	ea (sf)	CN	Description				
24	0,618	98	Paved park	ing, HSG B	В		
	0	98	Paved park	ing, HSG C	C		
;	3,973	98	Paved park	ing, HSG D	D		
3	9,809	69	50-75% Gra	ass cover, F	Fair, HSG B		
	0	79	50-75% Grass cover, Fair, HSG C				
	2,620	84	50-75% Grass cover, Fair, HSG D				
28	7,020	94	94 Weighted Average				
4:	2,429	14.78% Pervious Area					
24	4,591 85.22% Impervious Area				ırea		
Tc I	Length	Slope	,	Capacity	•		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
5.0					Direct Entry,		

Subcatchment PDA-2B: BLDG AREA



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 56

Summary for Subcatchment PDA-3: Area to Wetland DP-3

Runoff = 4.03 cfs @ 12.08 hrs, Volume= 0.292 af, Depth= 3.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

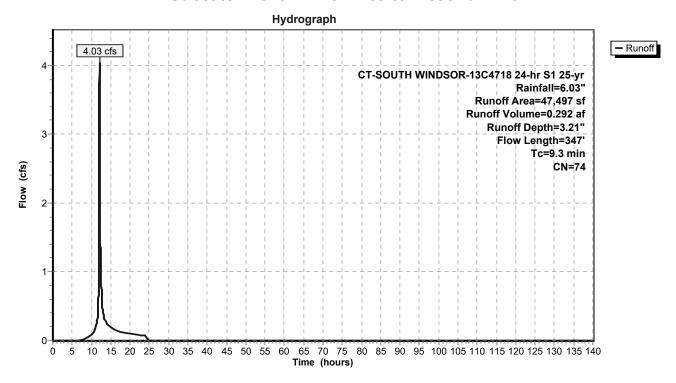
A	rea (sf)	CN [Description					
	63	98 F	8 Paved parking, HSG B					
	0	98 F	Paved park	ing, HSG C				
	0	98 F	Paved park	ing, HSG [
	20,845	69 5	50-75% Gra	ass cover, l	Fair, HSG B			
	0	79 5	50-75% Gra	ass cover, l	Fair, HSG C			
	7,654	84 5	50-75% Gra	ass cover, l	Fair, HSG D			
	2,047	56 E	Brush, Fair, HSG B					
	16,888	77 E	Brush, Fair,	HSG D				
	47,497	74 V	Veighted A	verage				
	47,434	ç	9.87% Per	vious Area	l			
	63	C).13% Impe	ervious Are	a			
Tc	Length	Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
7.5	100	0.0400	0.22		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2			
					Grassed Waterway Kv= 15.0 fps			
1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3			
					Woodland Kv= 5.0 fps			
9.3	347	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 57

Subcatchment PDA-3: Area to Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 58

Summary for Subcatchment PDA-4: Area to Wetland DP-4

Runoff = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af, Depth= 2.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

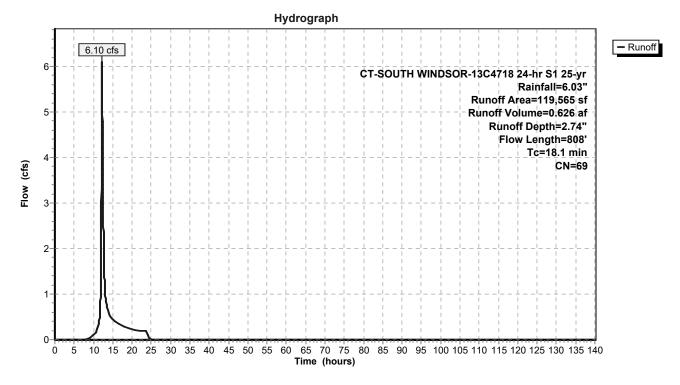
A	rea (sf)	CN D	escription					
	0	98 F	98 Paved parking, HSG B					
	0			ing, HSG C				
	0	98 F	aved park	ing, HSG D)			
	57,679	69 5	0-75% Gra	ass cover, l	Fair, HSG B			
	26,837	79 5	0-75% Gra	ass cover, l	Fair, HSG C			
	0	84 5	0-75% Gra	ass cover, l	Fair, HSG D			
	25,526	56 B	rush, Fair,	, HSG B				
	9,523	70 E	rush, Fair,	, HSG C				
1	19,565	69 V	Veighted A	verage				
1	19,565	1	00.00% P	ervious Are	a			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
5.7	100	0.0800	0.29		Sheet Flow, 1			
					Grass: Short n= 0.150 P2= 3.11"			
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2			
					Grassed Waterway Kv= 15.0 fps			
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3			
					Woodland Kv= 5.0 fps			
0.1	74	0.0270	19.82	194.19	Channel Flow, 4			
					Area= 9.8 sf Perim= 15.7' r= 0.62'			
					n= 0.009 Corrugated PE, smooth interior			
18.1	808	Total						

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 59

Subcatchment PDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 60

Summary for Reach DP-1: Detention Basin 7

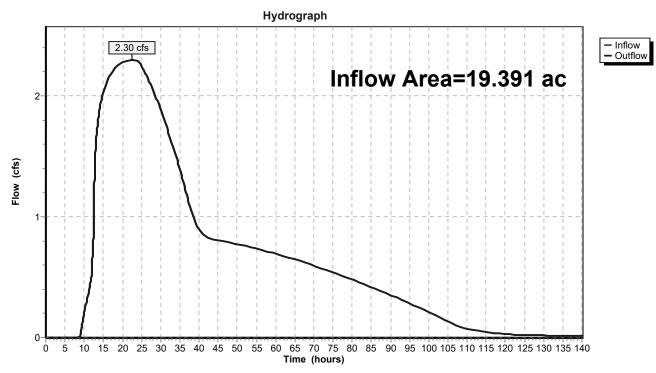
Inflow Area = 19.391 ac, 78.54% Impervious, Inflow Depth > 4.54" for 25-yr event

Inflow = 2.30 cfs @ 22.39 hrs, Volume= 7.335 af

Outflow = 2.30 cfs @ 22.39 hrs, Volume= 7.335 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



C-DAT-13C4718-PROPOSED HCT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 61

Summary for Reach DP-2: Wetland DP-2

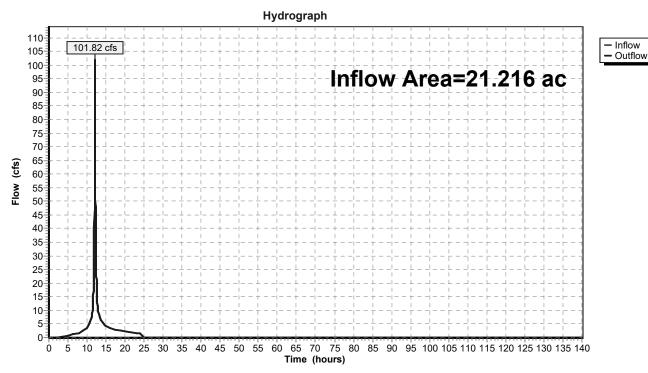
Inflow Area = 21.216 ac, 60.56% Impervious, Inflow Depth = 4.54" for 25-yr event

Inflow = 101.82 cfs @ 12.05 hrs, Volume= 8.028 af

Outflow = 101.82 cfs @ 12.05 hrs, Volume= 8.028 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 62

Summary for Reach DP-3: Wetland DP-3

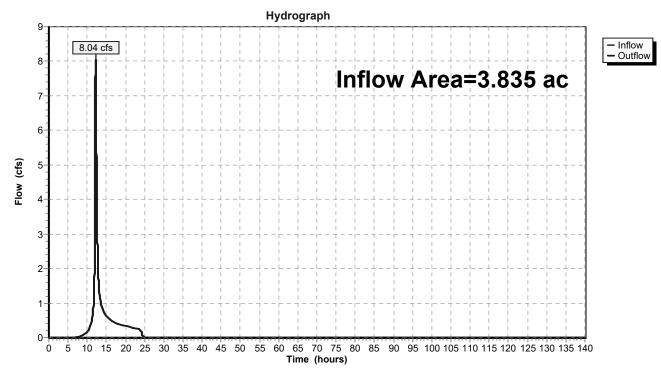
Inflow Area = 3.835 ac, 0.04% Impervious, Inflow Depth = 2.87" for 25-yr event

Inflow = 8.04 cfs @ 12.21 hrs, Volume= 0.918 af

Outflow = 8.04 cfs @ 12.21 hrs, Volume= 0.918 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 63

Summary for Reach DP-4: Wetland DP-4

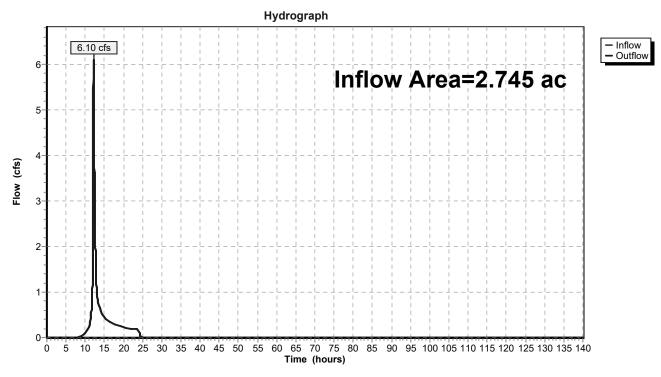
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 2.74" for 25-yr event

Inflow = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af

Outflow = 6.10 cfs (a) 12.21 hrs, Volume= 0.626 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 64

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 10.792 ac, 67.01% Impervious, Inflow Depth = 4.65" for 25-yr event

Inflow 60.69 cfs @ 12.06 hrs, Volume= 4.186 af

Outflow 59.98 cfs @ 12.08 hrs, Volume= 4.186 af, Atten= 1%, Lag= 1.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 8.16 fps, Min. Travel Time= 0.8 min Avg. Velocity = 2.11 fps, Avg. Travel Time= 3.1 min

Peak Storage= 2,917 cf @ 12.07 hrs Average Depth at Peak Storage= 0.66' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

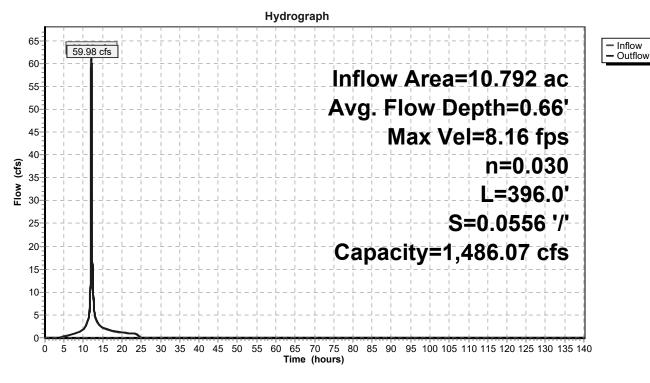
10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 65

Inflow

Outflow

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 2.74" for 25-yr event

Inflow = 6.10 cfs @ 12.21 hrs, Volume= 0.626 af

Outflow = 6.06 cfs @ 12.25 hrs, Volume= 0.626 af, Atten= 1%, Lag= 2.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 3.47 fps, Min. Travel Time= 1.7 min Avg. Velocity = 1.44 fps, Avg. Travel Time= 4.0 min

Peak Storage= 601 cf @ 12.22 hrs Average Depth at Peak Storage= 0.17' Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

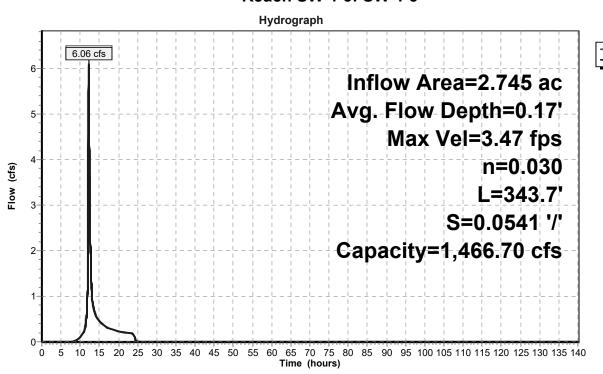
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 66

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.391 ac, 78.54% Impervious, Inflow Depth = 4.77" for 25-yr event

Inflow = 49.74 cfs @ 12.17 hrs, Volume= 7.705 af

Outflow = 2.30 cfs @ 22.39 hrs, Volume= 7.335 af, Atten= 95%, Lag= 613.6 min

Primary = 2.30 cfs @ 22.39 hrs, Volume= 7.335 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 97.07' @ 22.39 hrs Surf.Area= 32,725 sf Storage= 175,878 cf

Plug-Flow detention time= 1,473.3 min calculated for 7.335 af (95% of inflow)

Center-of-Mass det. time= 1,325.4 min (2,529.1 - 1,203.7)

Volume	Inv	ert Avail.Sto	orage Storage	Description	
#1	90	00' 280,7	70 cf Custom	n Stage Data (P	rismatic)Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
90.0		12,621	Ó	0	
92.0	00	22,814	35,435	35,435	
94.0	00	26,576	49,390	84,825	
96.0	00	30,501	57,077	141,902	
98.0	00	34,640	65,141	207,043	
100.0	00	39,087	73,727	280,770	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	88.00'	18.0" Round	d Culvert	
					headwall, Ke= 0.500
					34.50' S= 0.0493 '/' Cc= 0.900
					ooth interior, Flow Area= 1.77 sf
#2	Device	1 91.00'	4.0" Vert. Or	ifice/Grate C=	0.600

6.0" Vert. Orifice/Grate C= 0.600

6.0" Vert. Orifice/Grate C= 0.600

Limited to weir flow at low heads

36.0" x 78.0" Horiz. Orifice/Grate C= 0.600

Primary OutFlow Max=2.30 cfs @ 22.39 hrs HW=97.07' (Free Discharge)

-1=Culvert (Passes 2.30 cfs of 24.55 cfs potential flow)

95.00'

98.00'

99.00'

2=Orifice/Grate (Orifice Controls 1.02 cfs @ 11.70 fps)

—3=Orifice/Grate (Orifice Controls 1.28 cfs @ 6.50 fps)

-4=Orifice/Grate (Controls 0.00 cfs)
-5=Orifice/Grate (Controls 0.00 cfs)

#3

#4

#5

Device 1

Device 1

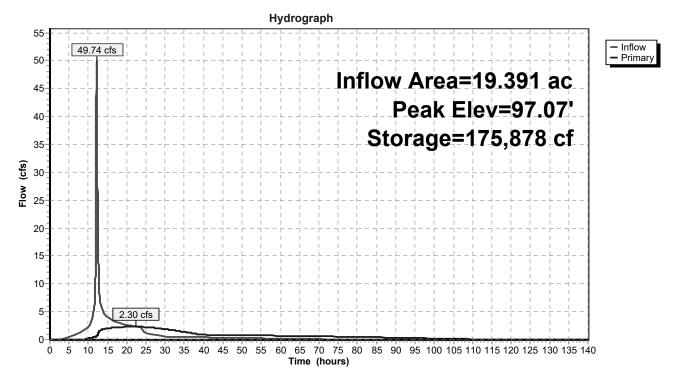
Device 1

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 67

Pond P-7: Dentention Basin 7



C-DAT-13C4718-PROPOSED HCT-SOUTH WINDSOR-13C4718 24-hr S1 25-yr Rainfall=6.03"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

<u>Page 68</u>

Summary for Pond S-1: Subsurface Det

Inflow Area = 7.794 ac, 85.88% Impervious, Inflow Depth = 5.33" for 25-yr event

Inflow = 56.09 cfs @ 12.03 hrs, Volume= 3.460 af

Outflow = 1.47 cfs @ 15.53 hrs, Volume= 2.886 af, Atten= 97%, Lag= 210.1 min

Primary = 1.47 cfs @ 15.53 hrs, Volume= 2.886 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 104.85' @ 15.53 hrs Surf.Area= 31,034 sf Storage= 104,765 cf

Plug-Flow detention time= 1,189.7 min calculated for 2.886 af (83% of inflow)

Center-of-Mass det. time= 1,109.0 min (1,881.3 - 772.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	100.25'	49,002 cf	228.33'W x 135.92'L x 6.75'H Field A
			209,482 cf Overall - 86,977 cf Embedded = 122,504 cf x 40.0% Voids
#2A	101.00'	86,977 cf	ADS_StormTech MC-4500 +Capx 800 Inside #1
			Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf
			Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap
			25 Rows of 32 Chambers
			Cap Storage= +35.7 cf x 2 x 25 rows = 1,785.0 cf
		125 070 of	Total Available Ctarage

135,979 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	101.50'	24.0" Round Culvert
	•		L= 200.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 101.50' / 99.50' S= 0.0100 '/' Cc= 0.900
			n= 0.009 Corrugated PE, smooth interior, Flow Area= 3.14 sf
#2	Device 1	101.55'	3.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
#3	Device 1	103.40'	5.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
#4	Device 1	106.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=1.47 cfs @ 15.53 hrs HW=104.85' (Free Discharge)

1=Culvert (Passes 1.47 cfs of 23.18 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.71 cfs @ 8.52 fps)

-3=Orifice/Grate (Orifice Controls 0.76 cfs @ 5.45 fps)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

<u>Page 69</u>

Pond S-1: Subsurface Det - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-4500 + Cap (ADS StormTech® MC-4500 with cap volume)

Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap Cap Storage= +35.7 cf x 2 x 25 rows = 1,785.0 cf

100.0" Wide + 9.0" Spacing = 109.0" C-C Row Spacing

32 Chambers/Row x 4.02' Long +2.56' Cap Length x 2 = 133.92' Row Length +12.0" End Stone x 2 = 135.92' Base Length

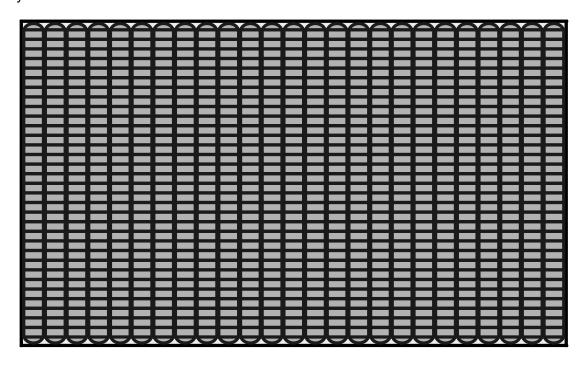
25 Rows x 100.0" Wide + 9.0" Spacing x 24 + 12.0" Side Stone x 2 = 228.33' Base Width 9.0" Base + 60.0" Chamber Height + 12.0" Cover = 6.75' Field Height

800 Chambers x 106.5 cf + 35.7 cf Cap Volume x 2 x 25 Rows = 86,977.3 cf Chamber Storage

209,481.6 cf Field - 86,977.3 cf Chambers = 122,504.2 cf Stone x 40.0% Voids = 49,001.7 cf Stone Storage

Chamber Storage + Stone Storage = 135,979.0 cf = 3.122 af Overall Storage Efficiency = 64.9% Overall System Size = 135.92' x 228.33' x 6.75'

800 Chambers 7,758.6 cy Field 4,537.2 cy Stone



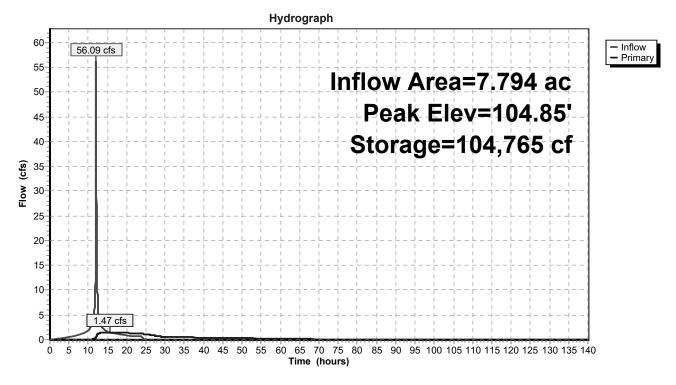


Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 70

Pond S-1: Subsurface Det



C-DAT-13C4718-PROPOSED CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 71

Time span=0.00-140.00 hrs, dt=0.01 hrs, 14001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentPDA-1A: Area to Detention Runoff Area=505,152 sf 73.61% Impervious Runoff Depth=6.70" Flow Length=1,379' Tc=15.9 min CN=91 Runoff=63.92 cfs 6.472 af

SubcatchmentPDA-1B: Area to Runoff Area=339,526 sf 85.88% Impervious Runoff Depth=7.05"

Tc=5.0 min CN=94 Runoff=72.33 cfs 4.581 af

SubcatchmentPDA-2A: Area to Wetland Runoff Area=470,105 sf 67.01% Impervious Runoff Depth=6.34" Flow Length=1,211' Tc=8.0 min CN=88 Runoff=80.53 cfs 5.705 af

SubcatchmentPDA-2B: BLDG AREA Runoff Area=287,020 sf 85.22% Impervious Runoff Depth=7.05"

Tc=5.0 min CN=94 Runoff=61.15 cfs 3.873 af

SubcatchmentPDA-3: Area to Wetland DP-3 Runoff Area=47,497 sf 0.13% Impervious Runoff Depth=4.72" Flow Length=347' Tc=9.3 min CN=74 Runoff=5.88 cfs 0.429 af

SubcatchmentPDA-4: Area to WetlandRunoff Area=119,565 sf 0.00% Impervious Runoff Depth=4.15"
Flow Length=808' Tc=18.1 min CN=69 Runoff=9.28 cfs 0.950 af

Reach DP-1: Detention Basin 7 Inflow=3.59 cfs 10.105 af
Outflow=3.59 cfs 10.105 af

Reach DP-2: Wetland DP-2Inflow=136.05 cfs 10.957 af

Outflow=136.05 cfs 10.957 af

Reach DP-3: Wetland DP-3Inflow=12.23 cfs 1.379 af
Outflow=12.23 cfs 1.379 af

Reach DP-4: Wetland DP-4

Inflow=9.28 cfs 0.950 af

Outflow=9.28 cfs 0.950 af

Reach SW 2-3: Wetland Swale 2-3 Avg. Flow Depth=0.78' Max Vel=9.01 fps Inflow=80.53 cfs 5.705 af n=0.030 L=396.0' S=0.0556'/' Capacity=1,486.07 cfs Outflow=79.74 cfs 5.705 af

Reach SW 4-3: SW 4-3Avg. Flow Depth=0.22' Max Vel=4.07 fps Inflow=9.28 cfs 0.950 af n=0.030 L=343.7' S=0.0541 '/' Capacity=1,466.70 cfs Outflow=9.23 cfs 0.950 af

Pond P-7: Dentention Basin 7 Peak Elev=98.75' Storage=233,651 cf Inflow=65.43 cfs 10.479 af

Outflow=3.59 cfs 10.105 af

Pond S-1: Subsurface Det

Peak Elev=106.32' Storage=127,577 cf Inflow=72.33 cfs 4.581 af

Outflow=4.08 cfs 4.007 af

Total Runoff Area = 40.608 ac Runoff Volume = 22.011 af Average Runoff Depth = 6.50" 30.85% Pervious = 12.529 ac 69.15% Impervious = 28.078 ac

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 72

Summary for Subcatchment PDA-1A: Area to Detention Basin 7

Runoff = 63.92 cfs @ 12.17 hrs, Volume= 6.472 af, Depth= 6.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

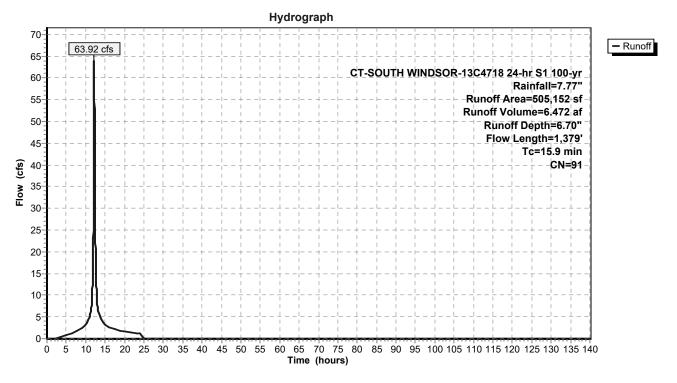
A	rea (sf)	CN D	escription								
	82,516	98 P	aved park	ing, HSG B							
2	75,911			ing, HSG C							
	12,219	98 P	Paved parking, HSG B								
	1,191			ing, HSG C							
	78,680				Fair, HSG B						
	41,252		50-75% Grass cover, Fair, HSG C								
	10,535		50-75% Grass cover, Fair, HSG B								
	467 79 50-75% Grass cover, Fair, HSG C										
	0		Fravel road								
	389		Fravel road	•							
	1,992		Gravel road	•							
	0		Gravel road	-							
	05,152		Veighted A								
	33,315	_		vious Area							
371,837 73.61% Impervious Are				ervious An	ea						
Тс	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description						
7.9	100	0.0900	0.21	(010)	Sheet Flow,						
1.5	100	0.0300	0.21		Grass: Dense n= 0.240 P2= 3.11"						
2.7	249	0.0480	1.53		Shallow Concentrated Flow,						
		0.0.00	1.00		Short Grass Pasture Kv= 7.0 fps						
4.8	383	0.0078	1.32		Shallow Concentrated Flow, swale						
					Grassed Waterway Kv= 15.0 fps						
0.5	647	0.0400	20.80	65.35	Pipe Channel,						
					24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'						
					n= 0.009 Corrugated PE, smooth interior						
15.9	1,379	Total									

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 73

Subcatchment PDA-1A: Area to Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 74

Summary for Subcatchment PDA-1B: Area to Subsurface Detention System

Runoff 72.33 cfs @ 12.03 hrs, Volume= 4.581 af, Depth= 7.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

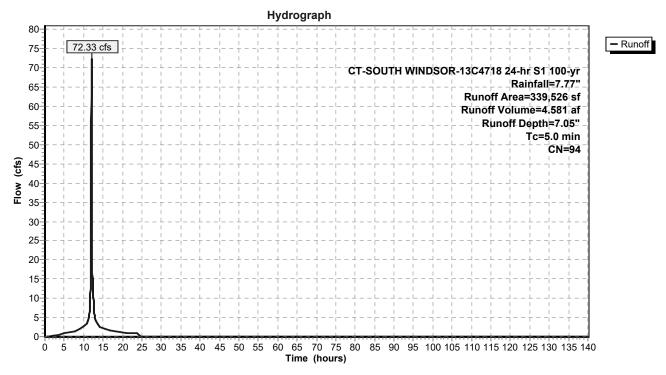
Area	a (sf)	CN	Description										
269	9,483	98	Paved parking, HSG B										
22	2,100	98	Paved parking, HSG C										
	0	98	ved parking, HSG B										
	0	98	Paved parking, HSG C										
43	3,113	69	50-75% Grass cover, Fair, HSG B										
4	1,830	79	50-75% Grass cover, Fair, HSG C										
	0	69	50-75% Grass cover, Fair, HSG B										
	0	79	50-75% Grass cover, Fair, HSG C										
	0	85	Gravel roads, HSG B										
	0	89	Gravel roads, HSG C										
	0	85	ravel roads, HSG B										
	0	89	Gravel roads, HSG C										
339	9,526	94	Weighted Average										
47	7,943		14.12% Pervious Area										
291	1,583		85.88% Impervious Area										
T	41.	01	Wileston Operation Description										
	ength	Slop											
(min)	(feet)	(ft/f											
5.0			Direct Entry,										

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 75

Subcatchment PDA-1B: Area to Subsurface Detention System



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 76

Summary for Subcatchment PDA-2A: Area to Wetland DP-2

Runoff = 80.53 cfs @ 12.06 hrs, Volume= 5.705 af, Depth= 6.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

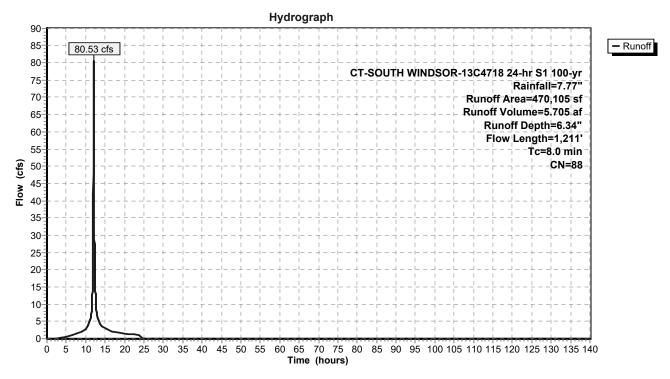
A	rea (sf)	CN E	escription										
3	04,766	98 F	1 37										
	10,251	98 F	aved park	eved parking, HSG C									
	0	98 F	aved park	ed parking, HSG D									
1	54,500	69 5	0-75% Gra	5% Grass cover, Fair, HSG B									
	588			,	Fair, HSG C								
	0	84 5	0-75% Gra	ass cover, F	Fair, HSG D								
4	70,105		Veighted A										
	55,088			vious Area									
3	15,017	6	7.01% lmp	ervious Ar	ea								
_													
Tc	Length	Slope	•	Velocity Capacity Description									
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)									
3.6	74	0.1350	0.34		Sheet Flow, 1								
					Grass: Short n= 0.150 P2= 3.11"								
0.4	26	0.0250	1.13		Sheet Flow, 2								
	040	0.0050	0.04		Smooth surfaces n= 0.011 P2= 3.11"								
1.1	216	0.0250	3.21		Shallow Concentrated Flow, 3								
17	711	0.0050	7.05	00.44	Paved Kv= 20.3 fps								
1.7	744	0.0050	7.35	23.11	Pipe Channel, RCP_Round 24" 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50'								
					n= 0.009 Corrugated PE, smooth interior								
1.2	151	0.0200	2.12		Shallow Concentrated Flow, 4								
1.2	101	0.0200	2.12		Grassed Waterway Kv= 15.0 fps								
8.0	1,211	Total			5.45554 **A.51**4y 1(* 15.5 lpc								
0.0	1,411	i Otai											

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 77

Subcatchment PDA-2A: Area to Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 78

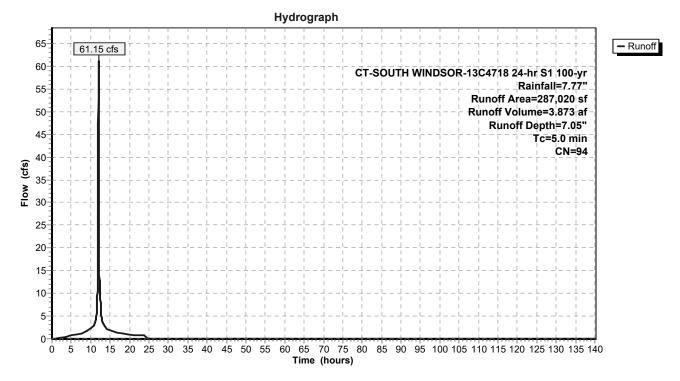
Summary for Subcatchment PDA-2B: BLDG AREA

Runoff = 61.15 cfs @ 12.03 hrs, Volume= 3.873 af, Depth= 7.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

Are	ea (sf)	CN	Description									
24	0,618	98	Paved park	ing, HSG B	В							
	0	98	Paved park	ing, HSG C	C							
;	3,973	98	Paved parking, HSG D									
3	9,809	69	1 9									
	0	79										
	2,620	84	34 50-75% Grass cover, Fair, HSG D									
28	7,020	94 Weighted Average										
4:	2,429		14.78% Per	vious Area	a							
24	4,591		85.22% lmp	ervious Ar	ırea							
Tc I	Length	Slope	,	Capacity	•							
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)								
5.0					Direct Entry,							

Subcatchment PDA-2B: BLDG AREA



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 79

Summary for Subcatchment PDA-3: Area to Wetland DP-3

Runoff = 5.88 cfs @ 12.08 hrs, Volume= 0.429 af, Depth= 4.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

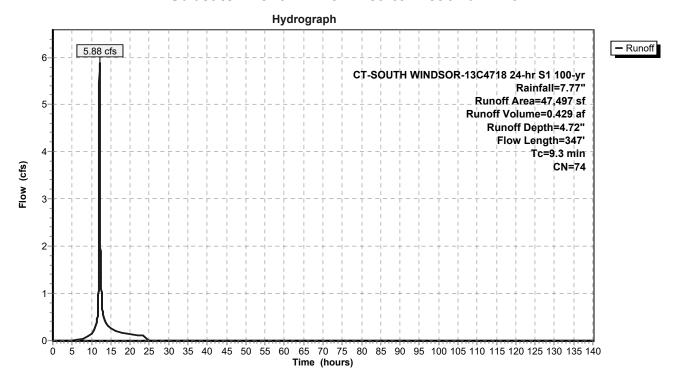
A	rea (sf)	CN [Description											
	63	98 F	Paved park	ing, HSG E	3									
	0	98 F	Paved park	ing, HSG C										
	0	98 F	Paved park	ved parking, HSG D										
	20,845	69 5	0-75% Grass cover, Fair, HSG B											
	0	79 5	50-75% Grass cover, Fair, HSG C											
	7,654	84 5	50-75% Grass cover, Fair, HSG D											
	2,047	56 E	Brush, Fair,	HSG B										
	16,888	77 E	Brush, Fair,	HSG D										
	47,497	74 V	Veighted A	verage										
	47,434	ç	9.87% Per	vious Area	l									
	63	C).13% Impe	ervious Are	a									
Tc	Length	Slope	Velocity	Capacity	Description									
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)										
7.5	100	0.0400	0.22		Sheet Flow, 1									
					Grass: Short n= 0.150 P2= 3.11"									
0.6	125	0.0480	3.29		Shallow Concentrated Flow, 2									
					Grassed Waterway Kv= 15.0 fps									
1.2	122	0.1060	1.63		Shallow Concentrated Flow, 3									
					Woodland Kv= 5.0 fps									
9.3	347	Total												

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 80

Subcatchment PDA-3: Area to Wetland DP-3



HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 81

Summary for Subcatchment PDA-4: Area to Wetland DP-4

Runoff = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af, Depth= 4.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

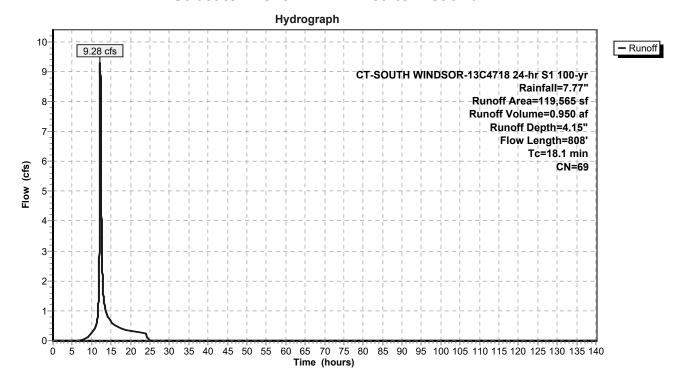
A	rea (sf)	CN E	escription									
	0	98 F	aved park	ing, HSG B	3							
	0	98 F	aved park	ing, HSG C								
	0	98 F	Paved parking, HSG D									
	57,679	69 5	0-75% Gra	ass cover, l	Fair, HSG B							
	26,837	79 5	0-75% Gra	ass cover, l	Fair, HSG C							
	0	84 5	0-75% Gra	ass cover, f	Fair, HSG D							
	25,526	56 E	Brush, Fair,	, HSG B								
	9,523	70 E	<u> Brush, Fair,</u>	, HSG C								
1	19,565	69 V	Veighted A	verage								
1	19,565	1	00.00% Pe	ervious Are	a							
Tc	Length	Slope		Capacity	Description							
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)								
5.7	100	0.0800	0.29		Sheet Flow, 1							
					Grass: Short n= 0.150 P2= 3.11"							
0.2	39	0.0800	4.24		Shallow Concentrated Flow, 2							
					Grassed Waterway Kv= 15.0 fps							
12.1	595	0.0270	0.82		Shallow Concentrated Flow, 3							
					Woodland Kv= 5.0 fps							
0.1	74	0.0270	19.82	194.19	•							
					Area= 9.8 sf Perim= 15.7' r= 0.62'							
					n= 0.009 Corrugated PE, smooth interior							
18.1	808	Total										

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 82

Subcatchment PDA-4: Area to Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 83

Summary for Reach DP-1: Detention Basin 7

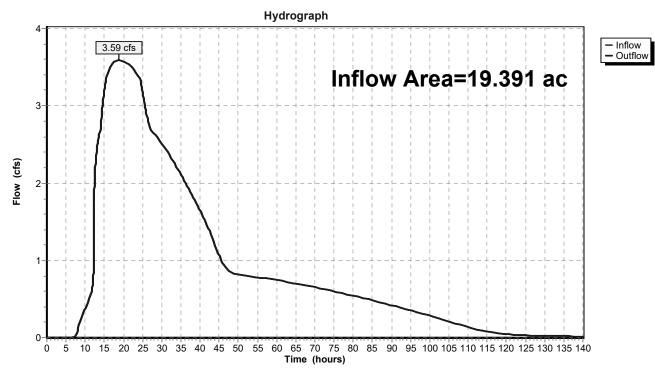
Inflow Area = 19.391 ac, 78.54% Impervious, Inflow Depth > 6.25" for 100-yr event

Inflow = 3.59 cfs @ 18.93 hrs, Volume= 10.105 af

Outflow = 3.59 cfs @ 18.93 hrs, Volume= 10.105 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-1: Detention Basin 7



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 84

Summary for Reach DP-2: Wetland DP-2

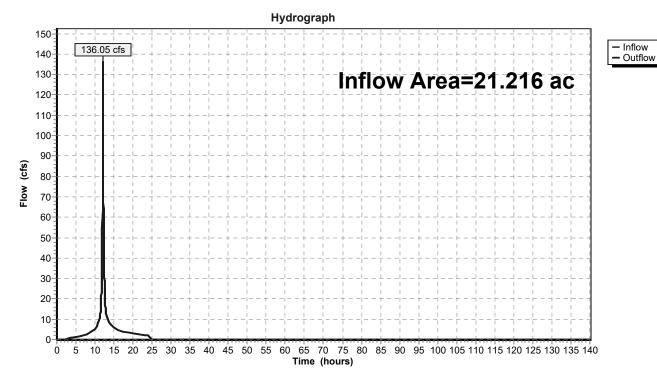
Inflow Area = 21.216 ac, 60.56% Impervious, Inflow Depth = 6.20" for 100-yr event

Inflow = 136.05 cfs @ 12.05 hrs, Volume= 10.957 af

Outflow = 136.05 cfs @ 12.05 hrs, Volume= 10.957 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-2: Wetland DP-2



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 85

Summary for Reach DP-3: Wetland DP-3

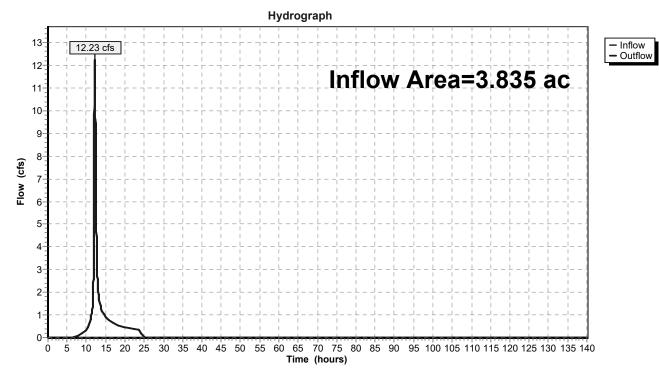
Inflow Area = 3.835 ac, 0.04% Impervious, Inflow Depth = 4.32" for 100-yr event

Inflow = 12.23 cfs @ 12.20 hrs, Volume= 1.379 af

Outflow = 12.23 cfs @ 12.20 hrs, Volume= 1.379 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-3: Wetland DP-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 86

Summary for Reach DP-4: Wetland DP-4

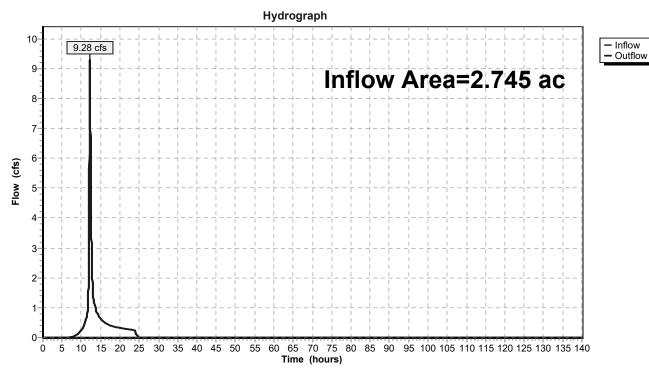
Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 4.15" for 100-yr event

Inflow = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af

Outflow = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Reach DP-4: Wetland DP-4



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 87

Summary for Reach SW 2-3: Wetland Swale 2-3

Inflow Area = 10.792 ac, 67.01% Impervious, Inflow Depth = 6.34" for 100-yr event

Inflow = 80.53 cfs @ 12.06 hrs, Volume= 5.705 af

Outflow = 79.74 cfs @ 12.08 hrs, Volume= 5.705 af, Atten= 1%, Lag= 1.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 9.01 fps, Min. Travel Time= 0.7 min Avg. Velocity = 2.33 fps, Avg. Travel Time= 2.8 min

Peak Storage= 3,511 cf @ 12.07 hrs Average Depth at Peak Storage= 0.78'

Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,486.07 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

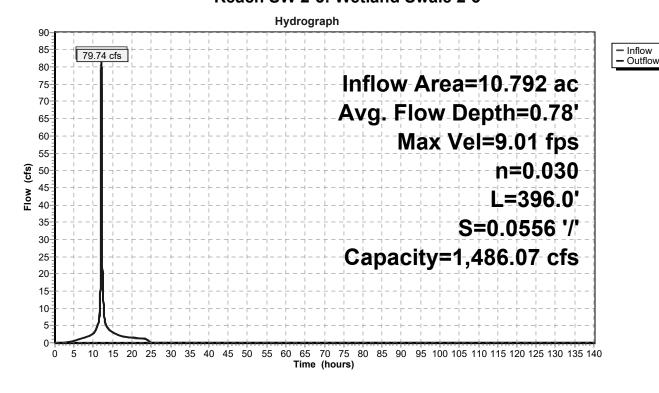
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 396.0' Slope= 0.0556 '/'

Inlet Invert= 127.00', Outlet Invert= 105.00'



Reach SW 2-3: Wetland Swale 2-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 88

Summary for Reach SW 4-3: SW 4-3

Inflow Area = 2.745 ac, 0.00% Impervious, Inflow Depth = 4.15" for 100-yr event

Inflow = 9.28 cfs @ 12.20 hrs, Volume= 0.950 af

Outflow = 9.23 cfs @ 12.24 hrs, Volume= 0.950 af, Atten= 1%, Lag= 2.1 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs

Max. Velocity= 4.07 fps, Min. Travel Time= 1.4 min Avg. Velocity = 1.49 fps, Avg. Travel Time= 3.8 min

Peak Storage= 780 cf @ 12.22 hrs
Average Depth at Peak Storage= 0.22'

Bank-Full Depth= 4.00' Flow Area= 67.2 sf, Capacity= 1,466.70 cfs

10.00' x 4.00' deep channel, n= 0.030 Earth, grassed & winding

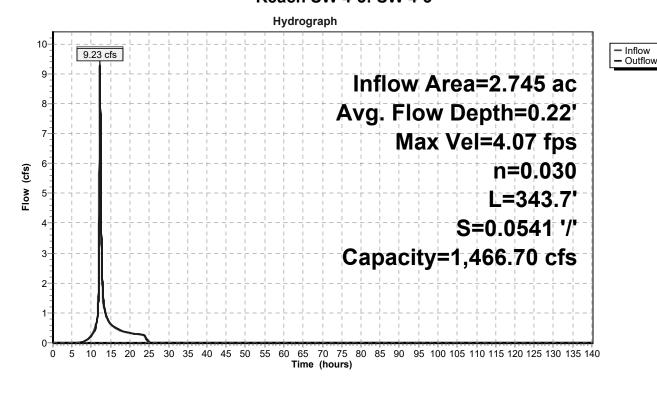
Side Slope Z-value= 1.7 '/' Top Width= 23.60'

Length= 343.7' Slope= 0.0541 '/'

Inlet Invert= 123.60', Outlet Invert= 105.00'



Reach SW 4-3: SW 4-3



Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 89

Summary for Pond P-7: Dentention Basin 7

Inflow Area = 19.391 ac, 78.54% Impervious, Inflow Depth = 6.49" for 100-yr event

Inflow 65.43 cfs @ 12.17 hrs, Volume= 10.479 af

3.59 cfs @ 18.93 hrs, Volume= Outflow 10.105 af, Atten= 95%, Lag= 405.7 min

Primary 3.59 cfs @ 18.93 hrs, Volume= 10.105 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 98.75' @ 18.93 hrs Surf.Area= 36,308 sf Storage= 233,651 cf

Plug-Flow detention time= 1,347.2 min calculated for 10.105 af (96% of inflow)

Center-of-Mass det. time= 1,230.5 min (2,386.3 - 1,155.7)

Volume	Invert	Avail.Sto	rage Storage	Description	
#1	90.00'	280,77	70 cf Custom	Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation	Surf	.Area	Inc.Store	Cum.Store	
(feet)		sq-ft)	(cubic-feet)	(cubic-feet)	
90.00	12	2,621	0	0	
92.00	22	2,814	35,435	35,435	
94.00	26	6,576	49,390	84,825	
96.00	30	0,501	57,077	141,902	
98.00	34	4,640	65,141	207,043	
100.00	39	9,087	73,727	280,770	
Device R	outing	Invert	Outlet Devices	3	

Device	rtouting	IIIVEIL	Odilet Devices
#1	Primary	88.00'	18.0" Round Culvert
			L= 71.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 88.00' / 84.50' S= 0.0493 '/' Cc= 0.900
			n= 0.009 Corrugated PE, smooth interior, Flow Area= 1.77 sf
#2	Device 1	91.00'	4.0" Vert. Orifice/Grate C= 0.600
#3	Device 1	95.00'	6.0" Vert. Orifice/Grate C= 0.600
#4	Device 1	98.00'	6.0" Vert. Orifice/Grate C= 0.600
#5	Device 1	99.00'	36.0" x 78.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

Primary OutFlow Max=3.59 cfs @ 18.93 hrs HW=98.75' (Free Discharge)

-1=Culvert (Passes 3.59 cfs of 26.91 cfs potential flow)

²⁼Orifice/Grate (Orifice Controls 1.16 cfs @ 13.26 fps)

⁻³⁼Orifice/Grate (Orifice Controls 1.77 cfs @ 9.01 fps)

⁻⁴⁼Orifice/Grate (Orifice Controls 0.67 cfs @ 3.40 fps)

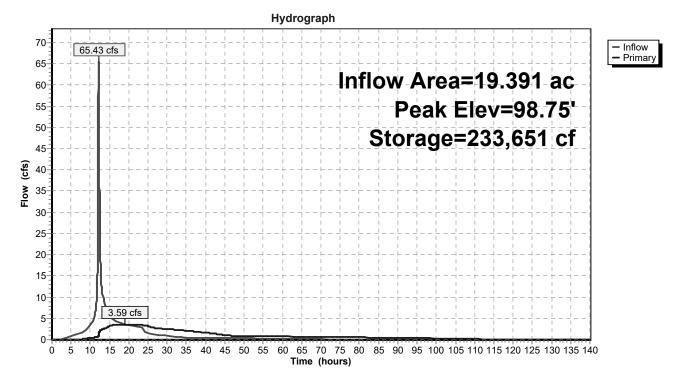
⁻⁵⁼Orifice/Grate (Controls 0.00 cfs)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 90

Pond P-7: Dentention Basin 7



C-DAT-13C4718-PROPOSED CT-SOUTH WINDSOR-13C4718 24-hr S1 100-yr Rainfall=7.77"

Prepared by BL Companies, Inc.

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 91

Summary for Pond S-1: Subsurface Det

Inflow Area = 7.794 ac, 85.88% Impervious, Inflow Depth = 7.05" for 100-yr event

Inflow = 72.33 cfs @ 12.03 hrs, Volume= 4.581 af

Outflow = 4.08 cfs @ 13.17 hrs, Volume= 4.007 af, Atten= 94%, Lag= 68.4 min

Primary = 4.08 cfs @ 13.17 hrs, Volume= 4.007 af

Routing by Stor-Ind method, Time Span= 0.00-140.00 hrs, dt= 0.01 hrs Peak Elev= 106.32' @ 13.17 hrs Surf.Area= 31,034 sf Storage= 127,577 cf

Plug-Flow detention time= 1,049.9 min calculated for 4.007 af (87% of inflow)

Center-of-Mass det. time= 983.7 min (1,748.5 - 764.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	100.25'	49,002 cf	228.33'W x 135.92'L x 6.75'H Field A
			209,482 cf Overall - 86,977 cf Embedded = 122,504 cf x 40.0% Voids
#2A	101.00'	86,977 cf	ADS_StormTech MC-4500 +Capx 800 Inside #1
			Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf
			Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap
			25 Rows of 32 Chambers
			Cap Storage= +35.7 cf x 2 x 25 rows = 1,785.0 cf
		125 070 of	Total Associable Ctanama

135,979 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	101.50'	24.0" Round Culvert
	•		L= 200.0' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 101.50' / 99.50' S= 0.0100 '/' Cc= 0.900
			n= 0.009 Corrugated PE, smooth interior, Flow Area= 3.14 sf
#2	Device 1	101.55'	3.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
#3	Device 1	103.40'	5.0" W x 4.0" H Vert. Orifice/Grate C= 0.600
#4	Device 1	106.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=4.08 cfs @ 13.17 hrs HW=106.32' (Free Discharge)

1=Culvert (Passes 4.08 cfs of 29.58 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.86 cfs @ 10.33 fps)

-3=Orifice/Grate (Orifice Controls 1.11 cfs @ 7.99 fps)

-4=Broad-Crested Rectangular Weir (Weir Controls 2.11 cfs @ 1.63 fps)

Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 92

Pond S-1: Subsurface Det - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-4500 +Cap (ADS StormTech® MC-4500 with cap volume)

Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap Cap Storage= +35.7 cf x 2 x 25 rows = 1,785.0 cf

100.0" Wide + 9.0" Spacing = 109.0" C-C Row Spacing

32 Chambers/Row x 4.02' Long +2.56' Cap Length x 2 = 133.92' Row Length +12.0" End Stone x 2 = 135.92' Base Length

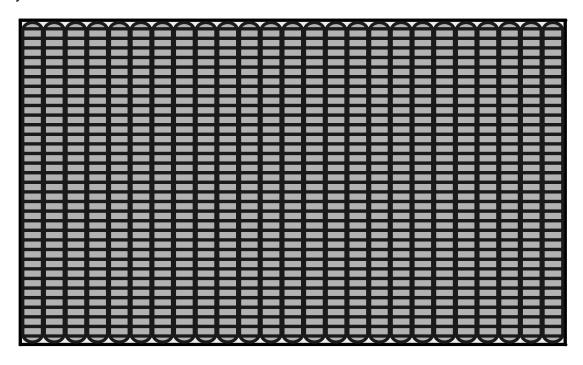
25 Rows x 100.0" Wide + 9.0" Spacing x 24 + 12.0" Side Stone x 2 = 228.33' Base Width 9.0" Base + 60.0" Chamber Height + 12.0" Cover = 6.75' Field Height

800 Chambers x 106.5 cf + 35.7 cf Cap Volume x 2 x 25 Rows = 86,977.3 cf Chamber Storage

209,481.6 cf Field - 86,977.3 cf Chambers = 122,504.2 cf Stone x 40.0% Voids = 49,001.7 cf Stone Storage

Chamber Storage + Stone Storage = 135,979.0 cf = 3.122 af Overall Storage Efficiency = 64.9% Overall System Size = 135.92' x 228.33' x 6.75'

800 Chambers 7,758.6 cy Field 4,537.2 cy Stone



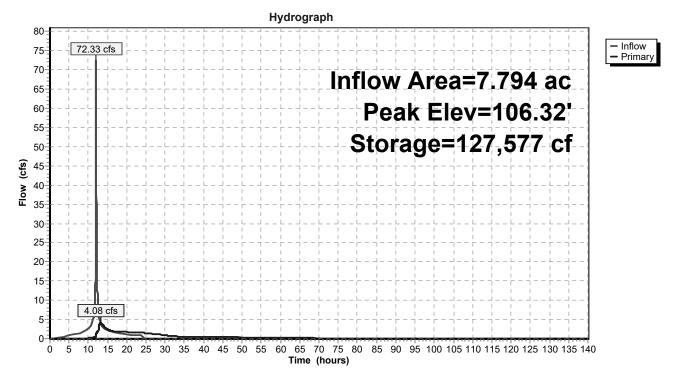


Printed 9/18/2019

HydroCAD® 10.00-21 s/n 01334 © 2018 HydroCAD Software Solutions LLC

Page 93

Pond S-1: Subsurface Det





APPENDIX D

WATER QUALITY CALCULATIONS

CTDEEP Water Quality Volume Calculations
CTDEEP Water Quality Flow Calculations
Treatment Train Efficiency Worksheet
CTDOT Hydrodynamic Separator Sizing
Flow Splitter Manhole Calculations

Water Quality Calculations

Determine Water Quality Volume

From CT 2004 Stormwater Quality Manual:

$$WQV = \frac{(1")(R)(A)}{12}$$

WQV = water quality volume (ac-ft)

R = volumetric runoff coefficient

I = percent impervious cover

A = site area in acres

$$R = 0.05 + 0.009(I)$$

Area	Total	Area	Impervio	ous Area	Impervious Cover	Volumetric Runoff Coefficient	Water Quality Volume (WQV)		
ID	ac ft ²		ac	ft²	%	R	acre-feet ft ³		
PDA-1B_1	4.762	4.762 207,431 4.056		176,659	85.17	0.817	0.324	14,113	
PDA-1B_2	2.847	124,036	2.453	106,866	86.16	0.825	0.196	8,538	
PDA-1B_3	0.341	14,868	0.307	13,387	90.03	90.03 0.860		1,045	
PDA-2	3.079 134,143		2.105	91,714	68.37	0.665	0.171	7,449	

0.715 31,145

Water Quality Calculations

Determine Water Quality Flow

From CT 2004 Stormwater Quality Manual:

$$CN = \frac{1000}{\left[10 + 5P + 10Q - 10(Q^2 + 1.25QP)^{\frac{1}{2}}\right]}$$

$$Q = \frac{[WQV (acre - feet) \times [12(inches / foot)]]}{DrainageArea(acres)}$$

$$WQF = (q_u)(A)(Q)$$

CN = Runoff Curve Number

P = design preciptation, inches, (1" for water quality storm)

Q = runoff depth (in watershed inches)

T_c = time of concentration

I_a = Initial abstraction, inches, from Table 4-1, Chapter 4, TR-55

q_u = unit peak discharge,

WQF = water quality flow (cfs)

	Hydrodynamic	Facility	T	Total Area		Imp Area		Imp Cover	R	WQV	Q	Р	CN		T _c	l _a	I _a /P	q _u ¹	WQF
Area	Separator	ID	ft ²	ac	mi ²	ft ²	ac	%	ı	acre-feet	in	in	-	mins	hours	in	-	cfs/mi ² /in	cfs
PDA-1B_1	HDS-211		207,431	4.762	0.0074	176,659	4.056	85.17	0.817	0.324	0.82	1.00	98	5.0	0.08	0.041	0.041	660	4.03
PDA-1B_2	HDS-204		124,036	2.847	0.0044	106,866	2.453	86.16	0.825	0.196	0.83	1.00	98	5.0	80.0	0.041	0.041	660	2.44
PDA-1B_3	HDS-100		14,868	0.341	0.0005	13,387	0.307	90.03	0.860	0.024	0.84	1.00	99	5.0	0.08	0.041	0.041	660	0.30
PDA-2	HDS-319		134,143	3.079	0.0048	91,714	2.105	68.37	0.665	0.171	0.67	1.00	97	5.0	0.08	0.062	0.062	660	2.13

From Exhibit 4-III: Unit peak discharge (q,) for SCS type III rainfall distribution, Urban Hydrology for Small Watersheds (TR-55), USDS< SCS, June 1986.

Best Management Practice (BMP) Treatment Train Efficiency Worksheet Prepared for: Retail Development Evergreen Walk South Windsor, CT Prepared by: BL Companies 100 Constitution Plaza Hartford, CT Date prepared: September 18, 2019 **Overall Site Treatment Train Efficiency OF-1** Efficiency TSS Removal Starting TSS Amount Remaining BMP Description Type pf Treatment Rate % **BMP** Type pf Treatment Rate Load Removed Load Et=[1-(1-E1)(1-E2)(1-E3)(1-E4)(1-E?)]*100 Impervious Surface Sweeping*** secondary (conventional) Impervious Surface Sweeping*** secondary (conventional) 0.9 catch basin (hoods & deep sumps)*** secondary (conventional) 25 catch basin (hoods & deep sumps)*** secondary (conventional) 0.25 0.9 0.225 0.675 Hydrodynamic Separator** 75 Hydrodynamic Separator ** Secondary 0.75 0.675 0.50625 0.16875 Micropool Extended Detention Pond Micropool Extended Detention Pond 0.16875 0.16875 Overall Treatment Train Efficiency (% 83% 83.125 % Total Suspended Solids (TSS) Remova Overall Treatment Train Efficiency (Et)= * 80% require per CT DEP Manufacturers claim 80% TSS removal

TSS Removal Rates (adapted from Schueler, 1996, & EPA, 1993)

* Schueler 1996 & EPA 1993 ** University of New Hampshire

BMP List	Design	Range of	Brief Design Requirements
	Rate	Average TSS	
		Removal Rates	
Extended Detention Pond	70%	60-80%	Sediment forebay
Wet Pond (a)	70%	60-80%	Sediment forebay
Constructed Wetland (b)	80%	65-80%	Designed to infiltrate or retain
Water Quality Swale	70%	60-80%	Designed to infiltrate or retain
Infiltration Trench	80%	75-80%	Pretreatment critical
Infiltration Basin	80%	75-80%	Pretreatment critical
		(predicted)	
Dry Well	80%	80% (predicted)	Rooftop runoff
			(uncontaminated only)
Sand Filter (c)	80%	80%	Pretreatment
Organic Filter (d)	80%	80%+	Pretreatment
Water Quality Inlet	25%	15-35% w/	Off-line only; 0.1" minimum Water Quality Volume (WQV) storage
		cleanout	
Sediment Trap (Forebay)	25%	25% w/	Storm flows for 2-year event must not cause erosion; 0.1" minimum WQV storage
Drainage Channel	25%	cleanout 25%	Check dams; non-erosive for 2-yr.
-			·
Deep Sump and Hooded Catch	25%	25% w/	Deep sump general rule = 4 x pipe diameter or 4.0' for pipes 18" or less
Basin		cleanout	
Street Sweeping	10%	10%	Discretionary non-structural credit, must be part of approved plan

Best Management Practice (BMP) Treatment Train Efficiency Worksheet Prepared for: Retail Development Evergreen Walk South Windsor, CT Prepared by: BL Companies 100 Constitution Plaza Hartford CT Date prepared: September 18, 2019 **Overall Site Treatment Train Efficiency OF-2** Efficiency TSS Removal Starting TSS Amount Remaining Rate BMP Description Type pf Treatment Rate % **BMP** Type pf Treatment Load Removed Load Et=[1-(1-E1)(1-E2)(1-E3)(1-E4)(1-E?)]*100 Impervious Surface Sweeping*** secondary (conventional) Impervious Surface Sweeping*** secondary (conventional) 0.1 0.1 0.9 E2 catch basin (hoods & deep sumps)*** secondary (conventional) 25 catch basin (hoods & deep sumps)*** secondary (conventional) 0.25 0.9 0.225 0.675 E3 Hydrodynamic Separator** Hydrodynamic Separator ** Secondary 0.675 0.16875 Overall Treatment Train Efficiency (% 83% 83.125 % Total Suspended Solids (TSS) Remova Overall Treatment Train Efficiency (Et)= 80% require per CT DEP * Manufacturers claim 80% TSS removal ** Schueler 1996 & EPA 1993

TSS Removal Rates (adapted from Schueler, 1996, & EPA, 1993)

* University of New Hampshire

BMP List	Design	Range of	Brief Design Requirements
	Rate	Average TSS	
		Removal Rates	
Extended Detention Pond	70%	60-80%	Sediment forebay
Wet Pond (a)	70%	60-80%	Sediment forebay
Constructed Wetland (b)	80%	65-80%	Designed to infiltrate or retain
Water Quality Swale	70%	60-80%	Designed to infiltrate or retain
Infiltration Trench	80%	75-80%	Pretreatment critical
Infiltration Basin	80%	75-80% (predicted)	Pretreatment critical
Dry Well	80%	80% (predicted)	Rooftop runoff
			(uncontaminated only)
Sand Filter (c)	80%	80%	Pretreatment
Organic Filter (d)	80%	80%+	Pretreatment
Water Quality Inlet	25%	15-35% w/	Off-line only; 0.1" minimum Water Quality Volume (WQV) storage
		cleanout	
Sediment Trap (Forebay)	25%	25% w/	Storm flows for 2-year event must not cause erosion; 0.1" minimum WQV storage
		cleanout	
Drainage Channel	25%	25%	Check dams; non-erosive for 2-yr.
Deep Sump and Hooded Catch	25%	25% w/	Deep sump general rule = 4 x pipe diameter or 4.0' for pipes 18" or less
Basin		cleanout	
Street Sweeping	10%	10%	Discretionary non-structural credit, must be part of approved plan

TABLE 2 - PERFORMANCE MATRIX FOR CONNDOT APPROVED HYDRODYNAMIC SEPARATORS

				Product Mo	del				
Maximum WQF (cfs)	Downstream Defender	Flogard	High Eff. CDS	Hydroguard	Stormceptor OSR	Stormceptor STC	Vortechs	Vortsentry	V2B1
0.4	4-ft	DVS-36	2015-4G; 2015-4	HG 4	065	450	1000	VS30	2
0.5	4-ft	DVS-36	2015-4G; 2015-4	HG 4	065	900	1000	VS30	2
0.6	4-ft	DVS-36	2015-4G; 2015-4	HG 4	065	900	1000	VS40	2
0.7	4-ft	DVS-48	2015-4G; 2015-4	HG 4	140	900	1000	VS40	2
0.8	4-ft	DVS-48	2015-4G; 2015-4	HG 4	140	900	1000	VS40	2
0.9	4-ft	DVS-48	2015-4G; 2015-4	HG 4	140	1200	1000	VS40	3
1.0	4-ft	DVS-48	2015-4G; 2015-4	HG 4	140	1800	1000	VS40	3
1.1	4-ft	DVS-48	2015-4G; 2015-4	HG 4	140	1800	1000	VS40	4
1.2	6-ft	DVS-48	2015	HG 5	140	2400	1000	VS50	4
1.3	6-ft	DVS-60	2015	HG 5	140	2400	1000	VS50	4
1.4	6-ft	DVS-60	2015	HG 5	140	2400	2000	VS50	4
1.5	6-ft	DVS-60	2020	HG 5	140	2400	2000	VS50	6
1.6	6-ft	DVS-60	2020	HG 5	140	2400	2000	VS50	6
1.7	6-ft	DVS-60	2020	HG 5	250	2400	2000	VS50	6
1.8	6-ft	DVS-60	2020	HG 6	250	2400	2000	VS50	7
1.9	6-ft	DVS-60	2020	HG 6	250	3600	2000	VS60	7
2.0	6-ft	DVS-60	2020	HG 6	250	3600	2000	VS60	7
2.1	6-ft	DVS-60	2020	HG 6	250	3600	2000	VS60	9
2.2	6-ft	DVS-72	2025	HG 6	250	3600	2000	VS60	8
2.3	6-ft	DVS-72	3020, 3020-D	HG 6	250	3600	2000	VS60	8
2.4	6-ft	DVS-72	3035; 3035-D	HG 6	250	4800	2000	VS60	8
2.5	6-ft	DVS-72	3035; 3035-D	HG 6	250	4800	3000	VS60	10
2.6	6-ft	DVS-72	3035; 3035-D	HG 6	250	4800	3000	VS60	11
2.7	6-ft	DVS-72	3035; 3035-D	HG 7	250	4800	3000	VS60	11
2.8	6-ft	DVS-72	3035; 3035-D	HG 7	250	4800	3000	VS70	11
2.9	6-ft	DVS-72	3035; 3035-D	HG 7	250	4800	3000	VS70	12
3.0	6-ft	DVS-72	3035; 3035-D	HG 7	390	4800	3000	VS70	12

TABLE 2 - PERFORMANCE MATRIX FOR CONNDOT APPROVED HYDRODYNAMIC SEPARATORS (continued)

				Product Mo	del				
Maximum WQF (cfs)	Downstream Defender	Flogard	High Eff. CDS	Hydroguard	Stormceptor OSR	Stormceptor STC	Vortechs	Vortsentry	V2B1
3.1	8-ft	DVS-72	3035; 3035-D	HG 7	390	4800	3000	VS70	12
3.2	8-ft	DVS-72	3035; 3035-D	HG 7	390	4800	3000	VS70	12
3.3	8-ft	DVS-72	3035; 3035-D	HG 7	390	4800	3000	VS70	14
3.4	8-ft	DVS-72	3035; 3035-D	HG 7	390	6000	3000	VS70	14
3.5	8-ft	DVS-72	3030; 3030-DV, 3030-D; 4030-D	HG 7	390	6000	3000	VS70	14
3.6	8-ft	DVS-72	4030	HG 7	390	6000	3000	VS70	14
3.7	8-ft	DVS-84	4030	HG 8	390	6000	3000	VS70	14
3.8	8-ft	DVS-84	4030	HG 8	390	6000	4000	VS70	13
3.9	8-ft	DVS-84	4030	HG 8	390	7200	4000	VS70	15
4.0	8-ft	DVS-84	4030	HG 8	390	7200	4000	VS80	15
4.1	8-ft	DVS-84	4030	HG 8	390	7200	4000	VS80	15
4.2	8-ft	DVS-84	4030	HG 8	390	7200	4000	VS80	16
4.3	8-ft	DVS-84	4030	HG 8	390	7200	4000	VS80	16
4.4	8-ft	DVS-84	4030	HG 8	390	7200	4000	VS80	16
4.5	8-ft	DVS-84	4030	HG 8	390	7200	4000	VS80	16
4.6	8-ft	DVS-84	5640-D	HG 8	390	7200	4000	VS80	17
4.7	8-ft	DVS-84	5640-D	HG 8	390	7200	4000	VS80	17
4.8	8-ft	DVS-84	5640-D	HG 8	390	7200	4000	VS80	17
4.9	8-ft	DVS-84	5640-D	HG 8	390	11000s	4000	VS80	17
5.0	8-ft	DVS-84	5640-D	HG 9	390	11000s	4000	VS80	19
5.2	8-ft	DVS-84	4040-D	HG 9	390	11000s	4000	VS80	20
5.4	8-ft	DVS-96	4040-D	HG 9	390	11000s	4000	VS100	20
5.5	8-ft	DVS-96	4045-D	HG 9	390	11000s	5000	VS100	18
5.6	8-ft	DVS-96	4045-D	HG 9	560	11000s	5000	VS100	18
6.0	8-ft	DVS-96	4040	HG 9	560	11000s	5000	VS100	18
6.1	8-ft	DVS-96	4040	HG 9	560	11000s	5000	VS100	21

TABLE 2 - PERFORMANCE MATRIX FOR CONNDOT APPROVED HYDRODYNAMIC SEPARATORS (continued)

				Product Mo	del				
Maximum WQF (cfs)	Downstream Defender	Flogard	High Eff. CDS	Hydroguard	Stormceptor OSR	Stormceptor STC	Vortechs	Vortsentry	V2B1
6.3	8-ft	DVS-96	4040	HG 9	560	11000s	5000	VS100	25
6.4	10-ft	DVS-96	4040	HG 9	560	11000s	5000	VS100	25
6.5	10-ft	DVS-96	4040	HG 10	560	11000s	5000	VS100	25
6.9	10-ft	DVS-96	4040	HG 10	560	11000s	5000	VS100	25
7.0	10-ft	DVS-96	4040	HG 10	560	11000s	5000	VS100	22
7.1	10-ft	DVS-96	5042-D	HG 10	560	11000s	5000	VS100	22
7.2	10-ft	DVS-96	5042-D	HG 10	560	13000s	5000	VS100	22
7.3	10-ft	DVS-96	4045	HG 10	560	13000s	5000	VS100	22
7.5	10-ft	DVS-96	5653-D	HG 10	560	13000s	7000	VS100	22
7.7	10-ft	DVS-120	5653-D	HG 10	560	13000s	7000	VS100	22
7.8	10-ft	DVS-120	5653-D	HG 10	560	13000s	7000	VS100	50
7.9	10-ft	DVS-120	5653-D	HG 10	780	13000s	7000	VS100	50
8.0	10-ft	DVS-120	5658-D	HG 10	780	13000s	7000	VS100	50
8.2	10-ft	DVS-120	5658-D	HG 10	780	16000s	7000	VS100	50
8.5	10-ft	DVS-120	5658-D	HG 12	780	16000s	7000	VS100	50
8.6	10-ft	DVS-120	5658-D	HG 12	780	16000s	7000	VS100	50
8.9	10-ft	DVS-120	5678-D	HG 12	780	16000s	7000	VS100	50
9.0	10-ft	DVS-120	5678-D	HG 12	780	16000s	7000	VS120	50
9.2	10-ft	DVS-120	5678-D	HG 12	780	16000s	7000	VS120	50
9.5	10-ft	DVS-120	5050-DV	HG 12	780	16000s	7000	VS120	50
9.6	10-ft	DVS-120	5050-DV	HG 12	780	16000s	7000	VS120	50
10.0	10-ft	DVS-120	5050-DV	HG 12	780	16000s	9000	VS120	50
10.1	10-ft	DVS-120	5050-DV	HG 12	780	16000s	9000	VS120	50
10.5	10-ft	DVS-120	5050-DV	HG 12	780		9000	VS120	50
10.9	10-ft	DVS-120	5050-DV	HG 12	780		9000	VS120	50
11.0	10-ft	DVS-120	7070-DV	HG 12	780		9000	VS120	50
11.2	10-ft	DVS-120	7070-DV	HG 12	1125		9000	VS120	50

TABLE 2 - PERFORMANCE MATRIX FOR CONNDOT APPROVED HYDRODYNAMIC SEPARATORS (continued)

				Product Mo	del				
Maximum WQF (cfs)	Downstream Defender	Flogard	High Eff. CDS	Hydroguard	Stormceptor OSR	Stormceptor STC	Vortechs	Vortsentry	V2B1
11.5		DVS-120	7070-DV	HG 12	1125		9000	VS120	50
11.8		DVS-120	7070-DV	HG 12	1125		9000	VS120	50
11.9		DVS-120	7070-DV	HG 12	1125		9000	VS120	60
12.0		DVS-120	7070-DV	HG 12	1125		9000	VS120	60
12.1		DVS-120	7070-DV	HG 12	1125		9000	VS120	60
12.5		DVS-120	7070-DV	HG 12	1125		11000	VS120	60
13.0		DVS-120	7070-DV		1125		11000	VS120	60
13.5		DVS-120	7070-DV		1125		11000	VS120	60
13.6		DVS-120	7070-DV		1125		11000	VS120	60
14.0		DVS-144	7070-DV		1125		11000	VS120	60
14.5		DVS-144	7070-DV		1125		11000		60
14.9		DVS-144	7070-DV		1125		11000		60
15.0		DVS-144	7070-DV		1125		16000		60
15.5		DVS-144	7070-DV		1125		16000		60
15.7		DVS-144	7070-DV		1125		16000		60
16.0		DVS-144	7070-DV				16000		60
16.5		DVS-144	7070-DV				16000		60
17.0		DVS-144	7070-DV				16000		
17.5		DVS-144	7070-DV				16000		
18.0		DVS-144	7070-DV				16000		
18.5		DVS-144	7070-DV				16000		
19.0		DVS-144	7070-DV				16000		
19.7		DVS-144	7070-DV				16000		
20.0		DVS-144	10060-DV				16000		
21.5		DVS-144	10060-DV				16000		
22.3		DVS-144	10060-DV				1319		
25.0			10060-DV				1319		
25.2			10060-DV				1319		

TABLE 2 - PERFORMANCE MATRIX FOR CONNDOT APPROVED HYDRODYNAMIC SEPARATORS (continued)

				Product Mo	del				
Maximum WQF (cfs)	Downstream Defender	Flogard	High Eff. CDS	Hydroguard	Stormceptor OSR	Stormceptor STC	Vortechs	Vortsentry	V2B1
27.6			10060-DV				1421		
29.3			10080-DV				1421		
30.0			10080-DV				1522		
31.2			10080-DV				1522		
33.6			100100-DV				1522		
35.0			100100-DV				1624		
38.2			100100-DV				1624		
40.0			100100-DV				1726		
43.2			100100-DV				1726		
49.3			100100-DV						

TABLE 3 - STANDARD SEDIMENT STORAGE CAPACITY FOR CONNDOT APPROVED HYDRODYNAMIC SEPARATORS

Sediment				Product Mod	lel				
Storage (cubic yards)	Downstream Defender	Flogard	High Eff. CDS	Hydroguard	Stormceptor OSR	Stormceptor STC	Vortechs	Vortsentry	V2B1
0.3		DVS-36					1000		
0.5	4-ft								
0.6							2000		
0.7		DVS-48		HG 4					
0.8					065	450		VS30	2; 3
0.9			2015-4G; 2015-4						
1.0 (minimum)							3000		
1.1					140	900			
1.2				HG 5					
1.3		DVS-60							
1.4							4000	VS40	
1.5			2015; 2020; 2025						
1.6									4
1.7				HG 6					
1.8	6-ft					1200			
1.9							5000		
2.0									
2.1									
2.2		DVS-72						VS50	
2.3				HG 7					
2.4									6; 7
2.5							7000		
2.6			3020, 3020-D; 3030, 3030-DV, 3030-D; 3035, 3035-D						
2.9					250	2400			

TABLE 3 - STANDARD SEDIMENT STORAGE CAPACITY FOR CONNDOT APPROVED HYDRODYNAMIC SEPARATORS (continued)

Sediment				Product Mode	l				
Storage (cubic yards)	Downstream Defender	Flogard	High Eff. CDS	Hydroguard	Stormceptor OSR	Stormceptor STC	Vortechs	Vortsentry	V2B1
3.0				HG 8					
3.1							9000	VS60	
3.2									8; 9
3.3						1800			
3.4									
3.5		DVS-84							
3.6									
3.7	8-ft		5640-D						
3.8				HG 9					
3.9							11000		
4.0									
4.2									10; 11; 12
4.3			4030-D; 4040-D; 4045-D					VS70	
4.5									
4.6									
4.7									13
5.0				HG 10					
5.1									
5.3		DVS-96	5042-DV; 5050-DV						
5.5									
5.6			4030; 4040; 4045; 5653-D; 5658-D; 5678-D				16000	VS80	
5.7									
6.0						3600			
6.5		_					-	_	

TABLE 3 - STANDARD SEDIMENT STORAGE CAPACITY FOR CONNDOT APPROVED HYDRODYNAMIC SEPARATORS (continued)

Sediment	Product Model								
Storage (cubic yards)	Downstream Defender	Flogard	High Eff. CDS	Hydroguard	Stormceptor OSR	Stormceptor STC	Vortechs	Vortsentry	V2B1
6.6							1319		
6.9									
7.0									
7.1									
7.2									
7.3									14; 15; 16; 17; 18
7.5				HG 12					
7.6							1421		
7.7									
8.0									
8.3									
8.4			7070-DV						
8.6						4800			
8.7	10-ft				390		1522	VS100	
9.0									
9.5									
9.6									
9.9							1624		
10.0									
10.3		DVS-120							
10.5									19; 20
11.0									
11.2							1726		
11.3						6000			
11.5									21; 22
11.8									

TABLE 3 - STANDARD SEDIMENT STORAGE CAPACITY FOR CONNDOT APPROVED HYDRODYNAMIC SEPARATORS (continued)

Sediment				Product Mod	lel				-
Storage (cubic yards)	Downstream Defender	Flogard	High Eff. CDS	Hydroguard	Stormceptor OSR	Stormceptor STC	Vortechs	Vortsentry	V2B1
12.0									
12.6								VS120	25
12.9					560				
13.0									
13.4						7200			
15.0									
17.5					780				
17.8		DVS-144	10060-DV;10080-DV; 100100-DV						
20.0									
22.3									50
25.0									
25.8					1125				
26.1						11000s			
26.2									
30.0									
34.1						13000s			
34.9									60
35.0									
38.7									
40.0									
40.7						16000s			

PROJECT	Retail Development - Evergreen Walk, South Windsor CT	PREPARED BY	J. Bates
DATE	9/18/2019	CHECKED BY	
SUBJECT	Splitter Manhole Calculation		

HDS-211:

WQF = 4.03 cfs

Low Flo	ow Orifice	Overflow Weir			Orifice A	Orifice "C"
Dia. (In.)	Inv. Elev.	Bottom El.	Height (Ft.)	Length (Ft.)	(s.f.)	
18	103.80	103.80	1.00	4.00	1.767	0.6

W.S. Elev.	H (Orifice)	Q (Orifice)	H (Weir)	Q (Weir)	Q (Total)
ft.	ft.	cfs	ft.	cfs	cfs
104.80	0.25	4.25	0.00	0.00	4.25
104.90	0.35	5.03	0.10	0.42	5.45
105.00	0.45	5.71	0.20	1.19	6.90
105.10	0.55	6.31	0.30	2.19	8.50
105.20	0.65	6.86	0.40	3.37	10.23
105.30	0.75	7.37	0.50	4.71	12.08
105.40	0.85	7.84	0.60	6.19	14.04
105.50	0.95	8.29	0.70	7.80	16.09
105.60	1.05	8.72	0.80	9.53	18.25
105.70	1.15	9.12	0.90	11.37	20.50
105.80	1.25	9.51	1.00	13.32	22.83
105.90	1.35	9.89	1.10	15.37	25.25
106.00	1.45	10.25	1.20	17.51	27.76
106.10	1.55	10.59	1.30	19.74	30.34
106.20	1.65	10.93	1.40	22.06	32.99
106.30	1.75	11.26	1.50	24.47	35.73
106.40	1.85	11.57	1.60	26.96	38.53
106.50	1.95	11.88	1.70	29.52	41.41
106.60	2.05	12.18	1.80	32.17	44.35
106.70	2.15	12.48	1.90	34.88	47.36
106.80	2.25	12.76	2.00	37.67	50.44
106.90	2.35	13.04	2.10	40.54	53.58
107.00	2.45	13.32	2.20	43.46	56.78
107.10	2.55	13.59	2.30	46.46	60.05
107.20	2.65	13.85	2.40	49.52	63.38
107.30	2.75	14.11	2.50	52.65	66.76
107.40	2.85	14.36	2.60	55.84	70.21
107.50	2.95	14.61	2.70	59.09	73.71
107.60	3.05	14.86	2.80	62.41	77.27
107.70	3.15	15.10	2.90	65.78	88.08
107.80	3.25	15.34	3.00	69.21	84.55
107.90	3.35	15.57	3.10	72.70	88.28
108.00	3.45	15.80	3.20	76.25	92.05
108.10	3.55	16.03	3.30	79.85	95.88
108.20	3.65	16.26	3.40	83.51	99.76
108.25	3.70	16.37	3.45	85.36	101.72
108.30	3.75	16.48	3.50	87.22	103.69
108.40	3.85	16.69	3.60	90.98	107.68

PROJECT	Retail Development - Evergreen Walk, South Windsor CT	PREPARED BY	J. Bates
DATE	9/18/2019	CHECKED BY	
SUBJECT	Splitter Manhole Calculation		

HDS-204:

WQF = 2.44 cfs

Low Flo	ow Orifice	Overflow Weir			Orifice A	Orifice "C"
Dia. (In.)	Inv. Elev.	Bottom El.	Height (Ft.)	Length (Ft.)	(s.f.)	
15	104.90	104.90	0.80	4.00	1.227	0.6

W.S. Elev.	H (Orifice)	Q (Orifice)	H (Weir)	Q (Weir)	Q (Total)
ft.	ft.	cfs	ft.	cfs	cfs
105.70	0.17	2.47	0.00	0.00	2.47
105.80	0.27	3.10	0.10	0.42	3.52
105.90	0.37	3.62	0.20	1.19	4.81
106.00	0.47	4.07	0.30	2.19	6.26
106.10	0.57	4.48	0.40	3.37	7.85
106.20	0.67	4.85	0.50	4.71	9.56
106.30	0.77	5.20	0.60	6.19	11.39
106.40	0.87	5.53	0.70	7.80	13.33
106.50	0.97	5.83	0.80	9.53	15.37
106.60	1.07	6.13	0.90	11.37	17.50
106.70	1.17	6.40	1.00	13.32	19.72
106.80	1.27	6.67	1.10	15.37	22.04
106.90	1.37	6.93	1.20	17.51	24.44
107.00	1.47	7.18	1.30	19.74	26.92
107.10	1.57	7.42	1.40	22.06	29.48
107.20	1.67	7.65	1.50	24.47	32.12
107.30	1.77	7.87	1.60	26.96	34.83
107.40	1.87	8.09	1.70	29.52	37.61
107.50	1.97	8.30	1.80	32.17	40.47
107.60	2.07	8.51	1.90	34.88	43.40
107.70	2.17	8.71	2.00	37.67	46.39
107.80	2.27	8.91	2.10	40.54	49.45
107.90	2.37	9.11	2.20	43.46	52.57
108.00	2.47	9.30	2.30	46.46	55.76
108.10	2.57	9.48	2.40	49.52	59.01
108.20	2.67	9.66	2.50	52.65	62.32
108.30	2.77	9.84	2.60	55.84	65.69
108.40	2.87	10.02	2.70	59.09	69.11
108.50	2.97	10.19	2.80	62.41	72.60
108.60	3.07	10.36	2.90	65.78	76.14
108.70	3.17	10.53	3.00	69.21	79.74
108.80	3.27	10.69	3.10	72.70	83.40
108.90	3.37	10.85	3.20	76.25	87.10
109.00	3.47	11.01	3.30	79.85	90.86
109.10	3.57	11.17	3.40	83.51	94.68
109.15	3.62	11.25	3.45	85.36	96.61
109.20	3.67	11.33	3.50	87.22	98.55
109.30	3.77	11.48	3.60	90.98	102.46

PROJECT	Retail Development - Evergreen Walk, South Windsor CT	PREPARED BY	J. Bates
DATE	9/18/2019	CHECKED BY	
SUBJECT	Splitter Manhole Calculation		

HDS-319:

WQF = 2.13 cfs

Low Flo	w Orifice	w Orifice Overflow Weir		Orifice A	Orifice "C"	
Dia. (In.)	Inv. Elev.	Bottom El.	Height (Ft.)	Length (Ft.)	(s.f.)	
15	105.40	105.40	0.80	4.00	1.227	0.6

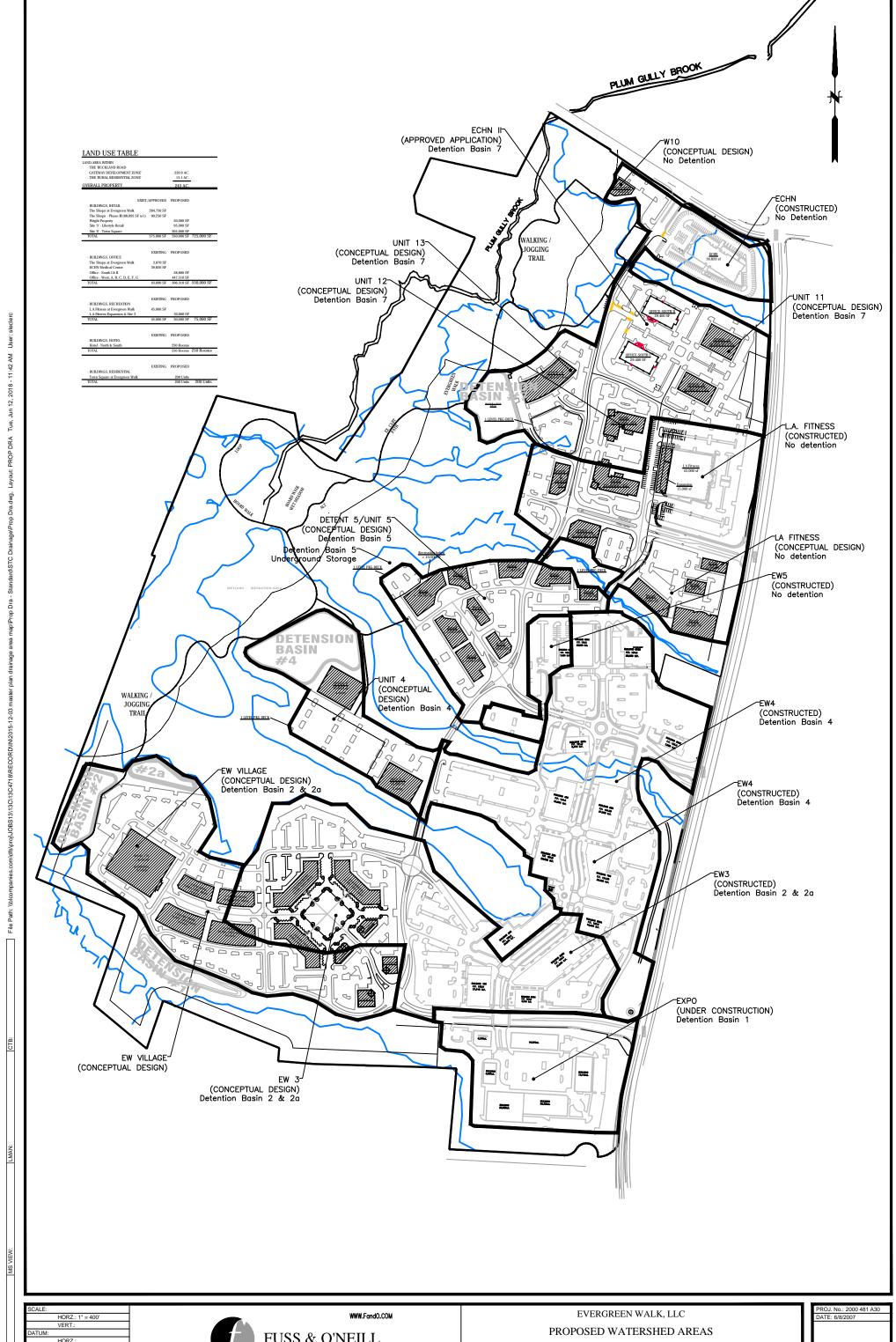
W.S. Elev.	H (Orifice)	Q (Orifice)	H (Weir)	Q (Weir)	Q (Total)
ft.	`ft.	cfs	`ft.	cfs	cfs
106.20	0.17	2.47	0.00	0.00	2.47
106.30	0.27	3.10	0.10	0.42	3.52
106.40	0.37	3.62	0.20	1.19	4.81
106.50	0.47	4.07	0.30	2.19	6.26
106.60	0.57	4.48	0.40	3.37	7.85
106.70	0.67	4.85	0.50	4.71	9.56
106.80	0.77	5.20	0.60	6.19	11.39
106.90	0.87	5.53	0.70	7.80	13.33
107.00	0.97	5.83	0.80	9.53	15.37
107.10	1.07	6.13	0.90	11.37	17.50
107.20	1.17	6.40	1.00	13.32	19.72
107.30	1.27	6.67	1.10	15.37	22.04
107.40	1.37	6.93	1.20	17.51	24.44
107.50	1.47	7.18	1.30	19.74	26.92
107.60	1.57	7.42	1.40	22.06	29.48
107.70	1.67	7.65	1.50	24.47	32.12
107.80	1.77	7.87	1.60	26.96	34.83
107.90	1.87	8.09	1.70	29.52	37.61
108.00	1.97	8.30	1.80	32.17	40.47
108.10	2.07	8.51	1.90	34.88	43.40
108.20	2.17	8.71	2.00	37.67	46.39
108.30	2.27	8.91	2.10	40.54	49.45
108.40	2.37	9.11	2.20	43.46	52.57
108.50	2.47	9.30	2.30	46.46	55.76
108.60	2.57	9.48	2.40	49.52	59.01
108.70	2.67	9.66	2.50	52.65	62.32
108.80	2.77	9.84	2.60	55.84	65.69
108.90	2.87	10.02	2.70	59.09	69.11
109.00	2.97	10.19	2.80	62.41	72.60
109.10	3.07	10.36	2.90	65.78	76.14
109.20	3.17	10.53	3.00	69.21	79.74
109.30	3.27	10.69	3.10	72.70	83.40
109.40	3.37	10.85	3.20	76.25	87.10
109.50	3.47	11.01	3.30	79.85	90.86
109.60	3.57	11.17	3.40	83.51	94.68
109.65	3.62	11.25	3.45	85.36	96.61
109.70	3.67	11.33	3.50	87.22	98.55
109.80	3.77	11.48	3.60	90.98	102.46



APPENDIX E

DRAINAGE MAPS

DRA-1 – Existing Master Plan Drainage Areas ED-1 – Existing Drainage Mapping PD-1 – Proposed Drainage Mapping



GRAPHIC SCALE

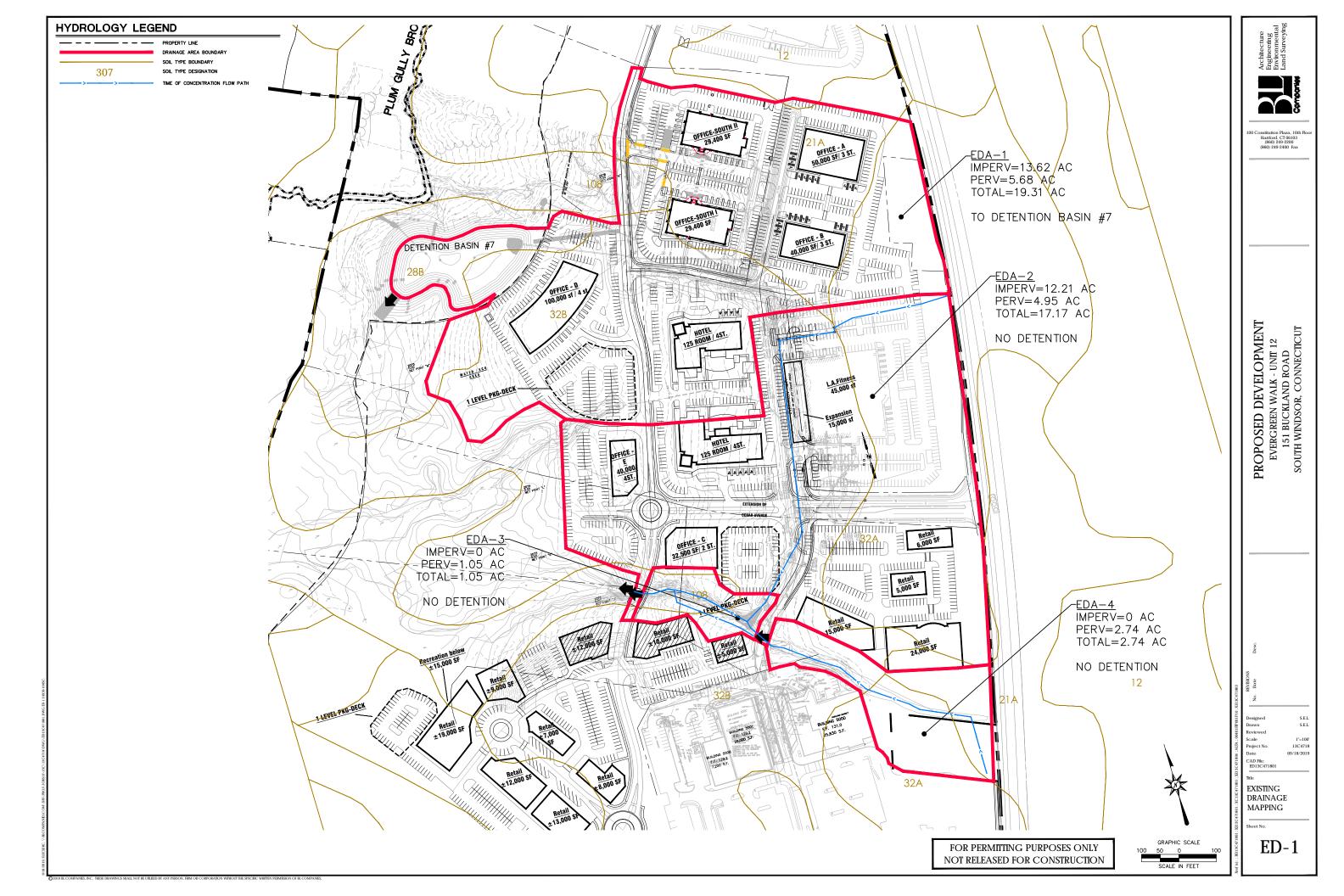
146 HARTFORD RD

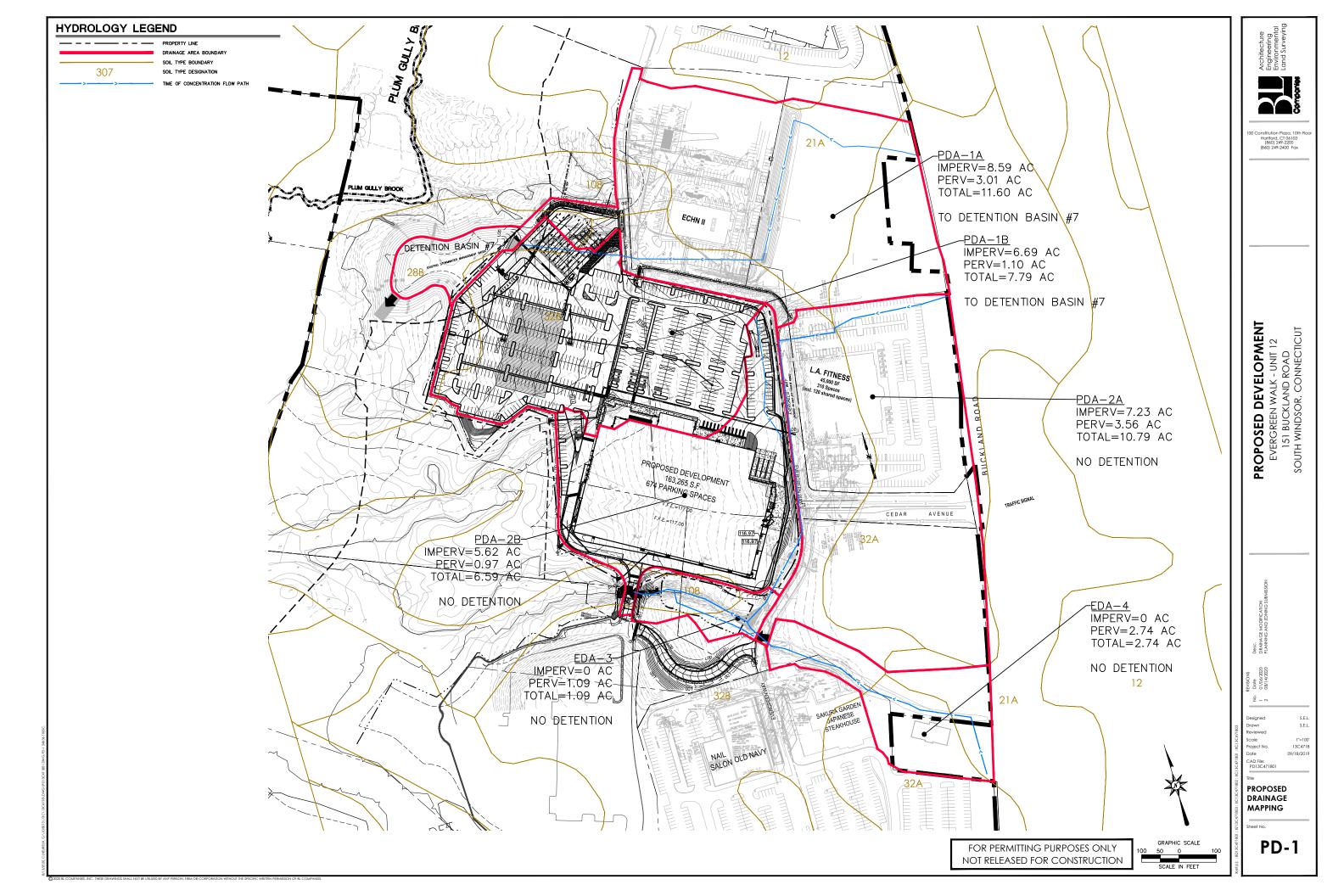
FUSS & O'NEILL Disciplines to Deliver MANCHESTER, CT 06040 860.646.2469 EVERGREEN WALK MASTER DEVELOPMENT PLAN

SOUTH WINDSOR

DRA-1

CONNECTICUT







APPENDIX F

STORMWATER SYSTEM OPERATION AND MAINTENANCE MANUAL

Appendix F:

Stormwater System Operations and Maintenance Plan

For the Proposed:

Retail Development at Evergreen Walk, Unit 12

Located at: 151 Buckland Road South Windsor, CT

Prepared for Submission to:
Town of South Windsor
Land Use Agencies

September 18, 2019 Revised August 14, 2020

Prepared by:



BL Companies

100 Constitution Plaza, 10th Floor Hartford, Connecticut 06103 Phone: (860) 249-2200

Fax: (860) 249-2400

Table of Contents

GENERAL OVERVIEW	1
PURPOSE & GOALS RESPONSIBLE PARTIES LIST OF PERMITS & SPECIAL CONDITIONS MAINTENANCE LOGS AND CHECKLISTS FORMS EMPLOYEE TRAINING SPILL CONTROL	1 2 2
STORM WATER MANAGEMENT	3
SYSTEM COMPONENTS	3 4 4
SITE MAINTENANCE	5
PARKING LOTS LANDSCAPING TRASH COLLECTION OUTDOOR STORAGE SNOW REMOVAL & STORAGE	5 5
UTILITIES	6
SANITARY SEWER SYSTEM	6 6

General Overview

The proposed project is to develop Unit 12 at Evergreen Walk with a Costco with fueling station. The building pad site consists of approximately 16.2 acres of the entire development parcel with a total area of approximately 240 acres. Unit 12 is part of the Evergreen Walk Master Plan Area.

The proposed site improvements will include the proposed Costco building, fueling station, paved parking areas, landscaped areas, pedestrian sidewalks, site utilities, and a stormwater management system. As a master planned development, the existing stormwater management system has been designed to convey the stormwater discharge from the previous approved developed site conditions. The current proposed site development will increase the amount of impervious ground cover on-site as well as a reduction of storage volume in Detention Basin #7 as necessary to accommodate the needs of the business. The existing stormwater management system will be supplemented with a subsurface detention system in order to maintain stormwater runoff rates as approved in the master plan.

The following Operations and Maintenance Plan was prepared specifically for this site development in South Windsor, Connecticut. The Plan was developed to satisfy the South Windsor Inland Wetland and Watercourse and Zoning Commissions.

Purpose & Goals

The purpose of this Manual is ensuring that the site is operated in accordance with all approvals and permits. The primary goal is to inform all the property managers about how the system operates and what maintenance items are necessary to protect downstream wetlands and watercourses. The secondary goal is to provide a practical, efficient means of maintenance planning and record keeping to verify permit compliance.

Responsible Parties

The property owner will be responsible for implementing the Plan on the property. The party may retain a management company to oversee the maintenance of the site.

Some utilities located on the site will be owned and maintained by the various utility companies in accordance with their standards. The property owner may maintain the service connections.

List of Permits & Special Conditions

The site will receive a number of permits, which may contain special conditions that require compliance by the owners and maintenance contractors. These permits may include the following:

Town of South Windsor:

Planning and Zoning Commission: Site Plan Approval.

Inland Wetlands and Water Course Commission: Inland Wetlands Approval

State of Connecticut:

OSTA

Maintenance Logs and Checklists

The property owner will keep a record of all maintenance procedures performed, date of inspection/ cleanings, etc. Copies of inspection reports and maintenance records shall be kept on site in the facility manager's office once it is established.

Forms

The following forms will be developed for annual maintenance. Copies of the forms will be kept on-site as part of the Storm Water Management Plan.

- Annual Checklist
- Quarterly Checklist
- Monthly Checklist

Employee Training

The site will have an employee-training program, with annual up-dates, to ensure that the employees charged with maintaining the site do so in accordance with the approved permit conditions. All sub-contractors (Vactor, landscaping, snowplowing, etc.) will be informed of requirements and responsibilities.

Spill Control

In addition to the good housekeeping and material management practices discussed in the previous sections of this plan, the following practices will be followed for spill prevention and clean-up:

- Manufacturer's recommended methods for spill clean-up will be clearly posted and site personnel will be made aware of the procedures and the location of the information and clean-up supplies.
- Materials and equipment necessary for spill clean-up will be kept in the material storage area on-site. Equipment and materials will include but not be limited to: absorbent booms or mats, brooms, dust pans, mops, rags, gloves, goggles, sand, and plastic and metal trash containers specifically for this purpose.
- All spills will be cleaned immediately after discovery.
- The spill area will be kept well-ventilated and personnel will wear appropriate protective clothing to prevent injury from contact with hazardous substance.
- Spills of toxic or hazardous material, regardless of size, will be reported to the appropriate State or local government agency.
- If a spill occurs, this plan will be adjusted to include measures to prevent this type of spill from reoccurring and how to clean the spill if there is another one. A description of the spill, the cause, and the remediation measures will also be included.

September 18, 2019 Revised August 14, 2020

A spill report shall be prepared by the property owner following each occurrence. The spill report shall present a description of the release, including quantity and type of material, date of spill, circumstances leading to the release, location of spill, response actions and personnel, documentation of notifications and corrective measures implemented to prevent reoccurrence.

The property owner shall identify an appropriately <u>qualified and trained</u> site employee involved with day-to-day site operations to be the spill prevention and clean-up coordinator. The name(s) of responsible spill personnel shall be posted on-site. Each employee shall be instructed that all spills are to be reported to the spill prevention and clean-up coordinator.

The operators of the fuel facility will be trained on the operation and maintenance of the isolation gate valves installed on the stormwater management system. In the event of a fuel spill that could potentially be captured by the stormwater management system the gave valves will immediately be closed to prevent any contaminants from discharging from the site. Isolation gate valves shall be actuated once yearly to ensure function and shall be maintained as needed.

Storm Water Management

System Components

The storm water management system has several components that are shown on the Grading and Drainage drawings (GD-1, and GD-2), and they perform various functions in treating storm water runoff:

Catch Basins are inlets, which trap road sand and floatable debris prior to draining through the storm sewer system. The proposed catch basins (CBs) are equipped with deep sumps with sump depths 4' below the outlet pipe, and hoods over the outlet pipes.

Catch Basins and Manholes

The property owner is responsible for cleaning the catch basins and manholes on the property. A Connecticut Licensed hauler shall clean the sumps and dispose of removed sand legally. The road sand may be reused for winter sanding but may not be stored on-site. As part of the hauling contract, the hauler shall notify the property owner in writing where the material is being disposed.

Each catch basin shall be inspected every four months, with one inspection occurring during the month of April. Any debris occurring within one foot from the bottom of each sump shall be removed by Vacuum "Vactor" type of maintenance equipment.

During the inspection of each of the catch basin sumps, the hoods (where provided) on each of the outlet pipes shall also be observed. In the event that a hood is damaged or off the hanger, it shall be reset or repaired.

Hydrodynamic Separators

The hydrodynamic separator manholes will be cleaned periodically during construction, and at the end of construction once the landscaped areas are fully stabilized.

For the first year of operation following construction, inspect each manhole once each month for the months of January, February, March and April, and once every four months thereafter. A graduated measuring device (stadia rod) shall be inserted into each grit chamber and measurements of any accumulations shall be recorded. Any debris, which has accumulated to within one foot of the water surface inside the grit chamber portion of each tank, shall be removed by vacuum "Vactor" type of equipment.

After the first year of operation, each manhole shall be inspected at a minimum, three times yearly with one inspection occurring in the month of April in the same manner as described above for the first season of operation. Any accumulations found to be occurring within one foot of the water surface shall be removed from the manhole and properly disposed off-site. Also, any floating material discovered during inspections shall be removed from the tank.

A detailed maintenance logbook shall be kept for each manhole. Information is to include, but not be limited to, the date of inspection, record of grit depth, condition of baffles, observation of any floatable, and date of cleaning performed.

Subsurface Stormwater Management Systems (underground Detention Systems)

The underground detention systems shall be inspected every six months in the months of April and October. Each of the inspection ports provided shall be opened and visually checked from the surface. Observation of grit inside of the detention system shall be noted and any deposits found to be 2 inches or more, as measured from the invert of pipe, shall be cleaned and removed. The underground detention system qualifies as a Confined Space under OSHA regulations, and any maintenance involving entry into the pipes should comply with OSHA Confined Space Entry Regulations.

Underground detention systems may be installed with an isolation gate valve (spill control device) as indicated on the Grading and Drainage Plan. Isolation gate valves shall be test operated to ensure functionality and lubricated when the underground detention system is inspected every six months in the months of April and October.

Rip Rap Aprons

The riprap aprons or swales are excavated depressions which are lined with rock riprap to prevent scouring. The depressions permit the dissipation of excessive energy and turbulence associated with the flow of stormwater being discharged from a conduit system.

Management actions include the following measures:

 Inspect the surface of the scour hole quarterly for the first year and adjust as necessary (but at least annually) to ensure surface is free of debris and the discharge is flowing via sheet flow and not concentrated. Remove accumulated sediment when sediment depth

- within the scour hole reaches 50% of the total depth. Frequency of cleaning depends on loading rate.
- Inspect the discharge lip area for low points and down gradient flow areas for active scour or soil erosion. Repair scour and rills with compacted sandy till, and riprap as needed to prevent scouring.

Site Maintenance

Parking Lots

Parking lots, driveways and sidewalks shall be swept regularly by the property owner to clean trash and other debris. The property owner will sweep parking lots on its property in the spring to remove winter accumulations of road sand.

Landscaping

The property owner will maintain landscaped areas. Normally the landscaping maintenance will consist of pruning, mulching, planting, mowing lawns, raking leaves, etc. Use of fertilizers and pesticides will be controlled and limited to minimal amounts necessary for healthy landscape maintenance.

Soil tests, possibly by the Connecticut Cooperative Extension Service, will be performed prior to fertilization. Trees, shrub and lawn fertilization will be done according to the recommendations of the soil test report. Liming of lawn areas to control pH will be done in the spring if testing indicates that it is necessary. The detention basin, drainage channels, and low-maintenance slopes will not be fertilized following initial planting and stabilization.

The lawn areas, once established, will be maintained at a typical height of $2\frac{1}{2}$ "-3". This will allow the grass to be maintained with minimal impact from weeds and/or pests. The low-maintenance slope areas will be maintained as a meadow or allowed to revert back to natural conditions.

Pesticides will only be used as a control method when a problem has been clearly identified and other natural control methods are not successful. All pesticide applications shall be by licensed applicators, where necessary.

Topsoil, brush, leaves, clippings, woodchips, mulch, and other material shall be stored off site.

Trash Collection

All trash will be contained in a self-contained trash container on site with access to an exterior door. The self-contained trash container will be enclosed. All trash will be collected on a regular basis and disposed of legally off-site.

Outdoor Storage

There will be no outdoor storage of hazardous chemicals, fertilizer, pesticides, or herbicides anywhere in the site.

Clean wooden pallets and baled cardboard may be stored outside periodically. These items will be removed from the site on a regular schedule.

Snow Removal & Storage

Snow shall be shoveled and plowed from sidewalks, driveways and parking areas as soon as practical during and after winter storms and stormed in snow storage areas on site where practical.

Utilities

Sanitary Sewer System

On-Site Collection Sewer: The property owner will annually inspect the manholes within the onsite sewer system on the property, and check for debris and blockage. Any low-flow lines with accumulations will be cleaned with water-jetting.

Water System

The on-site mains, fire hydrants, and off-site mains will be owned and maintained by others. The property owner will be responsible for maintaining the domestic and fire service lines to the buildings.

Gas\Electric\Telephone\Cable TV System

The electric system will be owned and maintained up to the switches and transformers by Eversource (aka CL&P). The gas system on site will be owned and maintained by the property owner and gas will be provided by Eversource (aka Yankee Gas). The property owner will maintain the secondary lines from the transformers to the buildings. The telephone system will be owned and maintained by Frontier (aka AT&T) up to the buildings. The cable TV system will be owned and maintained by Comcast of Connecticut.

Site Lighting

The property owner is responsible for maintaining the parking lot, driveway and building-mounted lights on the property.

MAINTENANCE SCHEDULE

During the First Year of	Operation:	
Task:	Completion Date:	Manager's Initials:
JANUARY:		
Employee Training Program with Spill Program		
*Catch Basin and HDS Inspection		
*Subsurface Stormwater System and Basin Inspection		
FEBRUARY:		
*Catch Basin and HDS Inspection		
MARCH:		
*Catch Basin and HDS Inspection		
APRIL:		
*Catch Basin and HDS Inspection		
*Subsurface Stormwater System and Basin Inspection		
Sweeping of Paved Surfaces		
Shrub Fertilization		
Lawn Liming (if necessary)		
JUNE:		
*Catch Basin and HDS Inspection		
Sweeping of Paved Surfaces		
SEPTEMBER:		
*Subsurface Stormwater System and Basin Inspection		
Sweeping of Paved Surfaces		
Tree and Lawn Fertilization		
DECEMBER:		
*Catch Basin and HDS Inspection		
*Subsurface Stormwater System and Basin Inspection		
*Bioretention Basin (Rain Garden) Inspection		
Sweeping of Paved Surfaces		

^{*}NOTE: Use appropriate worksheet found in this plan to conduct the inspection.

	After the First Year of	Operation:	
	FOR YEAR		
		Completion	
Task:		Date:	Manager's Initials:
	JANUARY:		<u> </u>
Employee Training Program v	vith Spill Program		
*Catch Basin and HDS Inspec			
·	APRIL:	-	
*Catch Basin and HDS Inspec	tion		
*Subsurface Stormwater Syst	em and Basin Inspection		
*Bioretention Basin (Rain Gai	rden) Inspection		
Sweeping of Paved Surfaces			
Shrub Fertilization			
Lawn Liming (if necessary)			
	JUNE:	·	
Sweeping of Paved Surfaces			
	SEPTEMBER:		
*Subsurface Stormwater Syst	em and Basin Inspection		
*Catch Basin and HDS Inspec	tion		
Sweeping of Paved Surfaces			
Tree and Lawn Fertilization			
	DECEMBER:		
*Catch Basin and HDS Inspec	tion		
Sweeping of Paved Surfaces			

^{*}NOTE: Use appropriate worksheet found in this plan to conduct the inspection.

CATCH BASIN / CATCH BASIN INSERT INSPECTION LOG

Name of Inspector: Date:

Catch Basin	Conditi	Debris above 1' within sump? ion (circle (If yes then catch basin is to be				of Catch aning (if debris	Condition of Hood (if applicable, remove	
ID		ne)		aned)		ter than 1')	trash/debris if necessary)	Comments:
	Excellent							
		1	.,		.,			
	Fair	Poor	Yes	No	Yes	No		
	Exc	<u>l</u> ellent						
	Fair	Poor	Yes	No	Yes	No		
	Exc	ellent						
	Fair	Poor	Yes	No	Yes	No		
	Exc	<u> </u> ellent						
	Fair	Poor	Yes	No	Yes	No		
	Exc	ellent						
	Fair	Poor	Yes	No	Yes	No		
	Evo	 ellent						
	Fair	Poor	Yes	No	Yes	No		
	Excellent							
	Fair	Poor	Yes	No	Yes	No		
	Excellent							
	Fair	Poor	Yes	No	Yes	No		
	Exc	ellent						

On-site Procedures for Inspection and Maintenance of Catch Basin Inserts and Hydrodynamic Separators (HDS)

- Secure traffic and pedestrian traffic with cones, barrels, etc.
- Clean surface area around each catch basin.
- Remove grates and set aside
- Clean grates, remove litter and debris that may be trapped within the grate
- Visually inspect condition of outlet hood and remove trash and debris from hood if necessary.
- Remove by vactor hose the debris that has been trapped in the trough area. Dispose of in accordance with local, state and federal regulatory agency requirements. Most debris that is captured in the trough or sump area will fall into the non-hazardous waste category.
- Visually inspect and check the condition of the trough area.
- Replace grate and lockdown as needed.
- Un-secure traffic control area.
- Complete service report and submit to facility owner.

	SUBS	URFA	CE STC	DRMWATI	ER DE	TENTIC	ON SYS	ГЕМ А	ND BAS	SIN INS	SPECTION LO	G
Name o	f Inspector:						Date:					
Basin	Overall o	conditio	n of	Condition of Facility			Debris and Inlets and		Date of	Comments		
ID	ID Inlet Pipe (circle one)		(circle one)			Sediment Outlets are		Cleaning				
							Removed		Clear and		Performed	
							from Basin? ¹ Functioning?		oning?			
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		
	Excellent	Fair	Poor	Excellent	Fair	Poor	Yes	No	Yes	No		

^{1 –} Sediment deposits shall be removed from the subsurface detention basin when the deposited material reaches a height of 2" measured from the top of the stone bedding.