

## Assessment

**To:** Scott Leonard, One Buckland Center, PO Box 799, Rocky Hill, CT 06067

**e-Mail:** ScottLeonard1@yahoo.com

**Phone:** (860) 550-2323

**Date:** August 27, 2020

**Re:** *Evaluation of Improved Access Provisions*

*One Buckland Center, South Windsor, Connecticut*

### Update Request

At the request of the Town Planner, we have updated herein our last iteration of this assessment for the One Buckland Center project, dated December 30, 2019, to account for the latest update by AECOM of the ultimate Evergreen Walk development proposal, to include the impact of the proposed Costco development at the north end of Evergreen Walk, and to incorporate the proposed revisions by the Town of Manchester to the intersection of Buckland Street at Pleasant Valley Road and Buckland Hills Drive.

For the purposes of the traffic operations analyses included in this assessment, we utilized the combined weekday pm and Saturday peak combined (build) traffic volumes from the Traffic Impact Analysis of the proposed Costco development prepared by BL Companies, dated September 2019, revised July 2020. In the case of the weekday am peak, since not included in the Costco study, we developed our own estimates for the weekday am peak with the aforementioned Evergreen Walk revisions and the proposed Costco development in place.

### Description

We are pleased to submit herein our latest, updated evaluation of a possible provision to facilitate access to the subject One Buckland Center site from the northbound lanes of Buckland Road.

As One Buckland Center currently exists, the subject site has one site drive on the west side of Buckland Road that is only accessible by southbound right-turning traffic both inbound and outbound, with no easy way for northbound traffic to access the site without passing the site in the northbound direction and then negotiating a U-turn somewhere to the north of the site to then approach the site in the southbound direction.

It is understood and proposed that exiting traffic will continue to exit the site by making a right turn onto Buckland Road and heading south on Buckland Road to either continue south, or by negotiating a U-turn at the signalized intersection at the entrance to Buckland Mall, to head north on Buckland Road.

### **Option Considered**

There has been some concern raised over the illegal U-turns that currently occur at the northern end of the median separating the northbound and southbound lanes of Buckland Road in the vicinity of One Buckland Center, presumably to reach the subject site from points west, south and east of the site; as well as to improve the restricted access currently afforded to the subject development. Note that this U-turn is posted as an illegal maneuver and poses a potential hazard to those making the maneuver as well as to southbound traffic that may come into conflict with those making this illegal maneuver.

Please refer to Exhibit 1 of the Appendix which shows what we consider a viable manner to avoid this illegal U-turn maneuver and instead provide a safe alternative for those wishing to gain access to the subject site at One Buckland Road from the northbound lanes of Buckland Road safely and efficiently. This alternative involves making a median break between the northbound and southbound lanes in line with the site drive and providing a northbound left-turn lane for storing, sheltering and processing traffic entering the site from the south.

### **Supporting Information**

In support of our recommendation, we have included herein the following information in the way of Exhibits in the Appendix and/or Tables below, some of which have been updated from the previous December 30, 2019 version of this assessment:

- A. Site-generated traffic volumes and distributions for the subject development assuming the proposed median break in place allowing additional access from the south:

Exhibit 8 - Site-Generated Weekday (build) AM Peak Hour Volumes

Exhibit 9 - Site-Generated Weekday (build) PM Peak Hour Volumes

Exhibit 10 - Site-Generated Saturday (build) Midday Peak Hour Volumes

A review of Exhibits 8 through 10 show that the proposed full development of One Buckland Center with a combination of office and retail uses will remain a very low traffic generator.

- B. Revised Combined (Build) peak hour traffic volumes for the subject study area based on the information made available in the aforementioned Costco Study which addresses the aforementioned Evergreen Walk modifications and addition of Costco:

Revised Exhibit 11 - Combined (Build) Weekday AM Peak Hour

Revised Exhibit 12 - Combined (Build) Weekday PM Peak Hour

Revised Exhibit 13 - Combined (Build) Saturday Mid-Day Peak Hour

- C. Updated traffic operations analyses of projected combined (build) peak hour conditions with the aforementioned Evergreen Walk, Costco and One Buckland Center revisions and updates in place:

Revised Exhibit 18 - Traffic Operations Analysis Worksheets  
Combined Weekday AM Peak Hour

Revised Exhibit 19 - Traffic Operations Analysis Worksheets  
Combined Weekday PM Peak Hour

Revised Exhibit 20 - Traffic Operations Analysis Worksheets  
Combined Saturday Mid-Day Peak Hour

- D. Revised Table 3 which summarizes the updated traffic operations analyses during the three selected study peak periods based on projected combined (build) conditions with the aforementioned Evergreen Walk, Costco and One Buckland Center revisions and updates in place.

**A review of revised Table 3 indicates that the proposed addition of One Buckland Center's traffic, with the proposed improvements in place, will continue to provide satisfactory levels of service, and have an insignificant impact on the levels of service that would otherwise be in place without any revisions to One Buckland Center as proposed.**

- E. Revised Table 4 which summarizes the estimated 95<sup>th</sup> percentile queues at the three study intersections based on projected combined (build) conditions with the aforementioned Evergreen Walk, Costco and One Buckland Center revisions and updates in place.

**A review of revised Table 4 indicates that the proposed addition of One Buckland Center's traffic, with the proposed improvements in place, can be accommodated satisfactorily without causing conditions where queued vehicles block key turn lanes and/or intersections.**

**Revised Table 3**  
**Summary of Traffic Operations Analysis**  
**Levels of Service**  
**Proposed Retail Center**  
**One Buckland Center**  
**South Windsor, Connecticut**

**Combined Build**

**(With Updated Evergreen Walk Traffic  
and Costco Traffic as Background)**

<b>AM Peak</b>	<b>PM Peak</b>	<b>Sat Peak</b>
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**Buckland Road at Evergreen Walk (Tamarack) and Lowe's Drive**

Buckland Road northbound approach	C	E	D
Buckland Road southbound approach	C	D	D
Tamarack Road eastbound approach	B	C	D
Lowe's Site Drive westbound approach	B	D	E
<b>Overall</b>	<b>- C -</b>	<b>- D -</b>	<b>- D -</b>
<b>Average Delay per Vehicle (sec.)</b>	<b>23.6</b>	<b>53.7</b>	<b>49.4</b>

**Buckland Road at One Buckland Center Site Drive**

Buckland Road northbound approach	A	A	A
Buckland Road southbound approach	B	B	B
Proposed Site Drive eastbound (outbound) right	A	A	A
<b>Overall</b>	<b>- A -</b>	<b>- A -</b>	<b>- A -</b>
<b>Average Delay per Vehicle (sec.)</b>	<b>7.3</b>	<b>6.6</b>	<b>9.4</b>

**Buckland Street at Pleasant Valley Road and Buckland Hills Road**

Buckland Street northbound approach	B	D	E
Buckland Street southbound approach	B	D	E
Pleasant Valley Road eastbound approach	C	D	E
Buckland Hills Drive westbound approach	C	D	E
<b>Overall</b>	<b>- B -</b>	<b>- D -</b>	<b>- E -</b>
<b>Average Delay per Vehicle (sec.)</b>	<b>19.9</b>	<b>46.4</b>	<b>63.5</b>

**Revised Table 4**  
**Summary of Queuing Analyses**  
**95th Percentile Queues**  
**Proposed Retail Center**  
**One Buckland Center**  
**South Windsor, Connecticut**

Approach	Segment	<--- Combined Build --->			<u>COMMENT</u>	
		<--- (With Updated Evergreen Walk Traffic --->		<u>SAT Peak</u>		
		<--- and Costco Traffic as Background) --->		<u>AM Peak</u>		
Available Storage (feet)		<u>PM Peak</u>	<u>PM Peak</u>	<u>STATUS</u>		
Buckland Road southbound	<u>Between Tamarack Avenue and One Buckland Center:</u>					
	Southbound Through Lanes:	750 feet	367 feet	356 feet	OK	
	Southbound Right-Turn Lane:	125 feet	0 feet	0 feet	OK	
	<u>Between proposed site drive and Pleasant Valley Road:</u>					
	Southbound Left-Turn Lanes:	550 feet	34 feet	201 feet	OK	
	Southbound Through Lanes:	650 feet	108 feet	325 feet	OK	
	Southbound Right-Turn Lane:	650 feet	194 feet	663 feet	OK	
Buckland Road northbound	<u>Between Buckland Hills Road and One Buckland Center:</u>					
	Northbound left-turn lane:	150 feet	46 feet	106 feet	-----	
	Northbound through lanes:	600 feet	0 feet	0 feet	OK	
	<u>Between One Buckland Center and Lowe's Site Drive:</u>					
	Northbound Left-Turn Lanes:	450 feet	105 feet	357 feet	OK	
	Northbound Through Lanes:	775 feet	393 feet	635 feet	OK	
	Northbound Right-Turn Lane:	100 feet	3 feet	11 feet	OK	
					Provide minimum 150-foot long, full width left turn lane.	

## Recommendation

Our previous and updated findings indicate that the provision of a signalized left-turn lane in the Buckland Road northbound direction at the site drive serving One Buckland Center, which is located on the west side of the road, can be accommodated and would serve this site and area well with no apparent adverse impacts.

We recommend that a traffic signal be installed on Buckland Road at the subject site drive which only stops Buckland Road southbound traffic when there is either demand for a northbound vehicle to make a left-turn into the site, or for an exiting vehicle to make a right-turn out of the site. Left-turns OUT of the site will continue NOT to be allowed. This revision would also enable Buckland Road northbound traffic wishing to make a U-turn to head south to do so in protected fashion as well. Approaching northbound left-turning traffic should be provided a dedicated left-turn lane in the median area that is controlled by a left-turn signal which displays only upon demand at a designated point in the signal's cycle.

We further recommend that this signal be interconnected to the immediate traffic signal to the north at the intersection of Buckland Road at Tamarack Road (Evergreen Walk) and Lowe's site drive. Specifically, the display of the left-turn phase at the proposed signal and turn lane should coincide with the display of the northbound double-left turn phase at the Tamarack Road signal when Buckland Road southbound traffic is already stopped at that location to allow the left-turn movement into Evergreen Walk. Additionally, the outbound right-turn out of the subject site should coincide with the phase allocated for moving Lowe's traffic out of that site. Such should pose the least constraints to existing traffic operations while providing a significant improvement.

Very truly yours,  
Bubaris Traffic Associates




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James G. Bubaris, P.E.  
Conn. Reg. No. 9203  
Principal



**Site Traffic Evaluation Study  
One Buckland Center  
South Windsor, Connecticut**

**Revised Appendix**

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Revised Exhibit 11	Combined (Build) Weekday AM Peak Hour
Revised Exhibit 12	Combined (Build) Weekday PM Peak Hour
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-----	
Exhibit 14	Definitions of Levels of Service – Signalized Intersections
-----	
Revised Exhibit 18	Traffic Operations Analysis Worksheets Combined Weekday AM Peak Hour
Revised Exhibit 19	Traffic Operations Analysis Worksheets Combined Weekday PM Peak Hour
Revised Exhibit 20	Traffic Operations Analysis Worksheets Combined Saturday Mid-Day Peak Hour
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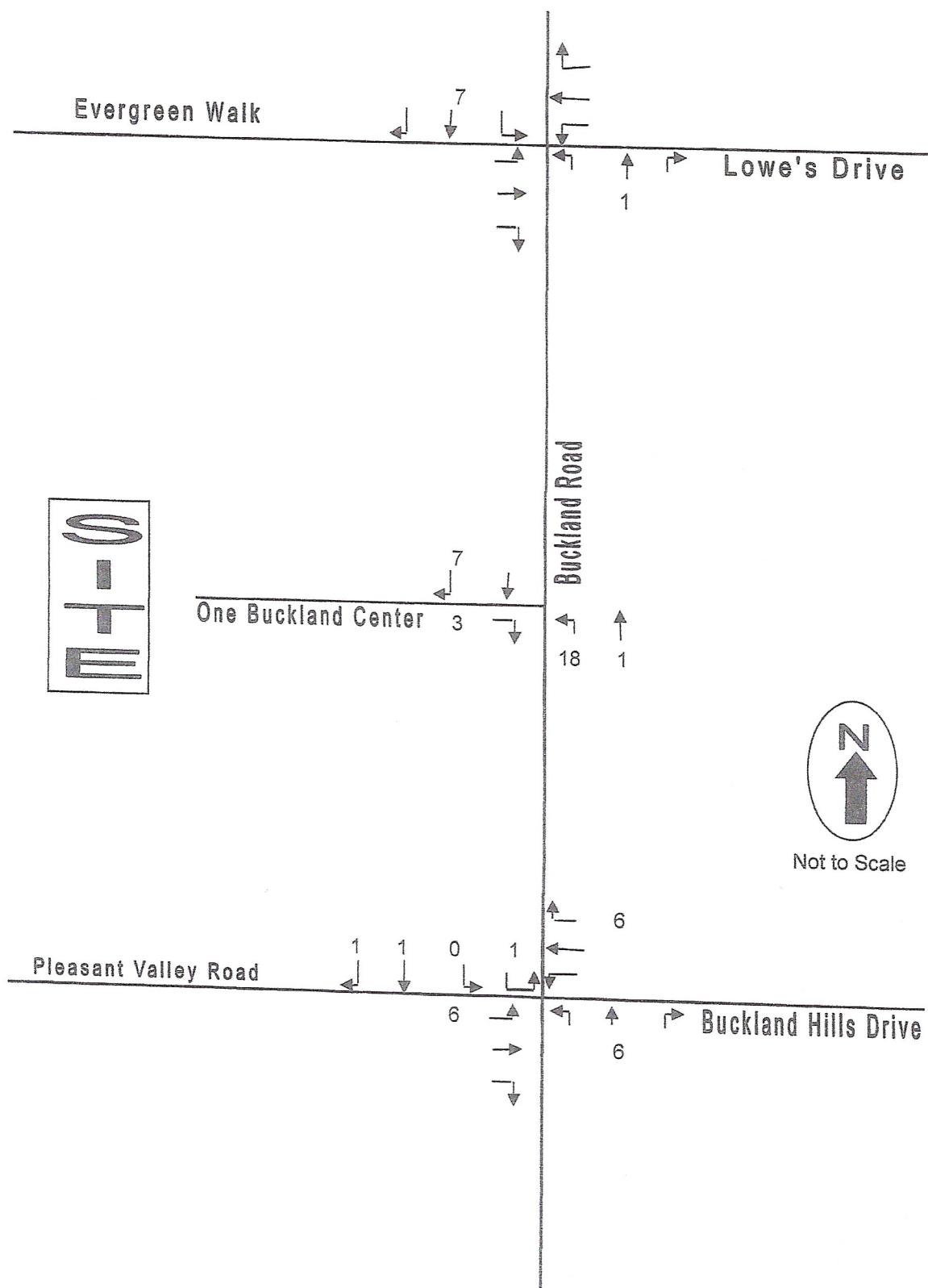
**Revised Exhibit 2  
Proposed Northbound Left-Turn Lane  
One Buckland Center  
South Windsor, Connecticut**



**Exhibit 8**  
**One Buckland Center Site Generated Weekday AM Peak Hour**

July 2018

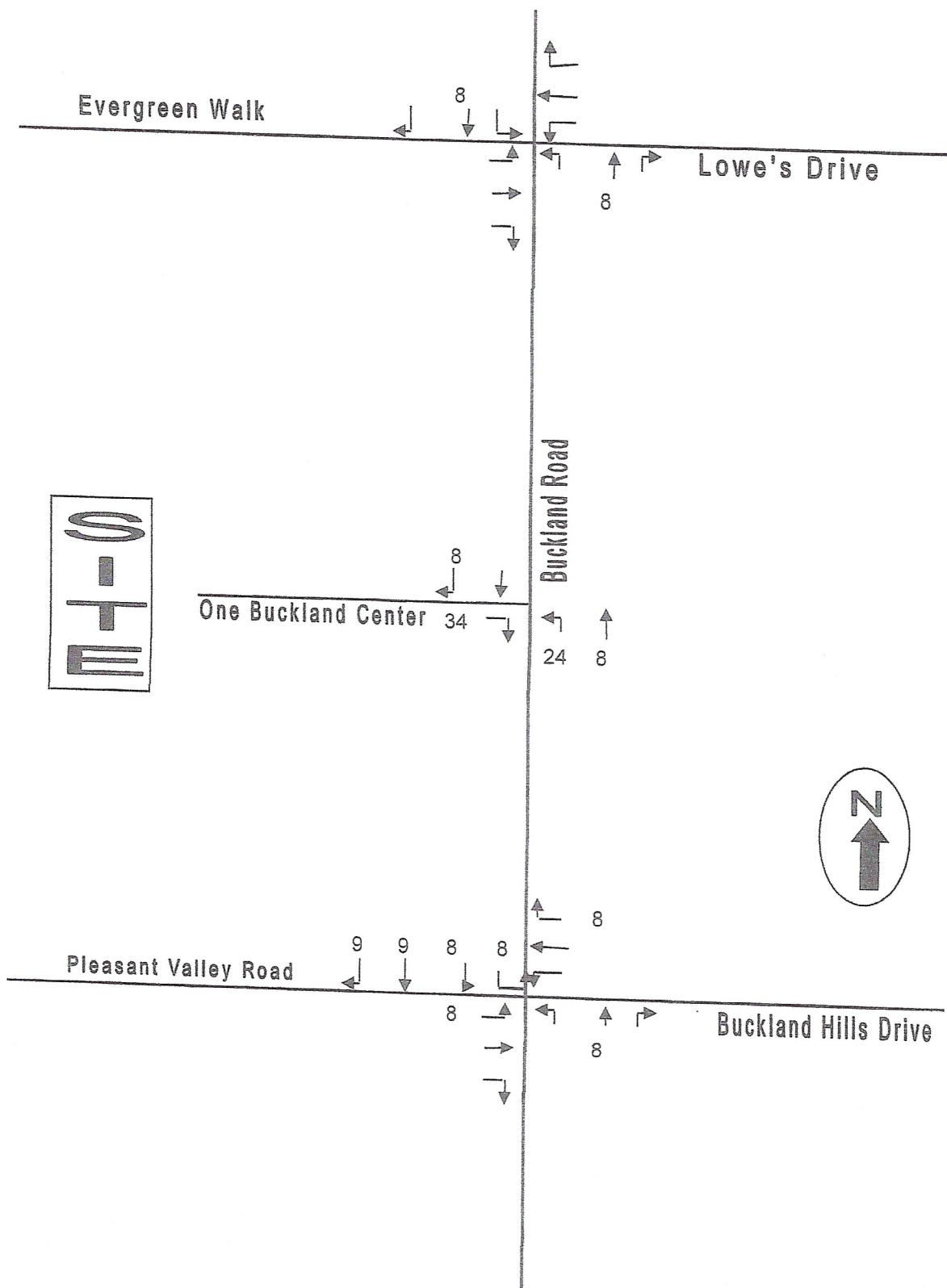
One Buckland Center  
South Windsor, Connecticut



**Exhibit 9**  
**One Buckland Center Site Generated Weekday PM Peak Hour**

July 2018

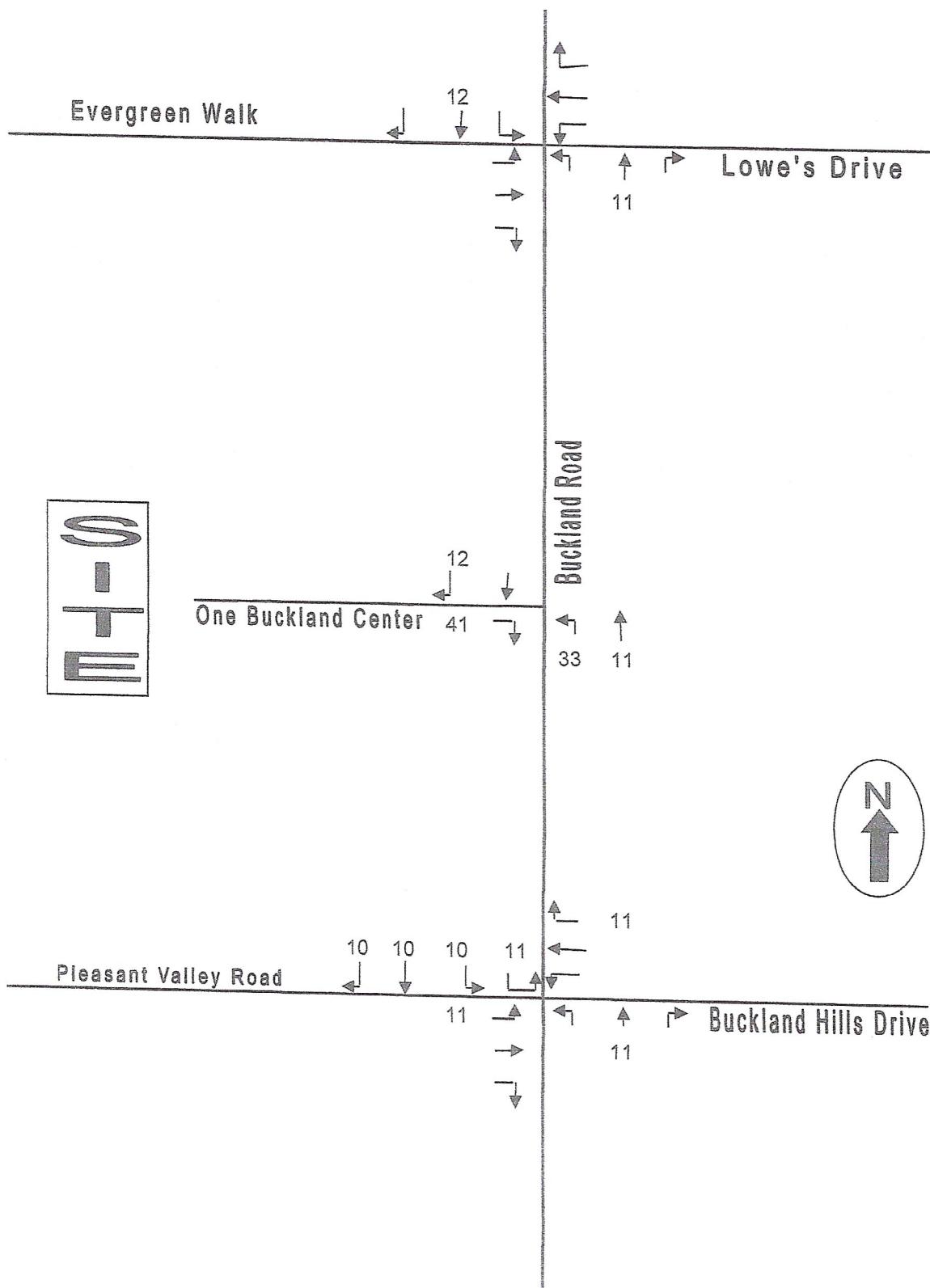
One Buckland Center  
South Windsor, Connecticut



**Exhibit 10**  
**One Buckland Center Site Generated Saturday Mid-day Peak Hour**

July 2018

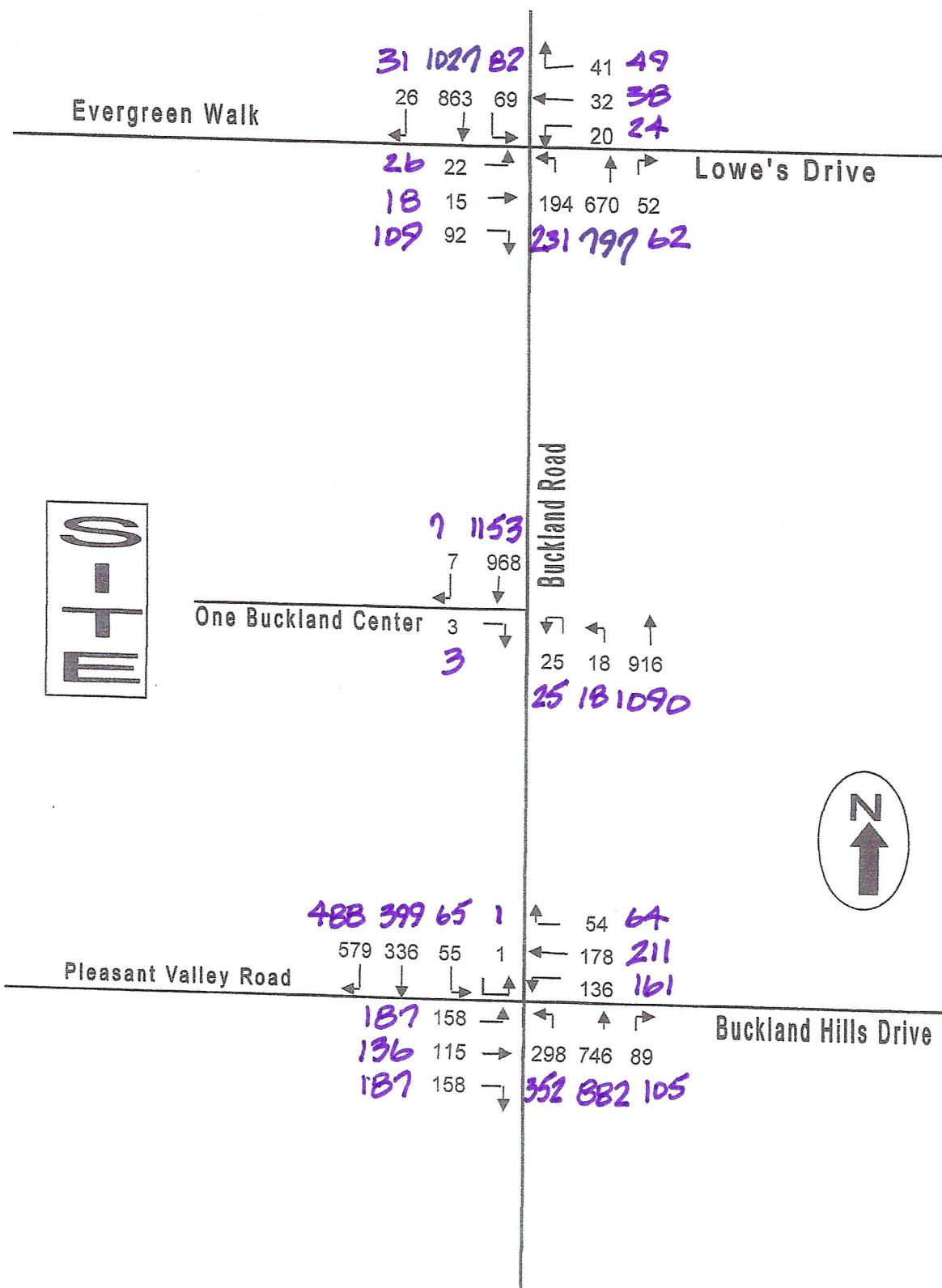
One Buckland Center  
South Windsor, Connecticut



**Revised Exhibit 11  
Combined (Build) Weekday AM Peak Hour**

November 2019

One Buckland Center  
South Windsor, Connecticut



Revised

**Revised Exhibit 12  
Combined (Build) Weekday PM Peak Hour**

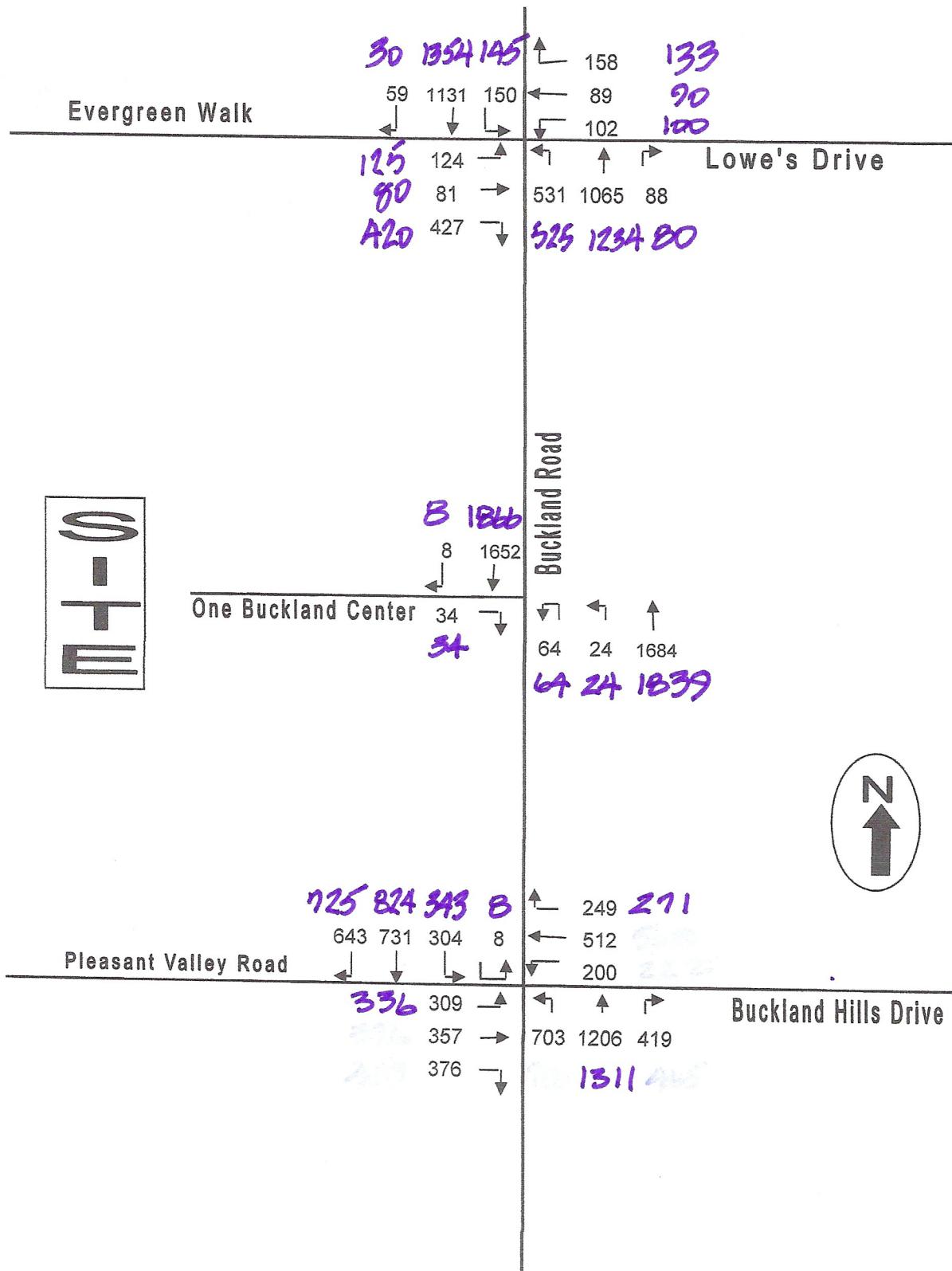
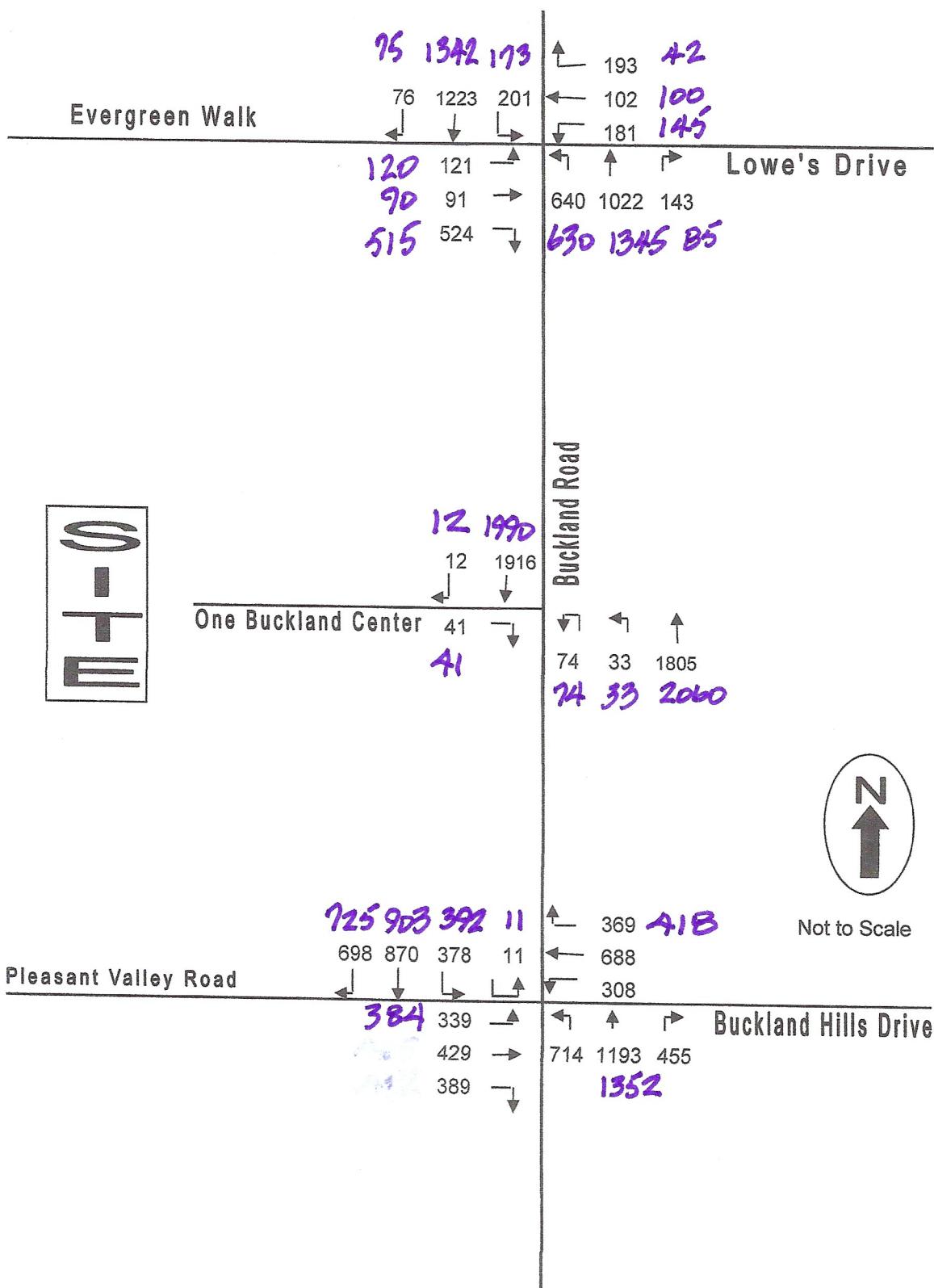
*Revised*

Exhibit 12  
Combined 2020 Weekday PM  
Peak Hour Traffic Volumes

**Revised Exhibit 13  
Combined (Build) Saturday Mid-Day Peak Hour**

November 2019

One Buckland Center  
South Windsor, Connecticut



**EXHIBIT 14**  
**LEVEL OF SERVICE CRITERIA**  
**SIGNALIZED INTERSECTIONS**

SOURCE: **HIGHWAY CAPACITY MANUAL (HCM), 2010**  
**TRANSPORTATION RESEARCH BOARD (1)**

Level of Service for **signalized intersections** is defined in terms of control delay, which is a measure of driver discomfort, frustration, increased fuel consumption, and lost travel time. The delay experienced by a motorist is comprised of a number of factors that relate to control, geometric, traffic, and incidents. Total delay is the difference between the travel time actually experienced and the reference travel time that would result during base conditions in the absence of traffic control, geometric delay, any incidents, and any other vehicles. Specifically, LOS criteria for traffic signals are stated in terms of the average control delay per vehicle, typically for a 15-minute analysis period. Delay is a complex measure and depends on a number of variables, including the quality of progression, the cycle length, the green ratio, and the volume-to-capacity (v/c) ratio for the lane group.

In the case of **signalized intersections**, the Level of Service for each approach is computed, and an overall Level of Service for the entire intersection is determined.

Levels of Service (LOS) for **signalized intersections** are defined as follows:

LEVEL OF SERVICE	CONTROL DELAY PER VEHICLE (SECONDS)	CONDITION
LOS A	$\leq 10$	LOW DELAY
LOS B	> 10 TO 20	SHORT DELAY
LOS C	> 20 TO 35	AVERAGE DELAY
LOS D	> 35 TO 55	CONGESTION NOTICEABLE
LOS E	> 55 TO 80	LIMIT OF ACCEPTABLE DELAY
LOS F	> 80	UNACCEPTABLE

In today's environment, Levels of Service C to D are considered acceptable, and Levels of Service A to B are seldomly achieved at signalized intersections.

(1) HCM, Exhibit 16-2.

**Revised Exhibit 18  
Traffic Operations Analysis Worksheets  
Combined Weekday AM Peak Hour**



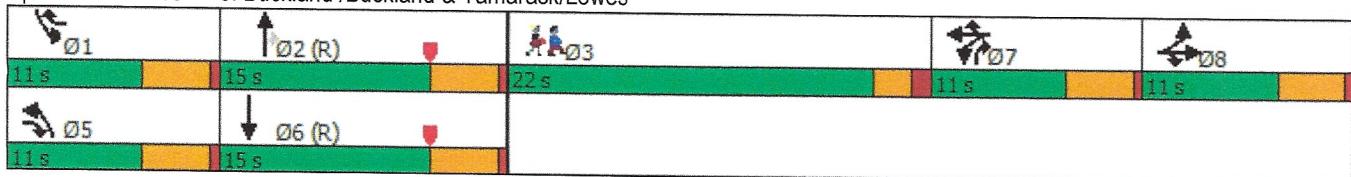
Lane Group	Ø3
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Fr <sub>t</sub>	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	3
Permitted Phases	



Lane Group	Ø3
Detector Phase	
Switch Phase	
Minimum Initial (s)	7.0
Minimum Split (s)	23.0
Total Split (s)	22.0
Total Split (%)	31%
Maximum Green (s)	19.0
Yellow Time (s)	2.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	13.0
Pedestrian Calls (#/hr)	5
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

Queue shown is maximum after two cycles.

Splits and Phases: 3: Buckland /Buckland & Tamarack/Lowes











**Revised Exhibit 19**  
**Traffic Operations Analysis Worksheets**  
**Combined Weekday PM Peak Hour**



Lane Group	Ø3
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Fr <sub>t</sub>	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	3
Permitted Phases	



Lane Group	Ø3
Detector Phase	
Switch Phase	
Minimum Initial (s)	7.0
Minimum Split (s)	23.0
Total Split (s)	23.0
Total Split (%)	23%
Maximum Green (s)	20.0
Yellow Time (s)	2.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	13.0
Pedestrian Calls (#/hr)	5
Act Effct Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

## Lanes, Volumes, Timings

### 3: Buckland /Buckland & Tamarack/Lowes

Combined Weekday PM with Cosco (08.20)

08/26/2020

Queue shown is maximum after two cycles.

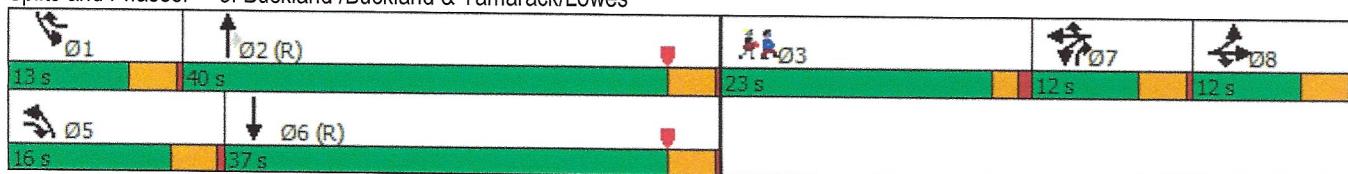
# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 3: Buckland /Buckland & Tamarack/Lowes





Lanes, Volumes, Timings  
6: One Buckland Center & Buckland

Combined Weekday PM with Cosco (08.20)  
08/26/2020



Lane Group	EBL	EBR	NBU	NBL	NBT	SBT	SBR
Detector Phase		3	1	1	1 2 3	2	2
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	9.5	9.5	9.5		22.5	22.5	
Total Split (s)	9.5	19.0	19.0		71.5	71.5	
Total Split (%)	9.5%	19.0%	19.0%		71.5%	71.5%	
Maximum Green (s)	5.0	15.0	15.0		67.0	67.0	
Yellow Time (s)	3.5	3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	0.5	0.5		1.0	1.0	
Lost Time Adjust (s)	0.0		0.0		0.0	0.0	
Total Lost Time (s)	4.5		4.0		4.5	4.5	
Lead/Lag		Lead	Lead		Lag	Lag	
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None		C-Min	C-Min	
Act Efft Green (s)	5.0		15.0	100.0	67.0	67.0	
Actuated g/C Ratio	0.05		0.15	1.00	0.67	0.67	
v/c Ratio	0.19		0.36	0.56	0.86	0.01	
Control Delay	2.1		42.6	0.7	10.8	0.2	
Queue Delay	0.0		0.0	0.0	0.0	0.0	
Total Delay	2.1		42.6	0.7	10.8	0.2	
LOS	A		D	A	B	A	
Approach Delay	2.1			2.6	10.8		
Approach LOS	A			A	B		
Queue Length 50th (ft)	0		56	0	115	0	
Queue Length 95th (ft)	0		106	0	356	m0	
Internal Link Dist (ft)	901			83	760		
Turn Bay Length (ft)			150		200		
Base Capacity (vph)	198		265	3539	2371	1063	
Starvation Cap Reductn	0		0	0	10	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.19		0.36	0.56	0.86	0.01	

#### Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Offset: 13 (13%), Referenced to phase 2:NBSB, Start of Yellow

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.86

Intersection Signal Delay: 6.6

Intersection LOS: A

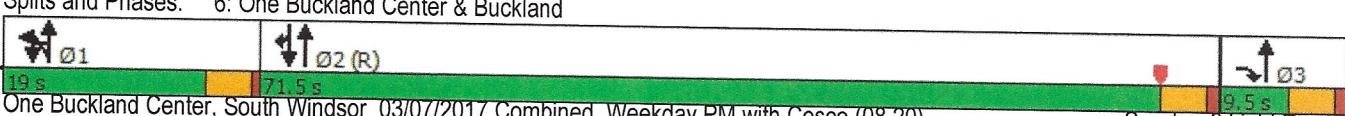
Intersection Capacity Utilization 71.5%

ICU Level of Service C

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: One Buckland Center & Buckland







**Revised Exhibit 20**  
**Traffic Operations Analysis Worksheets**  
**Combined Saturday Mid-Day Peak Hour**



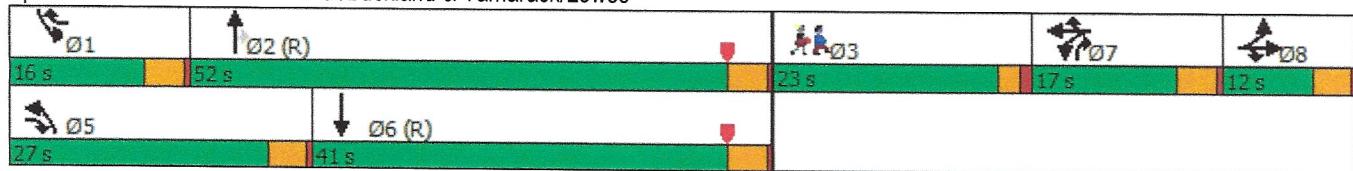
Lane Group	Ø3
Lane Configurations	
Traffic Volume (vph)	
Future Volume (vph)	
Ideal Flow (vphpl)	
Storage Length (ft)	
Storage Lanes	
Taper Length (ft)	
Lane Util. Factor	
Fr <sub>t</sub>	
Flt Protected	
Satd. Flow (prot)	
Flt Permitted	
Satd. Flow (perm)	
Right Turn on Red	
Satd. Flow (RTOR)	
Link Speed (mph)	
Link Distance (ft)	
Travel Time (s)	
Peak Hour Factor	
Adj. Flow (vph)	
Shared Lane Traffic (%)	
Lane Group Flow (vph)	
Enter Blocked Intersection	
Lane Alignment	
Median Width(ft)	
Link Offset(ft)	
Crosswalk Width(ft)	
Two way Left Turn Lane	
Headway Factor	
Turning Speed (mph)	
Number of Detectors	
Detector Template	
Leading Detector (ft)	
Trailing Detector (ft)	
Detector 1 Position(ft)	
Detector 1 Size(ft)	
Detector 1 Type	
Detector 1 Channel	
Detector 1 Extend (s)	
Detector 1 Queue (s)	
Detector 1 Delay (s)	
Detector 2 Position(ft)	
Detector 2 Size(ft)	
Detector 2 Type	
Detector 2 Channel	
Detector 2 Extend (s)	
Turn Type	
Protected Phases	3
Permitted Phases	



Lane Group	Ø3
Detector Phase	
Switch Phase	
Minimum Initial (s)	7.0
Minimum Split (s)	23.0
Total Split (s)	23.0
Total Split (%)	19%
Maximum Green (s)	20.0
Yellow Time (s)	2.0
All-Red Time (s)	1.0
Lost Time Adjust (s)	
Total Lost Time (s)	
Lead/Lag	
Lead-Lag Optimize?	
Vehicle Extension (s)	3.0
Recall Mode	None
Walk Time (s)	7.0
Flash Dont Walk (s)	13.0
Pedestrian Calls (#/hr)	5
Act Effect Green (s)	
Actuated g/C Ratio	
v/c Ratio	
Control Delay	
Queue Delay	
Total Delay	
LOS	
Approach Delay	
Approach LOS	
Queue Length 50th (ft)	
Queue Length 95th (ft)	
Internal Link Dist (ft)	
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

- Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer.
- Queue shown is maximum after two cycles.
- m Volume for 95th percentile queue is metered by upstream signal.
- dl Defacto Left Lane. Recode with 1 though lane as a left lane.

Splits and Phases: 3: Buckland /Buckland & Tamarack/Lowes





Lanes, Volumes, Timings  
6: One Buckland Center & Buckland

Combined Saturday Peak with Cosco (08.20)

08/26/2020



Lane Group	EBL	EBR	NBU	NBL	NBT	SBT	SBR
Detector Phase		3	1	1	1 2 3	2	2
Switch Phase							
Minimum Initial (s)	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	9.5	9.5	9.5		22.5	22.5	
Total Split (s)	9.5	24.0	24.0		86.5	86.5	
Total Split (%)	7.9%	20.0%	20.0%		72.1%	72.1%	
Maximum Green (s)	5.0	20.0	20.0		82.0	82.0	
Yellow Time (s)	3.5	3.5	3.5		3.5	3.5	
All-Red Time (s)	1.0	0.5	0.5		1.0	1.0	
Lost Time Adjust (s)	0.0		0.0		0.0	0.0	
Total Lost Time (s)	4.5		4.0		4.5	4.5	
Lead/Lag		Lead	Lead		Lag	Lag	
Lead-Lag Optimize?							
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0	
Recall Mode	Min	None	None		C-Min	C-Min	
Act Effct Green (s)	5.0		20.0	120.0	82.0	82.0	
Actuated g/C Ratio	0.04		0.17	1.00	0.68	0.68	
v/c Ratio	0.24		0.39	0.63	0.89	0.01	
Control Delay	3.0		49.1	0.9	16.3	4.2	
Queue Delay	0.0		0.0	0.0	0.0	0.0	
Total Delay	3.0		49.1	0.9	16.3	4.2	
LOS	A		D	A	B	A	
Approach Delay	3.0			3.2	16.2		
Approach LOS	A			A	B		
Queue Length 50th (ft)	0		81	0	305	0	
Queue Length 95th (ft)	0		141	0	m644	m1	
Internal Link Dist (ft)	901			83	760		
Turn Bay Length (ft)			150		200		
Base Capacity (vph)	188		295	3539	2418	1084	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.24		0.39	0.63	0.89	0.01	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBSB, Start of Yellow

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.89

Intersection Signal Delay: 9.4

Intersection LOS: A

Intersection Capacity Utilization 75.9%

ICU Level of Service D

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: One Buckland Center & Buckland



One Buckland Center, South Windsor 03/07/2017 Combined Saturday Peak with Cosco (08.20)  
Bubaris Traffic Associates

Synchro 9 Light Report  
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## Lanes, Volumes, Timings

### 9: Buckland & Pleasant Valley/Buckland Hills

Combined Saturday Peak with Cosco (08.20)

08/27/2020

Splits and Phases: 9: Buckland & Pleasant Valley/Buckland Hills

